

# DEPARTMENT OF TRANSPORTATION STATE OF GEORGIA

## OFFICE OF DESIGN POLICY & SUPPORT INTERDEPARTMENTAL CORRESPONDENCE

**FILE** P.I. # 0014131

**OFFICE** Design Policy & Support

Cherokee County  
GDOT District 6 - Cartersville  
SR 20 Widening from CR 281/Scott  
Road to CR 762/Union Hill Road

**DATE** 11/14/2017

**FROM**  for Brent Story, State Design Policy Engineer

**TO** SEE DISTRIBUTION

**SUBJECT** APPROVED CONCEPT REPORT

Attached is the approved Concept Report for the above subject project.

Attachment

**DISTRIBUTION:**

Hiral Patel, Director of Engineering  
Joe Carpenter, Director of P3  
Albert Shelby, Director of Program Delivery  
Darryl VanMeter, Assistant Director of P3/State Innovative Delivery Administrator  
Kim Nesbitt, Program Delivery Administrator  
Bobby Hilliard, Program Control Administrator  
Cindy VanDyke, State Transportation Planning Administrator  
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Andrew Heath, State Traffic Engineer  
Angela Robinson, Financial Management Administrator  
Lisa Myers, State Project Review Engineer  
Monica Flournoy, State Materials and Testing Administrator  
Patrick Allen, State Utilities Engineer  
Benny Walden, Statewide Location Bureau Chief  
DeWayne Comer, District Engineer  
David Acree, District Preconstruction Engineer  
Jun Birnkammer, District Utilities Engineer  
Cleopatra James, Project Manager  
BOARD MEMBER - 11th Congressional District

DEPARTMENT OF TRANSPORTATION  
STATE OF GEORGIA  
PROJECT CONCEPT REPORT

Project Type: Reconstruction/ Rehabilitation  
P.I. Number: 0014131  
GDOT District: 6  
County: Cherokee  
Federal Route Number: N/A  
State Route Number: 20  
Project Number: N/A

Widening of SR 20 from CR 281/Scott Rd to CR 762/Union Hill Rd

Submitted for approval:  
Scott Gero, AECOM

6/30/17

Consultant Designer & Firm

Date  
7/14/17

State Program Delivery Administrator

Cleopatra James

Date

7/7/17

GDOT Project Manager

Date

Recommendation for approval:

ERIC DUFF\*/EKP

Date  
7/18/2017

State Environmental Administrator

Date

FOR State Traffic Engineer

CHRISTINA BARRY\*/EKP

Date  
7/28/2017

FOR Project Review Engineer

ERIK ROHDE\*/EKP

Date  
7/29/2017

FOR State Utilities Engineer

KEVIN COWAN\*/EKP

Date  
7/27/2017

FOR District Engineer

DAVID ACREE\*/EKP

Date  
7/28/2017

☒ MPO Area: This project is consistent with the MPO adopted Regional Transportation Plan (RTP)/Long Range Transportation Plan (LRTP).

☐ Rural Area: This project is consistent with the goals outlined in the Statewide Transportation Plan (SWTP) and/or is included in the State Transportation Improvement Program (STIP).

CINDY VANDUYKE\*/EKP

Date  
7/19/2017

State Transportation Planning Administrator

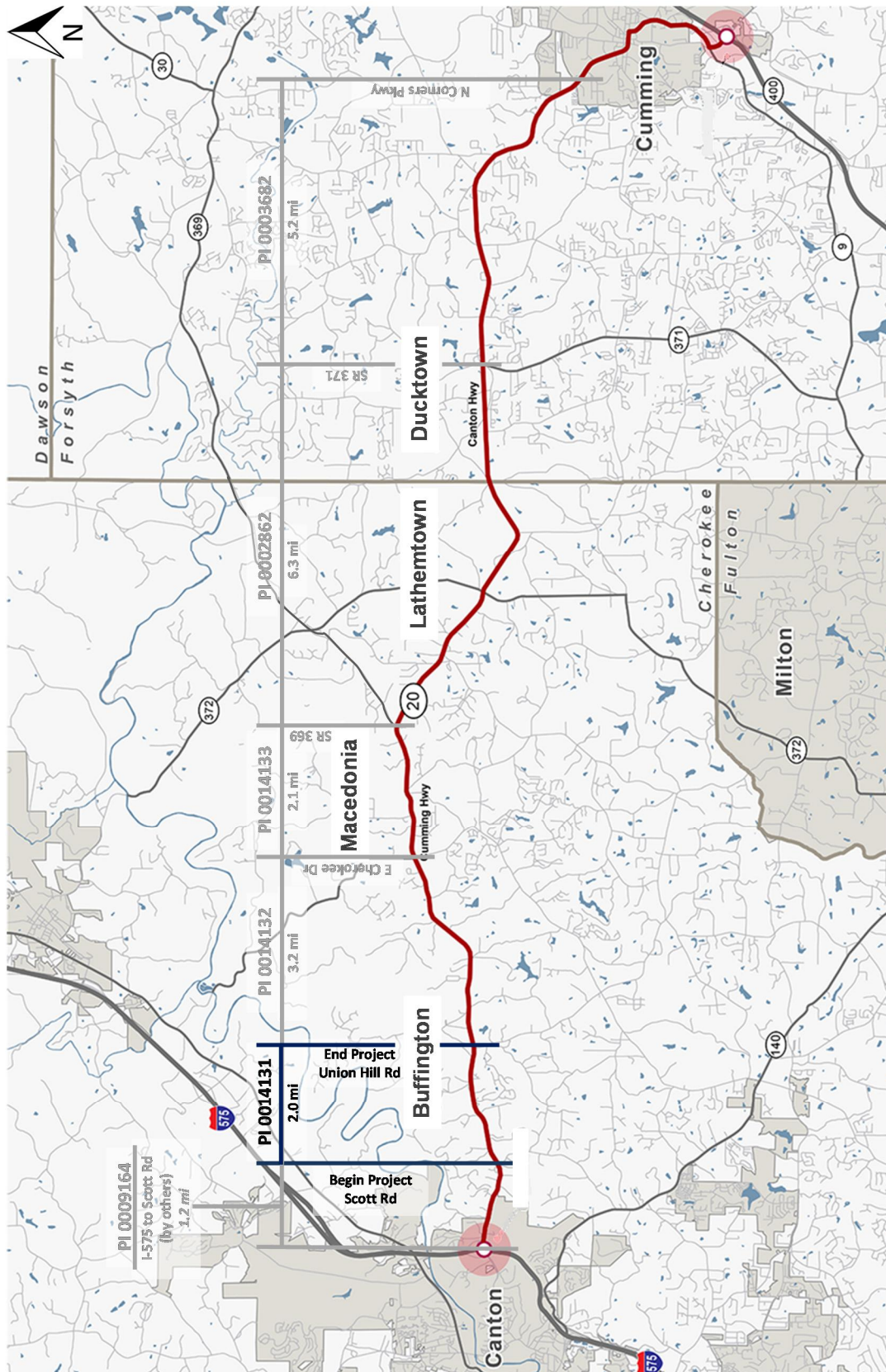
Date

\* - RECOMMENDATION ON FILE



County: Cherokee

## PROJECT LOCATION MAP



County: Cherokee

## PLANNING AND BACKGROUND

**Project Justification Statement:** The following Project Justification Statement was provided for PI 0003681, PI 0002862, and PI 0003682 by the Office of Planning on June 18, 2012. PI 0003681 was later divided into three separate projects: PI 0014131, PI 0014132, and PI 0014133.

SR 20 is a two lane corridor from I-575 to just west of SR 400 where it changes to four lanes south of Crestbrook Drive/Forsyth County through the SR 400 interchange. Based on 2011 Average Annual Daily Traffic (AADT) the current level of service (LOS) of SR 20 from I-575 to SR 369 is “F” with an AADT of 25,650. The SR 369/Cherokee County to SR 371/Forsyth County segment has an AADT of 13,550 and LOS “D”. SR 20 from SR 371 to Crestbrook Drive has an AADT of 22,400 and LOS “E”. SR 20 increases to a four lane corridor from south of Crestbrook Drive to SR 400 and has a LOS “C” and AADT of 34,200.

On the western end of the project, the no build scenario design traffic (2040) for SR 20 is 53,550 with LOS “F”. Between SR 369 and SR 371, the 2040 traffic is 35,050 with LOS “F”. SR 20 between SR 371 and SR 400 has a LOS of “F”, with design traffic of 42,000 where SR 20 is two lanes. Where SR 20 is four lanes west of SR 400 it is LOS “D” (52,950).

SR 20 is classified as an urban principal arterial from I-575 to Union Hill Rd/Cherokee County, a rural principal arterial from Union Hill Rd. to County Line Rd, and then an urban principal arterial again from County Line Rd to SR 400/Forsyth County. The crash rates for the section of SR 20 in Cherokee County (east of I-575) were above the statewide average for the urban principal arterial and below for the rural principal arterial road in the years 2007-2009. The rates for the portion of SR 20 classified as an urban principal arterial in the years 2007-2009 were 245, 200, and 320 crashes per 100 million vehicle miles traveled (MVMT), whereas the statewide averages were 176, 170, and 165 crashes per 100 MVMT. The rates for the portion of SR 20 classified as a rural principal arterial in the years 2007- 2009 were 228, 186, and 173 crashes per 100 MVMT respectively, whereas the statewide averages were 249, 249, and 235 crashes per 100 MVMT. The crash rates for the portion of SR 20 in Forsyth County were all above the statewide averages. In the years 2007-2009 the crash rates were 480, 459, and 290 crashes per 100 MVMT for an urban principal arterial.

The future (2040) traffic for this section of the SR 20 corridor is anticipated to have deficient LOS, from I-575 to SR 400. West of I-575 traffic volumes on SR 20 decline from 23,500 ADT (LOS B) to 15,950 ADT (LOS D). Therefore, it is the opinion of the Office of Planning that I-575 could serve as the western logical termini. The four-lane section starting at Crestbrook Drive would serve as the eastern termini.

The Statewide Transportation Plan defines acceptable LOS as “A” to “C”, with sometimes “D” being used in large urban areas based on the circumstances. The goals of these projects are to alleviate present and future congestion along SR 20 between I-575 and SR 400 and to reduce the crash frequency along the corridor.

**Existing conditions:** The existing highway consists of a rural two-lane, undivided section from the project beginning at Scott Rd to the project end at Union Hill Rd, with some left and right turn lanes at larger intersections. Union Hill Rd is the only major intersection along the project. Scott Rd and Union Hill Rd are the only signalized intersections. There are no sidewalks, major structures, or major utilities.

### Other projects in the area:

PI 0014132 – SR 20 FROM CR 762/UNION HILL RD TO CR 765/EAST CHEROKEE DR  
 PI 0014133 – SR 20 FROM CR 765/EAST CHEROKEE DRIVE TO SR 369  
 PI 0002862 – SR 20 FROM SR 369/CHEROKEE TO SR 371/FORSYTH  
 PI 0003682 – SR 20 FROM SR 371 TO N CORNERS PKWY (West side of Cumming)  
 PI 0009164 – SR 20 FM 0.34 MI E OF I-575 TO 0.15 E OF CR 281/SCOTT RD

**MPO:** Atlanta TMA

**TIP #:** CH-020B

**Congressional District(s):** 11



County: Cherokee

**Federal Oversight:** ☐ PoDI ☐ Exempt ☒ State Funded ☐ Other

**Projected Traffic:** ADT 24 HR T: 16 %

Current Year (2011): 21,550 Open Year (2025): 31,900 Design Year (2045): 56,900

Traffic Projections Performed by: GCA, Inc.

Date approved by the GDOT Office of Planning: 5/20/14

**Functional Classification (Mainline):** Urban Principal Arterial

**Complete Streets - Bicycle, Pedestrian, and/or Transit Standard Warrants:**

Warrants met: ☐ None ☐ Bicycle ☒ Pedestrian ☐ Transit

Sidewalks will be provided throughout the project.

**Is this a 3R (Resurfacing, Restoration, & Rehabilitation) Project?** ☒ No ☐ Yes

**Pavement Evaluation and Recommendations**

Initial Pavement Evaluation Summary Report Required?	<input type="checkbox"/> No	<input checked="" type="checkbox"/> Yes
Initial Pavement Type Selection Report Required?	<input type="checkbox"/> No	<input checked="" type="checkbox"/> Yes
Feasible Pavement Alternatives:	<input type="checkbox"/> HMA <input type="checkbox"/> PCC	<input checked="" type="checkbox"/> HMA & PCC

## DESIGN AND STRUCTURAL

**Description of the proposed project:** PI 0014131 is the widening and reconstruction of SR 20 in Cherokee County near Canton from east of Scott Rd to east of Union Hill Rd to six-lanes (three lanes in each direction) with a 20 foot raised median and urban shoulders. Access to side roads and driveways will be controlled by Restricted Crossing U-Turns (RCUTs) placed in the median; RCUT locations are shown in the layouts but may change based on preliminary design. Truck turnarounds are provided at certain RCUT locations based on consideration of adjacent facilities that may draw tractor trailers (factories, farms with chicken houses, landscaping or stone supply companies, etc). The project resides within an MS4 area and on/near a ridgeline, which places almost all drainage areas near receiving stream headwaters having less than 5 mi<sup>2</sup> of drainage areas. To satisfy the requirements of the downstream hydrologic assessment (See section 10.2.1.1 of the 2016 Drainage Manual) the project proposes to capture all pavement runoff through use of curb and gutter (urban shoulder) into a closed drainage system, which would pipe roadway runoff to permanent post-construction stormwater dry detention basins to treat for water quality as well as to detain and provide protection from downstream flooding. The total project length is about 2 miles. There are no bridges or other major structures.

This project begins where PI 0009164 ends with the Scott Rd intersection being constructed with PI 0009164; PI 0009164 is scheduled to be let to construction before this project.

County: Cherokee

**Mainline Design Features:**

Typical Section: 6-lane urban, 11 &amp; 12 ft wide travel lanes, 20' raised median, curb &amp; gutter

Feature	Existing	Policy	Proposed
<b>Typical Section:</b>			
- Number of Lanes	2		6
- Lane Width(s)	12 ft	11 ft-12 ft	11 ft (inside & middle) 12 ft (outside)
- Median Width & Type	N/A	Varies	20 ft Raised
- Border Area Width	N/A	10 ft - 16 ft	16 ft
- Outside Shoulder Slope	Varies	2%	2%
- Inside Shoulder Width	N/A	C&G	C&G
- Sidewalks	N/A	5 ft	5 ft
- Auxiliary Lanes	N/A		N/A
- Bike Accommodation	N/A	N/A	N/A
Posted Speed	45 mph		45 mph
Design Speed	Unknown	45 mph	45 mph
Minimum Horizontal Curve Radius	Unknown	711	711
Maximum Superelevation Rate	Unknown	4%	4%
Maximum Grade	Unknown	7%	7%
Access Control	Unknown		Permitted
Design Vehicle	Unknown		WB-67
Pavement Type	Asphalt		TBD

\*According to current GDOT design policy if applicable

**Major Interchanges/Intersections:** Union Hill Rd/Harmony Dr**Lighting required:** ☒ No ☐ Yes**Off-site Detours Anticipated:** ☒ No, for mainline ☒ Undetermined, sideroads ☐ Yes

**Transportation Management Plan [TMP] Required:** ☒ No ☐ Yes

If Yes: Project classified as: ☐ Non-Significant ☐ Significant

TMP Components Anticipated: ☐ TTC ☐ TO ☐ PI

Note: TMP is not required because project is state funded.

**Is the project located on a NHS roadway?** ☐ No ☒ Yes



County: Cherokee

**Design Exceptions/Design Variances to FHWA or GDOT Controlling Criteria anticipated:**

<b>FHWA or GDOT Controlling Criteria</b>	<b>No</b>	<b>Undeter- mined</b>	<b>Yes</b>	<b>DE or DV</b>	<b>Approval Date (if applicable)</b>
1. Design Speed	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>		
2. Design Loading Structural Capacity	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>		
3. Stopping Sight Distance	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>		
4. Horizontal Curve Radius	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>		
5. Maximum Grade	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>		
6. Vertical Clearance	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>		
7. Superelevation Rate	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>		
8. Lane Width	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>		
9. Cross Slope	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>		
10. Shoulder Width	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>		

**Design Variances to GDOT Standard Criteria anticipated:**

<b>GDOT Standard Criteria</b>	<b>Reviewing Office</b>	<b>No</b>	<b>Undeter- mined</b>	<b>Yes</b>	<b>Approval Date (if applicable)</b>
1. Access Control	DP&S	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	
2. Shoulder Width	DP&S	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	
3. Intersection Sight Distance	DP&S	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	
4. Intersection Skew Angle	DP&S	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	
5. Tangent Lengths on Reverse Curves	DP&S	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	
6. Lateral Offset to Obstruction	DP&S	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	
7. Rumble Strips	DP&S	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	
8. Safety Edge	DP&S	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	
9. Median Usage	DP&S	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	
10. Roundabout Illumination Levels	DP&S	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	
11. Complete Streets Warrants	DP&S	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	
12. ADA Requirements in PROWAG	DP&S	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	
13. GDOT Construction Standards	DP&S	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	
14. GDOT Drainage Manual	DP&S	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	
15. GDOT Bridge & Structural Manual	Bridges	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	

**VE Study anticipated:** ☐ No ☐ Yes ☒ Completed – Date: 3/2/2017

See attachments for VE Implementation Letter.

**UTILITY AND PROPERTY**

**Railroad Involvement:** No railroads are in the vicinity of the project.

**Utility Involvements:**

AGL – Natural Gas

Cherokee County – Water & Sewer

Comcast

Georgia Power – Distribution Power

Sunesys – Telecom

**SUE Required:** ☐ No ☒ Yes ☐ Undetermined

County: Cherokee

Public Interest Determination Policy and Procedure recommended? ☒ No ☐ Yes

**Right-of-Way (ROW):** Existing width: 80-150ft. Proposed width: 150+ft.  
 Required Right-of-Way anticipated: ☐ None ☒ Yes ☐ Undetermined  
 Easements anticipated: ☐ None ☒ Temporary ☒ Permanent ☒ Utility ☐ Other

Anticipated total number of impacted parcels:	<u>101</u>
Displacements anticipated:	Businesses: <u>10</u>
	Residences: <u>18</u>
	Other: <u>0</u>
Total Displacements:	<u>28</u>

Location and Design approval: ☐ Not Required ☒ RequiredImpacts to USACE property anticipated? ☒ No ☐ Yes ☐ UndeterminedIs Federal Aviation Administration (FAA) coordination anticipated? ☐ No ☒ Yes

Note: Project is within 5 miles of the Cherokee County Airport.

## ROUNDABOUTS

Per email from the Office of Traffic Operations received 8/30/16, roundabouts do not need to be considered on six-lane roadways (see Attachment 6).

## CONTEXT SENSITIVE SOLUTIONS

### Issues of Concern:

Potential impacts that may require context-sensitive solutions along this project corridor include the following:

- Historic properties
- Streams and wetlands
- Residences and businesses

Impacts to these resources will be minimized by techniques such as utilizing steeper slopes with guardrail, walls, and coordinating with the agencies for optimal design solutions. We have also reduced the lane width of four of the lanes to 11 feet from 12 feet.

In addition, meetings have been held with the City of Canton and Cherokee County to determine the appropriate design for this corridor. Five rounds of PIOHs have been held to understand the needs of the general public and to develop and present the current concept layout. We will incorporate design elements to meet these needs as appropriate.

### Context Sensitive Solutions Proposed:

Alignment shifts (e.g., widening to the north, south, and symmetrical) will be utilized to minimize impacts to historic properties, streams/wetlands, residences, and businesses. In addition, narrower shoulders, steeper slopes, and the use of retaining walls will be considered to further reduce the footprint and impacts of the proposed improvements. Due to the safety concerns along the corridor, restricted crossing u-turn medians are proposed at frequent intervals along the corridor, which allow for passenger car and tractor trailer turn arounds and reduce the number of conflict points for the vehicles as compared to a full access median. Access to all parcels will be maintained throughout construction.



County: Cherokee

## ENVIRONMENTAL & PERMITS

### Anticipated Environmental Document:

NEPA: ☐ PCE ☐ CE ☐ EA-FONSI ☐ EIS  
 GEPA\*: ☐ Type A ☐ Type B ☐ EER ☒ None

\*A GEPA document must be prepared only for state funded projects where the project cost meets or exceeds \$100 million.

### Level of Environmental Analysis:

- ☐ The environmental considerations noted below are based on preliminary desktop or screening level environmental analysis and are subject to revision after the completion of resource identification, delineation, and agency concurrence.
- ☒ The environmental considerations noted below are based on the completion of resource identification, delineation, and agency concurrence.

### Water Quality Requirements:

**MS4 Permit Compliance – Is the project located in a MS4 area?** ☐ No ☒ Yes

Post-construction stormwater management with permanent practices and structures put in place to reduce, treat, or minimize stormwater pollution from stabilized, developed areas, are being considered and will be incorporated in the plans as needed. There is no project level exclusion that applies to this project.

**Is Protected Species water quality mitigation anticipated?** ☒ Yes ☐ No

### Environmental Permits/Variations/Commitments/Coordination anticipated:

Permit/ Variance/ Commitment/ Coordination Anticipated	No	Yes	Remarks
1. U.S. Coast Guard Permit	X		
2. Forest Service/NPS	X		
3. CWA Section 404 Permit		X	404 Permit will be evaluated on a corridor basis.
4. Tennessee Valley Authority Permit	X		
5. 33 USC 408 Decision	X		
6. Buffer Variance		X	Buffer variance will be evaluated on a corridor basis.
7. Coastal Zone Management Coordination	X		
8. NPDES		X	
9. FEMA		X	FEMA coordination will be evaluated on a corridor basis.
10. Cemetery Permit	X		
11. Other Permits	X		
12. Other Commitments		X	Special Provisions for protection of bats and darters anticipated
13. Other Coordination	X		

**Is a PAR required?** ☐ No ☒ Yes ☐ Completed – Date:

The Screen 2 Memo is being converted into a PAR document by using supplemental information. This process is ongoing as of the writing of this report and is being coordinated with the IRT.

County: Cherokee

**Environmental Comments and Information:**

**NEPA/GEPA:** The project is being advanced under GEPA as a state funded project with the lead agency as the U.S. Army Corps of Engineers (USACE).

**Ecology:** The 2016 ecological field survey identified 24 features including 11 upland drainage features and 13 jurisdictional features, including 8 intermittent streams, 2 perennial streams, and 3 wetlands. Features are inclusive to each PI number. A 404 Permit and a Stream Buffer Variance will be required.

**History:** The 2015 SHPO concurred with Historic Resource Survey Report identified 10 National Register- eligible properties. SHPO concurrence was received in 2015. Home plots containing family cemeteries are scattered throughout the corridor, but would not be impacted by the project.

**Archeology:** The archaeology field work is underway and no National Register eligible sites have been identified to date within these limits.

**Air Quality:**

Is the project located in an Ozone Non-attainment area?

☐ No☒ Yes

Is a Carbon Monoxide hotspot analysis required?

☐ No☒ Yes

A Carbon Monoxide hotspot analysis is required for the project corridor as the corridor contains at least one traffic signal, design year traffic volumes exceed 10,000 vpd, and the level of service is D, E or F.

**Noise Effects:** No noise study is required for the corridor as it is a state funded project. Noise studies will be completed for National Register Eligible historic properties.

**Public Involvement:** Five Public Involvement Open Houses (PIOH) were held: PIOH #1 (Scoping Mtg) on May 16, 2013 and May 21, 2013; PIOH #2 on December 10, 2013 and December 12, 2013; PIOH #3 on September 15, 2015 and September 17, 2015; and PIOH #4 on December 6, 2016 and December 15, 2016; PIOH #5 on May 8 and 16, 2017. Each public meeting was held in Canton and Cumming for the convenience of attendees. In addition, a Citizen's Advisory Committee and a Technical Advisory Committee were formed early in the project development to inform the alternatives evaluation.

**Major stakeholders:** Major stakeholders include the traveling public (local users and cross-county users), homeowners, business associations located on SR 20 and in the vicinity of the roadway project, and agencies/stakeholders with interest in the resources located along the corridor.

**CONSTRUCTION****Issues potentially affecting constructability/construction schedule:**

Due to the presence of protected bats along the corridor, there may be clearing restrictions; however, this is an ongoing co-ordination issue with resource agencies that will be determined through the GEPA process.

Due to the width of the proposed improvements, we anticipate maintaining traffic on the current corridor while constructing the improvements. It will require multiple stages to widen and shift traffic through completion of all improvements.

**Early Completion Incentives recommended for consideration:** ☒ No

☐ Yes



County: Cherokee

## COORDINATION, ACTIVITIES, RESPONSIBILITIES, AND COSTS

**Initial Concept Meeting:** The initial concept meeting was held on March 5, 2013 (District 1) and March 6, 2013 (District 6); meeting minutes are attached.

**Concept Meeting:** The concept meeting was held on March 10, 2017; meeting minutes are attached.

**Other coordination to date:** See Public Involvement section.

Project Activity	Party Responsible for Performing Task(s)
Concept Development	AECOM
Design	AECOM
Right-of-Way Acquisition	GDOT
Utility Coordination (Preconstruction)	GDOT, AECOM
Utility Relocation (Construction)	Utility Owner, Contractor
Letting to Contract	GDOT
Construction Supervision	GDOT
Providing Material Pits	Contractor
Providing Detours	Contractor
Environmental Studies, Documents, & Permits	AECOM
Environmental Mitigation	GDOT
Construction Inspection & Materials Testing	GDOT

### Project Cost Estimate Summary and Funding Responsibilities:

	PE Activities		ROW	Reimbursable Utilities	CST*	Total Cost
	PE Funding	Section 404 Mitigation				
Funded By	GDOT	GDOT	GDOT	GDOT	GDOT	
\$ Amount	\$2,015,345**	\$99,072***	\$13,967,000	\$1,975,000	\$22,320,761	\$40,377,178
Date of Estimate	12/15/15	8/31/17	6/13/17	2/22/17	8/24/17	

\*CST Cost includes: Construction, Engineering and Inspection, Contingencies and Liquid AC Cost Adjustment.

\*\*Total PE funding for PI 0003681 (which includes PIs 0009164, 0014131, 0014132, 0014133, 0002862, and 0003682) is \$20,153,451. The funding for this project was estimated based on the percentage this project makes up of the entire corridor.

\*\*\*Total estimated mitigation cost (excluding buffer impacts) for the entire corridor (including PIs 0014131, 0014132, 0014133, 0002862, and 0003682) is \$931,280. The cost for this project was estimated based on the percentage this project makes up of the entire corridor.

County: Cherokee

## ALTERNATIVES DISCUSSION

### Alternative selection:

<b>Preferred Alternative:</b> The proposed alignment will generally follow the existing roadway from Scott Rd to Union Hill Rd. Corrections to the horizontal and vertical alignment along that section were made to meet the design criteria and to minimize impacts to residents, businesses, historic properties, streams, and wetlands.			
<b>Estimated Property Impacts:</b>	101 parcels, 28 displacements	<b>Estimated Total Cost:</b>	\$40,377,178
<b>Estimated ROW Cost:</b>	\$13,967,000	<b>Estimated CST Time:</b>	24 months
<b>Rationale:</b> This alternative was chosen because it meets the goals outlined in the project justification statement. It is the best-fit in terms of avoidance of displacements, streams, wetlands, and historic properties.			

<b>No-Build Alternative:</b> No improvements to SR 20.			
<b>Estimated Property Impacts:</b>	0 parcels, 0 displacements	<b>Estimated Total Cost:</b>	\$0
<b>Estimated ROW Cost:</b>	\$0	<b>Estimated CST Time:</b>	0 months
<b>Rationale:</b> This alternative fails to address the need and purpose of the project.			

**Alternative 1:** This alternative (shown as Conceptual Alternatives 3A and 3B in Attachment 10) would construct a new, limited access facility to the north or south of existing SR 20.

**Impacts:** See Attachment 10 for detailed cost and impact analysis.

**Rationale:** This alternative was evaluated in the Screen 2 analysis. This alternative is not recommended to advance for further evaluation, as it is almost twice as expensive as the preferred alternative.

**Alternative 2:** This alternative (shown as Conceptual Alternative 4 in Attachment 10) would go off the existing SR 20 and implement a localized bypass, tying back in to existing at the beginning and end of the project.

**Impacts:** See Attachment 10 for detailed cost and impact analysis.

**Rationale:** This alternative was evaluated in the Screen 2 analysis. This alternative would have a similar construction cost to the preferred alternative and similar impacts to residents, businesses, historic properties, streams and wetlands. At the PIOHs, we heard that the public would prefer to widen existing rather than impact the surrounding communities with bypasses. Therefore, with state funding for the project, widening existing was selected as the preferred alternative.

County: Cherokee

**LIST OF ATTACHMENTS/SUPPORTING DATA**

1. Concept Layout
2. Typical sections
3. Detailed Cost Estimates:
  - a. Construction including Engineering and Inspection and Contingencies
  - b. Completed Liquid AC Cost Adjustment forms
  - c. Right-of-Way
  - d. Utilities
  - e. Environmental Mitigation
4. Traffic study
5. Traffic diagrams
6. Roundabout Data
7. Minutes of Concept meetings
8. Minutes of any meetings that shows support or objection to the concept
9. Screen 2 Conceptual Alternatives
  - a. Map
  - b. Displacements
  - c. Costs
  - d. Comprehensive Matrix
10. VE Implementation Letter

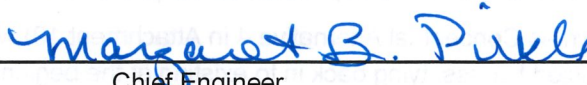
**APPROVALS**

Concur: \_\_\_\_\_



Director of Engineering

Approve: \_\_\_\_\_



Chief Engineer

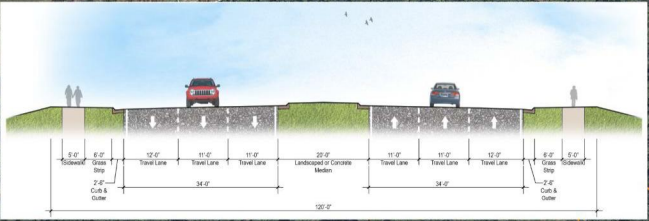
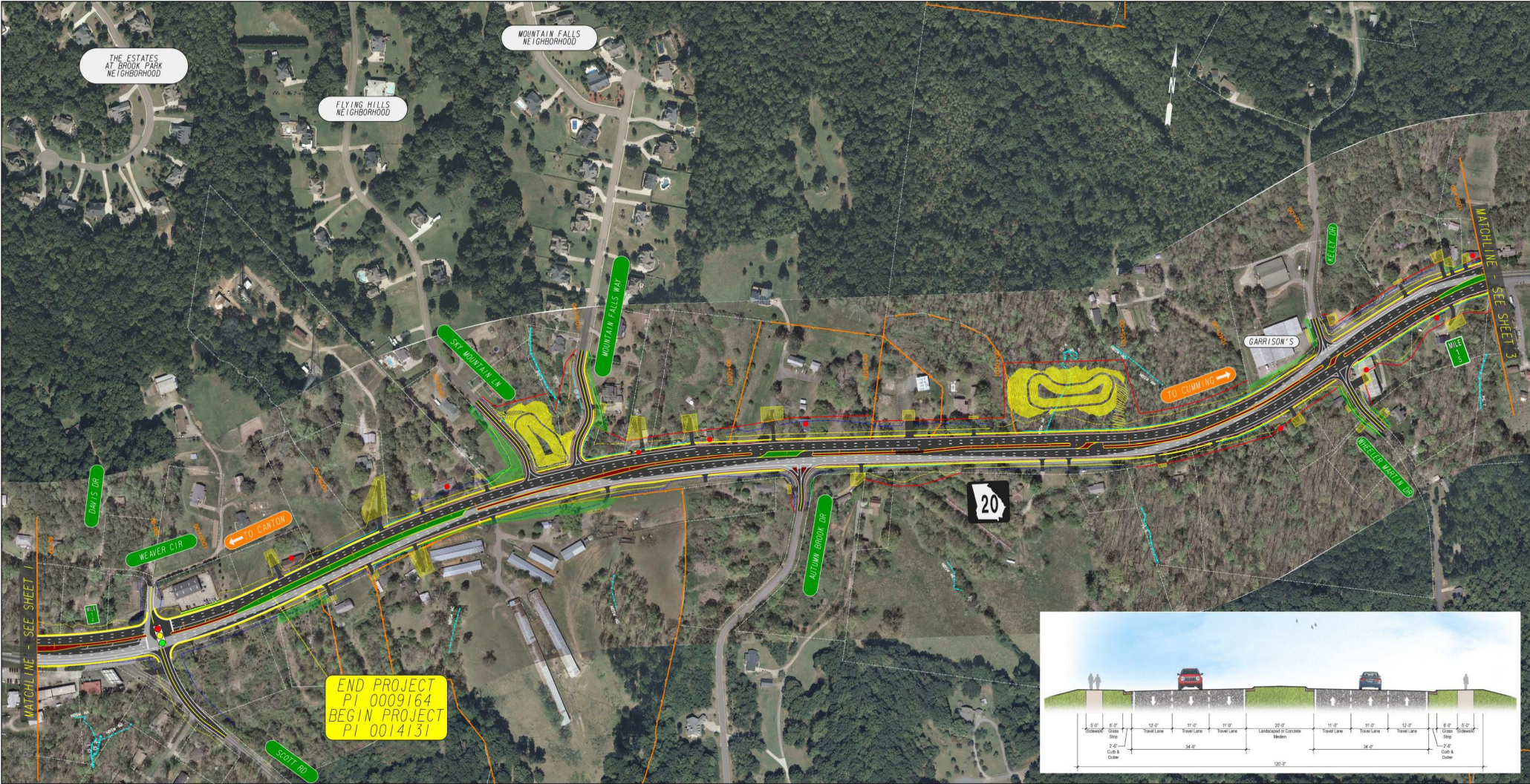


Date

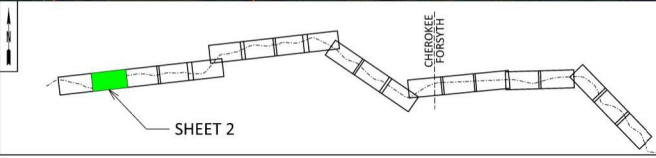
## **Attachment 1**

### **Concept Layout**





LEGEND					
	HISTORIC RESOURCES		LIMITS		GUARDRAIL
	STREAMS/WETLANDS		EXISTING PAVEMENT		CEMETERY
	REQUIRED RIGHT-OF-WAY (R/W)		PROPOSED PAVEMENT		SCHOOL
	DRIVEWAY EASEMENT		PAVED MEDIAN		CHURCH
	PERMANENT EASEMENT		GRASSED MEDIAN		EXISTING TRAFFIC SIGNAL
	POTENTIAL DISPLACEMENT		CURB AND GUTTER		POTENTIAL POND
	EXIST. PROPERTY & R/W		SIDEWALK		FLUSH MEDIAN
			WALL/BARRIER		

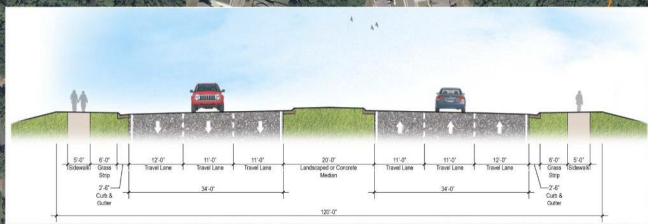


SR 20 - CORRIDOR IMPROVEMENTS  
PROPOSED ALIGNMENT

SCALE IN FEET  
0 100 200 400

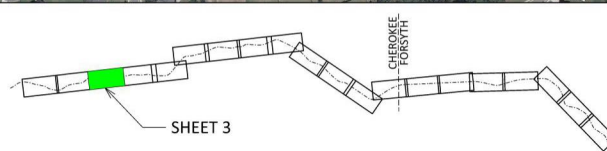
SHEET 2





## LEGEND

- |                             |                   |                         |
|-----------------------------|-------------------|-------------------------|
| HISTORIC RESOURCES          | LIMITS            | GUARDRAIL               |
| STREAMS/WETLANDS            | EXISTING PAVEMENT | CEMETERY                |
| REQUIRED RIGHT-OF-WAY (R/W) | PROPOSED PAVEMENT | SCHOOL                  |
| DRIVEWAY EASEMENT           | PAVED MEDIAN      | CHURCH                  |
| PERMANENT EASEMENT          | GRASSED MEDIAN    | EXISTING TRAFFIC SIGNAL |
| POTENTIAL DISPLACEMENT      | CURB AND GUTTER   | POTENTIAL POND          |
| EXIST. PROPERTY & R/W       | SIDEWALK          |                         |
|                             | WALL/BARRIER      |                         |



**Georgia Department of Transportation**

**AECOM**

**SR20**  
IMPROVEMENTS  
CANTON TO CUMMING

SR 20 - CORRIDOR IMPROVEMENTS  
PROPOSED ALIGNMENT

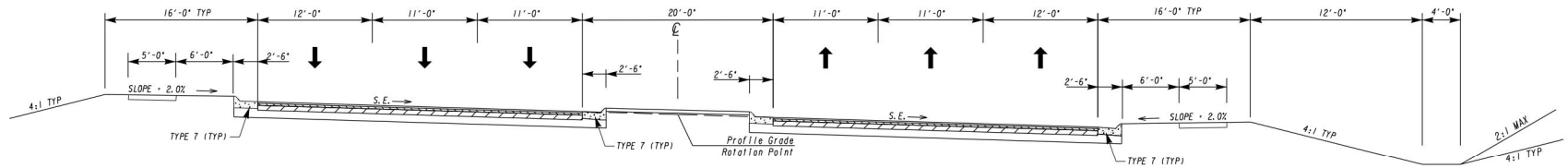
SCALE IN FEET  
0 100 200 400

SHEET 3

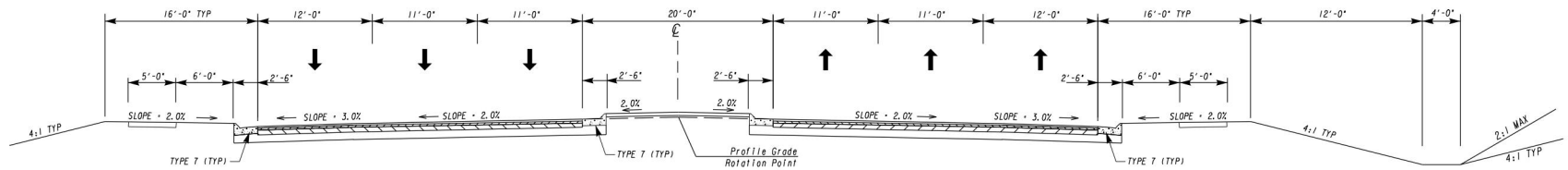


## **Attachment 2**

### **Typical Sections**



SUPERELEVATION  
6 LANE, 20' RAISED MEDIAN



NORMAL CROWN  
6 LANE, 20' RAISED MEDIAN

**AECOM**

NOT TO SCALE

REVISION DATES


TYPICAL SECTIONS

CHECKED:	DATE:	DRAWING No.
BACKCHECKED:	DATE:	
CORRECTED:	DATE:	
VERIFIED:	DATE:	

05-001

## **Attachment 3**

### **Detailed Cost Estimates**

# DEPARTMENT OF TRANSPORTATION STATE OF GEORGIA

## INTERDEPARTMENT CORRESPONDENCE

**FILE** P.I. No. **0014131** **OFFICE** Program Delivery

**PROJECT DESCRIPTION**  
SR 20 from CR 281/Scott Rd to CR 762/Union Hill Rd

**DATE** August 24, 2017

**From:** Albert V. Shelby, State Program Delivery Engineer

**To:** Lisa L. Myers, State Project Review Engineer  
via Email Mailbox: [CostEstimatesandUpdates@dot.ga.gov](mailto:CostEstimatesandUpdates@dot.ga.gov)

**Subject: REVISIONS TO PROGRAMMED COSTS**

**PROJECT MANAGER** Cleopatra James

**MGMT LET DATE** 7/15/2019

**MGMT ROW DATE** 7/17/2017

**PROGRAMMED COSTS (TPro W/OUT INFLATION)**

**LAST ESTIMATE UPDATE**

**CONSTRUCTION** \$ 13,230,000.00

**DATE** 9/2/2016

**RIGHT OF WAY** \$ 12,883,050.00

**DATE** 9/2/2016

**UTILITIES** \$

**DATE**

**REVISED COST ESTIMATES**

**CONSTRUCTION\*** \$ 22,320,761.26

**RIGHT OF WAY** \$ 13,967,000.00

**UTILITIES** \$ 1,975,000.00

\*Cost Contains **5** % Contingency

**REASONS FOR COST INCREASE AND CONTINGENCY JUSTIFICATION:**

The increase in construction costs was due to the previous estimate being based on 4 lanes instead of 6 lanes, rural shoulders instead of urban shoulders, open systems instead of closed drainage systems, and the addition of full depth paving and MS4 basins. A 5% contingency was added to the Construction estimate for risk. At the time of the last update, utilities information was not available and the current estimate is based on the best available information at the current stage. The ROW cost increase is based on a more thorough review of the current plans.

# CONTINGENCY SUMMARY

<b>A. CONSTRUCTION COST ESTIMATE:</b>	\$ 19,475,115.81	Base Estimate From CES	
<b>B. ENGINEERING AND INSPECTION (E &amp; I):</b>	\$ 973,755.79	Base Estimate (A) x	5 %
<b>C. CONTINGENCY:</b>	\$ 1,022,443.58	Base Estimate (A) + E & I (B) x	5 %
		<a href="#">See % Table in "Risk Based Cost Estimation" Memo</a>	
<b>D. TOTAL LIQUID AC ADJUSTMENT:</b>	\$ 849,446.08	Total From Liquid AC Spreadsheet	
<b>E. CONSTRUCTION TOTAL:</b>	\$ 22,320,761.26	(A + B + C + D = E)	

## REIMBURSABLE UTILITY COSTS

UTILITY OWNER	REIMBURSABLE COST
Southern Company (GPC Distribution)	\$ 1,975,000.00
<b>TOTAL</b>	\$ 1,975,000.00

### ATTACHMENTS: (File Copy in the Project Cost Estimate Folder)

Detailed Cost Estimate Printout  
Liquid AC Adjustment Spreadsheet

PROJ. NO. N/A  
P.I. NO. 0014131  
DATE 8/24/2017

CALL NO. 0/00/2016

INDEX (TYPE)	DATE	INDEX
REG. UNLEADED	Aug-17	\$ 2.185
DIESEL		\$ 2.457
LIQUID AC		\$ 361.00

Link to AC Index:  
<http://www.dot.ga.gov/PS/Materials/AsphaltFuelIndex>

#### LIQUID AC ADJUSTMENTS

PA=[((APM-APL)/APL)]xTMTxAPL

##### Asphalt

Price Adjustment (PA)					827368.68	\$	827,368.68
Monthly Asphalt Cement Price month placed (APM)	Max. Cap	60%	\$	577.60			
Monthly Asphalt Cement Price month project let (APL)			\$	361.00			
Total Monthly Tonnage of asphalt cement (TMT)					3819.8		

ASPHALT	Tons	%AC	AC ton
Leveling	2000	5.0%	100
12.5 OGFC		5.0%	0
12.5 mm	10435	5.0%	521.75
9.5 mm SP		5.0%	0
25 mm SP	50410	5.0%	2520.5
19 mm SP	13551	5.0%	677.55
	76396		3819.8

##### BITUMINOUS TACK COAT

Price Adjustment (PA)					\$ 22,077.40	\$	22,077.40
Monthly Asphalt Cement Price month placed (APM)	Max. Cap	60%	\$	577.60			
Monthly Asphalt Cement Price month project let (APL)			\$	361.00			
Total Monthly Tonnage of asphalt cement (TMT)					101.92704		

##### Bitum Tack

Gals	gals/ton	tons
23731	232.8234	101.92704

##### BITUMINOUS TACK COAT (surface treatment)

Price Adjustment (PA)					0	\$	-
Monthly Asphalt Cement Price month placed (APM)	Max. Cap	60%	\$	577.60			
Monthly Asphalt Cement Price month project let (APL)			\$	361.00			
Total Monthly Tonnage of asphalt cement (TMT)					0		

Bitum Tack	SY	Gals/SY	Gals	gals/ton	tons
Single Surf. Trmt.		0.20	0	232.8234	0
Double Surf. Trmt.		0.44	0	232.8234	0
Triple Surf. Trmt		0.71	0	232.8234	0

**TOTAL LIQUID AC ADJUSTMENT** \$ 849,446.08



DATE : 08/18/2017  
PAGE : 1

JOB ESTIMATE REPORT

JOB NUMBER : 0014131 SPEC YEAR: 13  
DESCRIPTION: SR 20 FROM CR 281/SCOTT RD TO CR 762/UNION HILL RD

ITEMS FOR JOB 0014131

LINE	ITEM	ALT	UNITS	DESCRIPTION	QUANTITY	PRICE	AMOUNT
0005	150-1000		LS	TRAFFIC CONTROL - 0014131	1.000	1500000.00	1500000.00
0010	150-5010		EA	TRAF CTRL,PORTABLE IMPACT ATTN	17.000	7788.52	132404.92
0015	153-1300		EA	FIELD ENGINEERS OFFICE TP 3	1.000	95413.10	95413.10
0019	201-1500		LS	CLEARING & GRUBBING - 0014131	1.000	1733600.00	1733600.00
0020	205-0001		CY	UNCLASS EXCAV	283754.000	6.51	1848084.13
0039	310-1101		TN	GR AGGR BASE CRS, INCL MATL	119099.000	21.75	2590928.48
0040	402-1812		TN	RECYL AC LEVELING,INC BM&HL	2000.000	80.00	160000.00
0045	402-3121		TN	RECYL AC 25MM SP,GP1/2,BM&HL	50410.000	80.00	4032800.00
0050	402-4510		TN	RECYL AC 12.5 MM SP,GP2ONLY,INC P-MBM&HL	10435.000	80.00	834800.00
0055	402-3190		TN	RECYL AC 19 MM SP,GP 1 OR 2 ,INC BM&HL	13551.000	80.00	1084080.00
0060	413-0750		GL	TACK COAT	23731.000	2.57	60988.67
0065	441-0016		SY	DRIVEWAY CONCRETE, 6 IN TK	892.000	39.93	35624.31
0070	441-0018		SY	DRIVEWAY CONCRETE, 8 IN TK	89.000	51.54	4587.63
0075	441-0104		SY	CONC SIDEWALK, 4 IN	2141.000	37.50	80306.32
0080	441-0740		SY	CONC MEDIAN, 4 IN	11107.000	23.11	256720.98
0085	441-4020		SY	CONC VALLEY GUTTER, 6 IN -	1288.000	39.36	50707.53
0090	441-6740		LF	CONC CURB & GUTTER/ 8X30 TP7 -	44389.000	12.78	567709.56
0095	620-0100		LF	TEMP BARRIER, METHOD NO. 1 -	19700.000	24.12	475202.61
0100	621-4060		LF	CONCRETE SIDE BARRIER, TY 6 -	100.000	240.00	24000.00
0105	621-4061		LF	CONCRETE SIDE BARRIER, TY 6A -	150.000	257.28	38592.00
0110	621-4062		LF	CONCRETE SIDE BARRIER, TY 6B -	150.000	240.00	36000.00
0115	621-4063		LF	CONCRETE SIDE BARRIER, TY 6C -	125.000	747.69	93461.25
0120	634-1200		EA	RIGHT OF WAY MARKERS -	246.000	108.11	26597.18
0125	641-1100		LF	GUARDRAIL, TP T -	88.000	69.50	6116.21
0130	641-1200		LF	GUARDRAIL, TP W -	7157.000	17.56	125722.08
0135	641-5001		EA	GUARDRAIL ANCHORAGE, TP 1 -	63.000	811.07	51097.62
0140	641-5020		EA	GUARDRL, ANCHOR, TP 12B,31 IN, FLR, E/A	4.000	2420.33	9681.32
0145	643-8200		LF	BARRIER FENCE (ORANGE), 4 FT -	5000.000	1.64	8213.00
0150	206-0002		CY	BORROW EXCAV, INCL MATL -	25000.000	7.25	181496.75
0155	441-0204		SY	PLAIN CONC DITCH PAVING, 4 IN -	2500.000	34.52	86320.48
0160	550-1180		LF	STM DR PIPE 18,H 1-10 -	12100.000	37.42	452839.48
0165	550-1240		LF	STM DR PIPE 24,H 1-10 -	638.000	53.76	34303.79
0170	550-1360		LF	STM DR PIPE 36,H 1-10 -	250.000	76.96	19241.51
0175	550-2180		LF	SIDE DR PIPE 18,H 1-10 -	5600.000	27.89	156207.41
0180	550-3418		EA	SAFETY END SECTION 18,SD,4:1 -	112.000	321.95	36059.40
0185	550-4218		EA	FLARED END SECT 18 IN, ST DR -	1.000	637.12	637.13
0190	550-4224		EA	FLARED END SECT 24 IN, ST DR -	4.000	737.03	2948.15
0195	550-4236		EA	FLARED END SECT 36 IN, ST DR -	2.000	1248.51	2497.03
0200	600-0001		CY	FLOWABLE FILL -	25.000	254.33	6358.27
0205	668-1100		EA	CATCH BASIN, GP 1 -	69.000	2442.07	168503.26
0210	668-1110		LF	CATCH BASIN, GP 1, ADDL DEPTH -	100.000	203.01	20301.68

DATE : 08/18/2017  
PAGE : 2

JOB ESTIMATE REPORT

0215	668-2100	EA	DROP INLET, GP 1 -	3.000	2156.67	6470.03
0220	668-2110	LF	DROP INLET, GP 1, ADDL DEPTH -	75.000	195.60	14670.03
0225	668-4300	EA	STORM SEW MANHOLE, TP 1 -	1.000	2194.01	2194.02
0230	999-3110	EA	DETENTION POND -	7.000	88000.00	616000.00
0235	163-0232	AC	TEMPORARY GRASSING -	18.000	698.42	12571.66
0240	163-0240	TN	MULCH -	676.000	173.61	117362.59
0245	163-0300	EA	CONSTRUCTION EXIT -	14.000	1351.37	18919.21
0250	163-0503	EA	CONSTR AND REMOVE SILT CONTROL GATE,TP 3 -	56.000	377.89	21162.10
0255	163-0520	LF	CONSTR AND REMOVE TEMP PIPE SLOPE DRAIN -	1500.000	13.84	20770.83
0260	163-0527	EA	CNST/REM RIP RAP CKDM,STN P RIPRAP/SN BG -	563.000	289.52	163002.94
0265	163-0531	EA	CONSTR & REM SEDIMENT BASIN,TP 1,STA NO- 7 LOCATIONS	7.000	13732.28	96126.01
0270	163-0550	EA	CONS & REM INLET SEDIMENT TRAP -	73.000	141.81	10352.84
0275	165-0030	LF	MAINT OF TEMP SILT FENCE, TP C -	14500.000	0.63	9212.00
0280	165-0041	LF	MAINT OF CHECK DAMS - ALL TYPES -	5630.000	2.18	12302.00
0285	165-0060	EA	MAINT OF TEMP SEDIMENT BASIN,STA NO - -	7.000	1361.34	9529.39
0290	165-0087	EA	MAINT OF SILT CONTROL GATE, TP 3 -	56.000	55.19	3091.11
0295	165-0101	EA	MAINT OF CONST EXIT -	14.000	586.35	8208.96
0300	165-0105	EA	MAINT OF INLET SEDIMENT TRAP -	73.000	53.23	3885.79
0305	167-1000	EA	WATER QUALITY MONITORING AND SAMPLING -	8.000	400.10	3200.82
0310	167-1500	MO	WATER QUALITY INSPECTIONS -	24.000	456.63	10959.23
0315	171-0030	LF	TEMPORARY SILT FENCE, TYPE C -	29000.000	3.20	93035.77
0320	603-2181	SY	STN DUMPED RIP RAP, TP 3, 18 -	180.000	47.83	8610.52
0325	603-7000	SY	PLASTIC FILTER FABRIC -	180.000	4.66	840.09
0330	700-6910	AC	PERMANENT GRASSING -	48.000	1039.82	49911.55
0335	700-7000	TN	AGRICULTURAL LIME -	138.000	106.07	14638.39
0340	700-8000	TN	FERTILIZER MIXED GRADE -	48.000	535.15	25687.27
0345	700-8100	LB	FERTILIZER NITROGEN CONTENT -	2400.000	2.18	5234.86
0350	710-9000	SY	PERM SOIL REINFORCING MAT -	2306.000	3.66	8439.96
0355	716-2000	SY	EROSION CONTROL MATS, SLOPES -	109000.000	0.81	88530.89
0360	636-1033	SF	HWY SIGNS, TP1MAT,REFL SH TP 9 -	230.000	19.59	4507.10
0365	636-1036	SF	HWY SGN,TP1MAT,REFL SH TP 11 -	1080.000	21.41	23122.80
0370	636-1072	SF	HWY SIGNS,ALUM EXTRD PNLS, RS TP 3 -	147.000	25.54	3755.27
0375	636-2070	LF	GALV STEEL POSTS, TP 7 -	730.000	7.68	5610.84
0380	636-2090	LF	GALV STEEL POSTS, TP 9 -	1478.000	6.28	9291.51
0385	636-3000	LB	GALV STEEL STR SHAPE POST -	977.000	5.27	5148.79
0390	639-2002	LF	STEEL WIRE STRAND CABLE, 3/8 -	107.000	11.11	1189.77
0395	639-4003	EA	STRAIN POLE, TP III -	4.000	7080.45	28321.81
0400	653-0110	EA	THERM PVMT MARK, ARROW, TP 1 -	3.000	69.63	208.92
0405	653-0120	EA	THERM PVMT MARK, ARROW, TP 2 -	67.000	75.69	5071.49
0410	653-0130	EA	THERM PVMT MARK, ARROW, TP 3 -	10.000	112.24	1122.45
0415	653-0170	EA	THERM PVMT MARK, ARROW, TP 7 -	12.000	106.62	1279.52
0420	653-0400	EA	THERM PVMT MARK, WORD, TP 21 -	3.000	316.00	948.00
0425	653-1501	LF	THERMO SOLID TRAF ST 5 IN, WHI -	30931.000	0.41	12779.14
0430	653-1502	LF	THERMO SOLID TRAF ST, 5 IN YEL -	20355.000	0.40	8294.87
0435	653-1704	LF	THERM SOLID TRAF STRIPE,24,WH -	48.000	8.36	401.76
0440	653-1804	LF	THERM SOLID TRAF STRIPE, 8,WH -	862.000	2.69	2320.05
0445	653-3501	GLF	THERMO SKIP TRAF ST, 5 IN, WHI -	41016.000	0.22	9112.11
0450	653-6004	SY	THERM TRAF STRIPING, WHITE -	1767.000	3.84	6793.21

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## JOB ESTIMATE REPORT

0455	653-6006	SY	THERM TRAF STRIPING, YELLOW -	173.000	4.34	751.63
0460	654-1001	EA	RAISED PVMT MARKERS TP 1 -	120.000	4.46	535.54
0465	654-1003	EA	RAISED PVMT MARKERS TP 3 -	627.000	4.25	2666.18
0470	647-1000	LS	TRAF SIGNAL INSTALLATION NO - UNION HILL ROAD	1.000	125000.00	125000.00
0475	626-0602	LF	TRAFFIC BARRIER, H -	390.000	150.00	58500.00
0480	627-1020	SF	MSE WALL FACE, 20 - 30 FT HT, WALL NO - -	5500.000	47.06	258860.64
0485	627-1030	SF	MSE WALL FACE, GTR 30 FT HT, WALL NO - -	6800.000	46.09	313448.31
-----						
ITEM TOTAL						19475115.81
INFLATED ITEM TOTAL						19475115.81
TOTALS FOR JOB 0014131						
-----						
ESTIMATED COST:						19475115.81
CONTINGENCY PERCENT ( 0.0 ):						0.00
ESTIMATED TOTAL:						19475115.81
-----						

GEORGIA DEPARTMENT OF TRANSPORTATION  
PRELIMINARY ROW COST ESTIMATE SUMMARY

Date: 6/13/2017  
Revised:

Project:  
County: Cherokee  
PI: 14131

Description: Widening of SR 20 from CR 281/Scott Rd to CR 762/Union Hill Rd  
Project Termini:

Parcels: 101  
Existing ROW: Varies  
Required ROW: Varies

Land and Improvements \$10,013,100.00

Proximity Damage \$30,000.00

Consequential Damage \$275,000.00

Cost to Cures \$100,000.00

Trade Fixtures \$150,000.00

Improvements \$1,522,400.00

Valuation Services \$442,500.00

Legal Services \$668,175.00

Relocation \$1,357,250.00

Demolition \$561,000.00

Administrative \$924,500.00

TOTAL ESTIMATED COSTS \$13,966,525.00

TOTAL ESTIMATED COSTS (ROUNDED) \$13,967,000.00

Preparation Credits	Hours	Signature
Jared E. Stiles	15	Jared E. Stiles

Prepared By:

Approved By:

Wesley K. Brock  
Wesley K. Brock

CG#: 5147

(DATE) 6/13/17

CG#: 6545

(DATE) 6/21/17 Revised 6/19/17

NOTE: No Market Appreciation is included in this Preliminary Cost Estimate

February 22, 2017

AECOM  
Mr. Scot Gero  
Project Manager  
1360 Peachtree Street, Suite 500  
Atlanta, GA 30309



RE: PI: 00014131-SR 20 Corridor Widening (Cherokee County)

Dear Mr. Gero:

Please find below the Preliminary Cost Estimate for each utility owner with facilities potentially located within the project limits:

Cardno

6649 Peachtree Industrial Blvd  
Suite I  
Peachtree Corners, GA 30092  
USA

Phone: +1 678 421 0080  
Fax: +1 770 421 0082

[www.cardno.com](http://www.cardno.com)

FACILITY OWNER	REIMBURSABLE	NON-REIMBURSABLE	TOTAL
Southern Company (GPC Distribution)	\$1,975,000.00	\$0.00	\$1,975,000.00
Sawnee EMC (SEMC)	\$0.00	\$0.00	\$0.00
Southern Company (AGL)	\$0.00	\$117,340.00	\$117,340.00
AT&T Telecommunications (ATT)	\$0.00	\$0.00	\$0.00
Crown Castle (Sunesys (SUN))	\$0.00	\$272,000.00	\$272,000.00
Windstream Communications (WST)	\$0.00	\$446,000.00	\$446,000.00
Comcast Communications/CATV	\$0.00	\$160,000.00	\$160,000.00
Cherokee County Water	\$0.00	\$181,110.00	\$181,110.00
Cherokee County Sewer	\$0.00	\$0.00	\$0.00
<b>TOTAL</b>	<b>\$1,975,000.00</b>	<b>\$1,176,450.00</b>	<b>\$3,151,450.00</b>

This estimate which was prepared by Venesia Horne, our Sr. Utility Coordinator, is based upon the current information and is preliminary. Cost are subject to change as plans and designs are developed further.

If you have any questions please feel free to call.

Sincerely,

Brandon Crawford  
Project Manager

Australia • Belgium • Indonesia • Kenya • New Zealand • Papua New Guinea  
United Arab Emirates • United Kingdom • United States • Operations in 60 countries

**Table 7: Cumulative impacts to field-delineated waters from I-575 to N Corners Pkwy along  
Alignment 2. Widen Existing within currently proposed construction limits**

Area of Design Influence	Feature	HUC	PI #	Length of impact (ft)	Area of impact (ac)
N/A	IS 1	03150104	0009164	95	
1	PS 17	03150104	0014132	49	
2	IS 37	03150104	0014132	11	
2	WL 38	03150104	0014132		0.001
3	PS 40	03150104	0014133	10	
4	PS 43	03150104	0014133	123	
4	PS 45	03150104	0014133	86	
5	WL 59	03150104	0002862		0.01
5	IS 60	03150104	0002862	209	
5	PS 62	03150104	0002862	143	
6	IS 70	03150104	0002862	72	
6	IS 76	03150104	0002862	135	
6	OW 75	03150104	0002862		0.013
7	PS 78	03130001	0002862	162	
7	PS 79	03130001	0002862	332	
7	IS 80	03130001	0002862	534	
8	IS 81	03130001	0002862	80	
9	IS 84	03150104	0002862	26	
10	IS 85	03150104	0002862	84	
10	WL 86	03150104	0002862		0.017
10	OW 87	03150104	0002862		0.054
11	IS 89	03130001	0003682	115	
12	IS 100	03130001	0003682	131	
12	PS 102	03130001	0003682	173	
12	PS 103	03130001	0003682	143	
12	IS 105	03130001	0003682	56	
12	IS 106	03130001	0003682	43	
12	PS 107	03130001	0003682	174	
12	PS 108	03130001	0003682	106	
12	PS 109	03130001	0003682	305	
12	WL 110	03130001	0003682		0.03
12	IS 111	03130001	0003682	146	
12	IS 112	03130001	0003682	191	
12	IS 113	03130001	0003682	85	
12	IS 114	03130001	0003682	80	
12	WL 115	03130001	0003682		0.005
13	PS 121	03130001	0003682	95	
TOTAL		03150104		1043	0.095
TOTAL		03130001		2951	0.035

# WORKSHEET 1: ADVERSE IMPACT FACTORS FOR RIVERINE SYSTEMS WORKSHEET

Stream Type Impacted	Intermittent 0.1			Perennial Stream > 15' in width 0.4			Perennial Stream ≤ 15' in width 0.8		
Priority Area	Tertiary 0.5			Secondary 0.8			Primary 1.5		
Existing Condition	Fully Impaired 0.25			Somewhat Impaired 0.5			Fully Functional 1.0		
Duration	Temporary 0.05			Recurrent 0.1			Permanent 0.2		
Dominant Impact	Shade/ Clear  0.05	Utility X-ing  0.4	Bank Armor  0.7	Deten- tion  1.5	Stream Crossing (≤ 100') 1.7	Impound  2.7	Morpho- logic Change 2.7	Pipe >100' 3.0	Fill  3.0
Scaling Factor (Based on # linear feet impacted)	< 100' impact  0	100-200' impact  0.05	201-500' impact  0.1	501- 1000' impact  0.2	> 1000' impact 0.4 for each 1000' feet of impact (round impacts to the nearest 1000') (example: 2,200' of impact – scaling factor = 0.8; 2,800' of impact – scaling factor = 1.2)				

Reaches to Be Impacted	Reach 1	Reach 2	Reach 3	Reach 4
<b>Complete the Following for Each Reach to Be Impacted</b>				
Simon Channel Evolution Stage				
Rosgen Stream Type/D50				
Criteria for Selecting Existing Condition for Each Reach				
Bankfull Width and Depth	Width: Depth:	Width: Depth:	Width: Depth:	Width: Depth:
Bankfull Indicators (attach photograph showing bankfull for each reach)				
Factors	Reach 1	Reach 2	Reach 3	Reach 4
Stream Type Impacted	0.1	0.8		
Priority Area	1.5	1.5		
Existing Condition	0.5	0.5		
Duration	0.2	0.2		
Dominant Impact	1.7	1.7		
Scaling Factor	0.8	0.8		
Sum of Factors M =	4.8	5.5		
Feet Stream in Reach Impacted LF =	1387.91	1553.47		
M X LF =	6662	8544		

**Total Mitigation Credits Required = (M X LF) = 17206**



# WETLANDS AND OPEN WATERS MITIGATION WORKSHEETS

## ADVERSE IMPACT FACTORS

Factor	Options						
Dominant Effect	Fill 2.0	Dredge 1.8	Impound 1.6	Drain 1.4	Flood 1.2	Clear 1.0	Shade 0.5
Duration of Effects	7+ years 2.0	5-7 years 1.5	3-5 years 1.0	1-3 years 0.5	< 1 year 0.1		
Existing Condition	Class 1 2.0	Class 2 1.5	Class 3 1.0	Class 4 0.5	Class 5 0.1		
Lost Kind	Kind A 2.0	Kind B 1.5	Kind C 1.0	Kind D 0.5	Kind E 0.1		
Preventability	High 2.0	Moderate 1.0	Low 0.5	None 0			
Rarity Ranking	Rare 2.0	Uncommon 0.5	Common 0.1				

† These factors are determined on a case-by-case basis.

## REQUIRED MITIGATION CREDITS WORKSHEET

Factor	Area 1	Area 2	Area 3	Area 4	Area 5	Area 6
Dominant Effect	2.0					
Duration of Effect	2.0					
Existing Condition	1.0					
Lost Kind	1.5					
Preventability	1.0					
Rarity Ranking	0.1					
Sum of r Factors	$R_1 = 7.6$	$R_2 =$	$R_3 =$	$R_4 =$	$R_5 =$	$R_6 =$
Impacted Area	$AA_1 = 0.029$	$AA_2 =$	$AA_3 =$	$AA_4 =$	$AA_5 =$	$AA_6 =$
$R \times AA =$	0.22					

**Total Required Credits =  $\Sigma (R \times AA) =$**

0.22

# WORKSHEET 1: ADVERSE IMPACT FACTORS FOR RIVERINE SYSTEMS WORKSHEET

Stream Type Impacted	Intermittent 0.1			Perennial Stream > 15' in width 0.4			Perennial Stream ≤ 15' in width 0.8		
Priority Area	Tertiary 0.5			Secondary 0.8			Primary 1.5		
Existing Condition	Fully Impaired 0.25			Somewhat Impaired 0.5			Fully Functional 1.0		
Duration	Temporary 0.05			Recurrent 0.1			Permanent 0.2		
Dominant Impact	Shade/ Clear  0.05	Utility X-ing  0.4	Bank Armor  0.7	Deten- tion  1.5	Stream Crossing (≤ 100') 1.7	Impound  2.7	Morpho- logic Change 2.7	Pipe >100' 3.0	Fill  3.0
Scaling Factor (Based on # linear feet impacted)	< 100' impact  0	100-200' impact  0.05	201-500' impact  0.1	501- 1000' impact  0.2	> 1000' impact 0.4 for each 1000' feet of impact (round impacts to the nearest 1000') (example: 2,200' of impact – scaling factor = 0.8; 2,800' of impact – scaling factor = 1.2)				

Reaches to Be Impacted	Reach 1	Reach 2	Reach 3	Reach 4
<b>Complete the Following for Each Reach to Be Impacted</b>				
Simon Channel Evolution Stage				
Rosgen Stream Type/D50				
Criteria for Selecting Existing Condition for Each Reach				
Bankfull Width and Depth	Width: Depth:	Width: Depth:	Width: Depth:	Width: Depth:
Bankfull Indicators (attach photograph showing bankfull for each reach)				
Factors	Reach 1	Reach 2	Reach 3	Reach 4
Stream Type Impacted	0.1	0.8	0.4	
Priority Area	1.5	1.5	1.5	
Existing Condition	0.5	0.5	0.5	
Duration	0.2	0.2	0.2	
Dominant Impact	1.7	1.7	1.7	
Scaling Factor	0.8	0.8	0.8	
Sum of Factors M =	4.8	5.5	5.1	
Feet Stream in Reach Impacted LF =	665.20	451.71	89.73	
M X LF =	3193	2484	458	

**Total Mitigation Credits Required = (M X LF) = 6135**

# WETLANDS AND OPEN WATERS MITIGATION WORKSHEETS

## ADVERSE IMPACT FACTORS

Factor	Options						
Dominant Effect	Fill 2.0	Dredge 1.8	Impound 1.6	Drain 1.4	Flood 1.2	Clear 1.0	Shade 0.5
Duration of Effects	7+ years 2.0	5-7 years 1.5	3-5 years 1.0	1-3 years 0.5	< 1 year 0.1		
Existing Condition	Class 1 2.0	Class 2 1.5	Class 3 1.0	Class 4 0.5	Class 5 0.1		
Lost Kind	Kind A 2.0	Kind B 1.5	Kind C 1.0	Kind D 0.5	Kind E 0.1		
Preventability	High 2.0	Moderate 1.0	Low 0.5	None 0			
Rarity Ranking	Rare 2.0	Uncommon 0.5	Common 0.1				

† These factors are determined on a case-by-case basis.

## REQUIRED MITIGATION CREDITS WORKSHEET

Factor	Area 1	Area 2	Area 3	Area 4	Area 5	Area 6
Dominant Effect	2.0	2.0				
Duration of Effect	2.0	2.0				
Existing Condition	1.0	0.5				
Lost Kind	1.5	0.5				
Preventability	1.0	1.0				
Rarity Ranking	0.1	0.1				
Sum of r Factors	$R_1 = 7.6$	$R_2 = 6.6$	$R_3 =$	$R_4 =$	$R_5 =$	$R_6 =$
Impacted Area	$AA_1 = 0.13$	$AA_2 = 0.07$	$AA_3 =$	$AA_4 =$	$AA_5 =$	$AA_6 =$
$R \times AA =$	0.99	0.46				

**Total Required Credits =  $\Sigma (R \times AA) =$**

1.45

Dunnahoo, Lindsey

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From: Crosby, John  
Sent: Wednesday, February 01, 2017 12:37 PM  
To: Dawood, Laura  
Cc: Covington, Christopher  
Subject: FW: Mitigation credits for SR 20  
Attachments: Stream worksheet.pdf; Wetland Worksheet.pdf

Follow Up Flag: Follow up  
Flag Status: Flagged

I just received a phone call from MRG. Wetland credits at their bank are permanently set at \$50,000. Wetland credits will equal \$84,000. Please let me know if you have any questions.

Thank you,

John Crosby  
Scientist II  
D: 864.234.3000 M: 404.275.8898  
[john.crosby@aecom.com](mailto:john.crosby@aecom.com)

AECOM  
10 Patewood Drive, Building VI, Suite 500, Greenville, South Carolina, 29615  
F 864.234.3069  
[www.aecom.com](http://www.aecom.com)

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From: Crosby, John  
Sent: Monday, January 30, 2017 1:51 PM  
To: Dawood, Laura; Covington, Christopher  
Cc: Wolfe, Kevin; Smith, William F  
Subject: Mitigation credits for SR 20

Good afternoon,

I have attached the results of the mitigation calculation. The only wetland credits I have found within the service area are at the Etowah River Road bank (MRG bank 404-308-0662). No one answered the phone but I left a voice mail about the prices. We will need 1.68 wetland credits based on the shapefiles that I have. Stream credits will require 21,182 credits at 40 dollars a credit (\$847,280). This was at Bannister Creek Mitigation Bank from Corblu. I spoke with Greg and he said it is likely that they would be available in 2019. They haven't sold many. Prices may change over time. Once the project gets in more of a final stage we can calculate impacts on a case by case scenario and that would minimize the credits. Please let me know if you have any questions. I will email again if MRG calls back.

Thank you,

John Crosby  
Scientist II  
D: 864.234.3000 M: 404.275.8898  
[john.crosby@aecom.com](mailto:john.crosby@aecom.com)

## **Attachment 4**

### **Traffic Study**

## 4. PI 0014131 Traffic Study

Section 4 analyses existing and future traffic conditions for the PI 0014131 project corridor: consisting of SR-20 from Scott Road to Union Hill Road.

### 4.1 Existing Conditions

Section 4.1 describes character of intersections, existing traffic volumes, and current crash statistics along the PI 0014131 corridor.

#### 4.1.1 Existing Transportation Facilities

Section 4.1.1 provides an overview of the existing major intersections along the project corridor. Most of the intersecting roadways in this project corridor are small neighborhood roads, with some limited business access and one school access point.

##### 4.1.1.1 Union Hill Road

Union Hill Road is the second signalized intersection in PI 0014131, and also serves as the project endpoint. SR-20 currently expands to two lanes in each direction with a separated right, thru and left turn at this intersection. The current posted speed limit on Union Hill Road is 45 mph. There are no bicycle lanes on either of these roadways. Although Buffington Elementary School is located along SR-20 in this area, there is no school zone or speed limit reduction. Union Hill Road intersection provides connectivity from SR-20 to Evans Cook Road, Henson Way, and East Cherokee Drive which travels northeast-southwest. Union Hill Road intersection is located in an area expected to develop with various residential and some village-style commercial uses.

#### 4.1.2 Existing Traffic Volumes

Existing traffic count data was collected by GCA, Inc. for GDOT under a separate contract in October 2011. Twenty-four hour traffic counts were collected at 30 points along the corridor. Vehicle classifications and peak turning count movements were collected at one location: Scott Road. Vehicle classification counts determine the relative proportions of cars, single-unit trucks and buses, and multi-unit or combination trucks utilizing the project corridor.

Plotted count locations provided by GCA, Inc. can be found in Appendix A. The existing traffic was utilized by GCA, Inc. to calculate K and D factors, truck percentages, and traffic growth rates as described in Section 4.2.1 of this report and Appendix B.

#### 4.1.3 Corridor Safety Analysis

Safety is one of the most important aspects of any functioning corridor. This section describes data collection and analysis of crash data for the project corridor.

##### 4.1.3.1 Crash Incidents

Total project corridor crash data was collected from the Georgia Electronic Accident Reporting System (GEARS)<sup>1</sup>. Crashes occurring between 2013 and 2015 were collected. County-level data was plotted using provided geographic coordinates, allowing for selection of project corridor incidents. Additional review of county-level crash data by street name ensured incidents

<sup>1</sup> Georgia Electronic Accident Reporting System (GEARS). Law enforcement reporting of traffic incidents in Georgia. Developed and maintained by Lexis Nexis on behalf of the Georgia Department of Transportation. <https://www.gearsportal.com/Pages/Public/Home.aspx>



along the corridor with incorrect or missing coordinate information were also included. The raw counts were parsed by injuries and/or fatalities and crash rates compared to statewide averages. The resulting crash incident summary for the project corridor is presented in Table 1.

**Table 1. Crash incidents SR-20 between Scott Rd and Union Hill Rd**

Year	Total			Injury			Fatal		
	# of crashes	Crashes per 100 MVM	State avg Crashes per 100 MVM	# of crashes	Crashes per 100 MVM	State avg Crashes per 100 MVM	# of crashes	Crashes per 100 MVM	State avg Crashes per 100 MVM
2013	46	283	686	10	62	142	1	6.15	1.29
2014	24	145	516	6	36	104	0	0.00	0.93
2015	36	213	n/a	4	24	n/a	0	0.00	n/a

Source: GCA, Inc. analysis of GEARS data

The total rate of crashes in the project corridor is lower than the rate statewide across all years. Injury crash rates are also lower than statewide averages across all years; however, the one fatal crash in the corridor creates a fatal crash rate per 100 MVM roughly five times the statewide average. The roadway improvements proposed by this project include a raised median, full median opening access only at signalized intersections, restricted median crossing U-turn access at moderately used un-signalized intersections, and indirect left access at low usage side streets and driveways. These improvements are expected to improve safety by reducing conflict points throughout the corridor.

## 4.2 Future Conditions

Section 4.2 describes the future traffic expected on the corridor, proposed design, and analysis of future corridor capacity after implementation of the proposed project along PI 0014131.

### 4.2.1 Traffic Forecast

Future traffic volumes were estimated by GCA, Inc. approved by GDOT and utilized by AECOM for a corridor capacity analysis.

#### 4.2.1.1 Growth Rate Methodology

GCA Inc. estimated growth rates in April 2012 for the project corridor which were approved by GDOT. Linear regression analysis was performed by GCA, Inc. using the historical traffic count data. Using the equations, future year traffic volumes were generated and growth rates were calculated. The growth rates estimated by two of these sets of data are summarized below.

**Table 2. PI 0014131 GDOT Approved Growth Rates**

Scenario	Date Range	Growth Rate (%)
No Build	2011-2025	2.0
No Build	2025-2045	1.7
Build	2011-2025	3.4
Build	2025-2045	2.9

Source: GCA, Inc. Memorandum, Appendix B

The analysis by GCA, Inc. and approved by GDOT which generated these growth estimates is included in Appendix B.

#### 4.2.1.2 Forecasted Volumes

The GDOT approved growth rates were applied to traffic in the opening and design years. This analysis estimates that the corridor will serve 31,900 vehicles per day by opening year 2025 and 56,900 vehicles per day in design year 2045.

All final projected volumes for the 2025 opening and 2045 design years are provided on traffic diagrams in Appendix C. These traffic volume diagrams were approved by GDOT, and include 2011 Existing Average Daily Traffic (ADT); 2011 Existing AM and PM Design Hour Volume (DHV); 2025 & 2045 ADT; 2025 AM and PM DHV; and 2045 AM and PM DHV.

#### 4.2.1.3 Signal Warrants

Major intersections along the project corridor were assessed for new signalization using forecasted average daily traffic for 2025 Opening year Build Scenario. As shown in Table 3, none of the PI 0014131 corridor intersections are projected to require signals.

**Table 3. PI 0014131 Signal Warrants, 2025 Opening Year Build Scenario**

Intersection with SR-20	2025 Opening Year Average Daily Traffic				GDOT Warrant 1A			GDOT Warrant 1B			New Signal Warranted?
	ADT Major Street (two way)	ADT Minor Street (one way)	5.6% of Major Street (two way)	5.6% of Minor Street (one way)	100%	70%	56%	100%	70%	56%	
Autumn Brook Dr	31,225	175	1,749	10	NO	NO	NO	NO	NO	NO	NO
Forest Creek Dr	29,525	975	1,654	55	NO	NO	NO	NO	YES	NO	NO
Old Cumming Dr (East)	28,225	375	1,581	21	NO	NO	NO	NO	NO	NO	NO
Union Hill Rd/Harmony Rd	25,200	1,975	1,412	111	NO	NO	NO	YES	YES	YES	NO*

Source: GCA, Inc. Analysis,

\*This location has an existing signal and does not need a new permit to be issued.

#### 4.2.2 Proposed Design

Previous alternatives analyses resulted in the decision to enhance the east-west mobility and safety of travelers in Cherokee and Forsyth Counties by improving SR-20. The project proposes a total of six lanes, with three travel lanes in each direction, separated by a raised median. The right of way required would range between 120 and 250 ft. This design of lanes and non-



signalized roadway access points has been utilized in the Capacity Analysis, and is described in more detail in Section 4.2.3.2.

### 4.2.3 Capacity Analysis

This analysis allows comparison of future traffic conditions associated with the proposed roadway design.

#### 4.2.3.1 Background

The 2010 HCM defines Level of Service (LOS) in terms of average control delay per vehicle, which is composed of initial deceleration delay, queue move-up time, stopped delay, and final acceleration delay. LOS A indicates operations with very low control delay, while LOS F describes operations with extremely high average control delay. Several factors affect the controlled delay for un-signalized intersections, such as availability and distribution of gaps in the conflicting traffic stream, critical gaps, and follow-up time for a vehicle in the queue. LOS in concept is visualized in Figure 5, and the various HCM LOS criteria are summarized in Table 4.



Figure 5. Level of Service (LOS) visualization, FDOT

**Table 4. Level of Service Criteria**

Level of Service	Average Control Delay (seconds per vehicle)	
	Signalized Intersection	Un-signalized Intersection
A	$\leq 10.0$	$\leq 10.0$
B	$> 10.0$ and $\leq 20.0$	$> 10.0$ and $\leq 15.0$
C	$> 20.0$ and $\leq 35.0$	$> 15.0$ and $\leq 25.0$
D	$> 35.0$ and $\leq 55.0$	$> 25.0$ and $\leq 35.0$
E	$> 55.0$ and $\leq 80.0$	$> 35.0$ and $\leq 50.0$
F	$> 80.0$	$> 50.0$

Source: 2010 Highway Capacity Manual

#### 4.2.3.2 Synchro Model Parameters

AECOM utilized Syncho 9.0 software for the project corridor capacity analysis. Syncho uses HCM methodology to model traffic along a corridor and then assigns LOS values to corridor intersections. The current roadway physical design was utilized for the 2011 Existing year model. The proposed design of a total of six lanes, with three travel lanes in each direction, separated by a raised median was applied for the 2025 Opening and 2045 Design year models.

Innovative intersection improvements were applied throughout the corridor. These improvements included numerous Restricted Crossing U-Turns (RCUT) as well as Median U-Turn Intersections (MUT) to improve safety.

Currently existing timing plans, typically running free, were utilized in the 2011 Existing year Synchro model. Signal timing was optimized at a 150 second cycle for the 2025 Opening year and 2045 Design year models. Splits were optimized in these plans.

Due to the limited turning movement counts collected in the project corridor, AECOM determined that peak hour factors should be estimated using all count locations, averaged and then applied throughout the entire project corridor from Scott Road to North Corners Parkway. These peak hour factors were calculated for left, right and thru movements on both the mainline and side streets as shown in Table 5, then utilized in Synchro. The data from which these factors were calculated can be found in Appendix D.

**Table 5. Peak Hour Factors utilized in Synchro Capacity Analysis**

Movement	Average Peak Hour Factor	
	AM	PM
Mainline Thru	0.86	0.91
Mainline Left	0.66	0.70
Mainline Right	0.65	0.76
Sidetreect Thru	0.57	0.67
Sidestreet Left	0.69	0.74
Sidestreet Right	0.68	0.68

*Source: AECOM Analysis, Appendix D*

Truck percentages calculated by GCA, Inc. were utilized for each corridor by project number. For PI 0014131, existing truck 24-hour truck percentage was approximately six percent: with four percent single-unit trucks and two percent of tractor trailers. For PI 0014131, average peak hour truck percentage of four percent: with three percent of single-unit trucks and one percent of tractor trailers. The following truck percentages were used in 2025 Opening and 2045 Design year models.

*24-hour Truck volumes = 16%, Single-Unit = 10%, Combination = 6%*  
*Peak hour Truck volumes = 12.5%, Single-Unit = 7.5%, Combination = 5%*

A more detailed explanation of the GCA, Inc. analysis resulting in these percentages is included in Appendix B.

#### 4.2.3.3 Capacity Analysis Results

Section 4.2.3.3 provides a summary of the capacity analysis results in terms of intersection level of service and intersection time delay.

**Table 6. PI 0014131 AM Peak Hour Capacity Analysis by Intersection: Existing 2011, and Opening Year 2025, Design Year 2045**

Intersection with SR-20	Control	No Build AM 2011 Existing Year		No Build AM 2025 Opening Year		Build AM 2025 Opening Year		No Build AM 2045 Design Year		Build AM 2045 Design Year	
		LOS	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)
Sky Mountain Lane†	Stop, SB	D	27.5	E	39.0	C	18.7	F	138.2	F	69.3
Mountain Falls Way†	Stop, SB	D	28.0	E	47.4	C	17.9	F	174.6	E	49.5
Autumn Brook Drive	Stop, NB	D	33.6	E	48.1	B	14.2	F	228.7	D	25.6
Roper Trail	Stop, NB	D	28.3	E	46.2	B	14.1	F	169.6	C	24.4
Kelly Drive†	Stop, NB	F	78.1	F	>300.0	B	14.3	F	>300.0	D	28.0
	Stop, SB†	F	104.1	F	>300.0	C	17.7	F	>300.0	F	50.7
Lawson Road	Stop, NB	D	28.9	E	46.6	B	13.9	F	169.6	C	23.5
Forest Creek Drive†	Stop, NB	F	107.6	F	>300.0	C	19.6	F	>300.0	F	239.1
Old Cumming Drive (West)†	Stop, SB	C	21.5	D	32.3	C	19.2	F	103.5	F	88.9
Bagwell Lane	Stop, NB	D	25.8	E	40.5	C	15.3	F	120.2	D	31.2
Old Cumming Drive (East)†	Stop, SB	C	24.6	E	39.7	C	16.2	F	125.6	E	37.6
Cox Ct	Stop, NB	D	25.9	E	40.5	B	13.8	F	113.6	C	23.2
Buffington Elementary School†	Stop, NB	E	41.0	F	176.6	C	19.9	F	>300.0	F	270.3
Dobson Circle†	Stop, SB	D	28.8	E	48.1	C	17.8	F	>300.0	F	58.9
Union Hill Road/Harmony Drive	Signal	C	24.3	D	52.1	C	20.9	F	144.9	D	38.0

Source: 2000 Highway Capacity Manual, GCA, Inc. Analysis, \*HCS 2000

As Table 6 shows, the PI 0014131 Build scenario is expected to provide a higher level of service and less delay than the No-Build scenario in both opening and design years.



**Table 7. PI 0014131 PM Peak Hour Capacity Analysis by Intersection: Existing 2011, and Opening Year 2025, Design Year 2045**

Intersection with SR-20	Control	No Build PM 2011 Existing Year		No Build PM 2025 Opening Year		Build PM 2025 Opening Year		No Build PM 2045 Design Year		Build PM 2045 Design Year	
		LOS	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)
Sky Mountain Lane	Stop, SB	F	50.9	F	96.3	C	17.7	F	>300.0	E	45.7
Mountain Falls Way†	Stop, SB	E	42.3	F	84.5	C	18.2	F	>300.0	F	55.8
Autumn Brook Drive†	Stop, NB	E	47.4	F	150.2	C	19.6	F	>300.0	F	63.8
Roper Trail†	Stop, NB	E	49.7	F	107.6	C	19.4	F	>300.0	F	54.8
Kelly Drive†	Stop, NB	F	140.5	F	>300.0	C	19.3	F	>300.0	F	65.7
	Stop, SB	F	140.5	F	>300.0	C	17.7	F	>300.0	E	48.4
Lawson Road†	Stop, NB	E	47.1	F	159.4	C	19.0	F	>300.0	F	56.5
Forest Creek Drive†	Stop, NB	F	131.9	F	>300.0	C	22.1	F	>300.0	F	235.1
Old Cumming Drive (West)	Stop, SB	E	43.6	F	169.6	C	19.2	F	56.1	C	22.4
Bagwell Lane†	Stop, NB	E	37.6	F	68.6	C	18.5	F	>300.0	E	47.3
Old Cumming Drive (East)†	Stop, SB	D	33.1	F	70.2	C	18.7	F	>300.0	F	110.0
Cox Ct†	Stop, NB	E	35.9	F	63.5	C	16.9	F	>300.0	E	41.3
Buffington Elementary School†	Stop, NB	F	125.3	F	>300.0	D	27.9	F	>300.0	F	>300.0
Dobson Circle†	Stop, SB	D	29.8	F	53.0	C	17.3	E	45.6	E	47.3
Union Hill Road/Harmony Drive	Signal	C	28.4	E	60.5	C	27.0	F	206.2	D	48.5

Source: 2000 Highway Capacity Manual, GCA, Inc. Analysis, \*HCS 2000

As Table 7 shows, the PI 0014131 Build scenario is expected to provide a higher level of service and less delay than the No-Build scenario in both opening and design years except in two cases: Buffington Elementary School and Dobson Circle. Buffington Elementary School driveway is expected to perform equally poorly in the design year No-Build and Build scenarios. Dobson Circle is expected to perform with 1.7 seconds more delay in the design year Build scenario than the No-Build scenario.

All intersections except Union Hill Road across the AM or PM periods experience LOS E or LOS F in the design year. These intersections are marked with a dagger (†) in Tables 6 and 7. Two main engineering mitigation actions were considered to improve these results: signals and additional turn lanes.

#### 4.2.3.4 Mitigation Actions

The potential for a new signal at failing intersections was assessed at a planning level using standards set by the U.S. Department of Transportation Federal Highway Administration Manual on Uniform Traffic Control Devices (MUTCD). No intersection in PI 0014131 with expected poor LOS meets these planning-level signal warrants.

Although additional turn lanes were considered for the other intersections predicted to perform poorly, we do not consider these values to be highly concerning to the project. Highway Capacity Software used in this analysis assumes consistent headways between vehicles, whereas in the real world vehicle tend to travel in platoons. These platoons enable vehicles on side roads like many of these poor LOS intersections to enter traffic during the gaps between platoons. Since these gaps are not accounted for in the software, it incorrectly assumes that these vehicles will be severely delayed, when in fact they may not wait as long in the real world.

### 4.3 Conclusions

This study focused on using existing and future traffic data to determine the appropriate transportation improvement to provide a safe and efficient transportation corridor. Existing facilities and future planned projects are aligned with widening capacity on SR-20 from Scott Road to Union Hill Road. Our review of crash data for the project corridor does not prohibit widening the road; indeed there is some evidence where congestion is causing additional safety concerns. Two scenarios were considered: No-Build and Build in existing 2011, opening 2025 and design 2045 years.

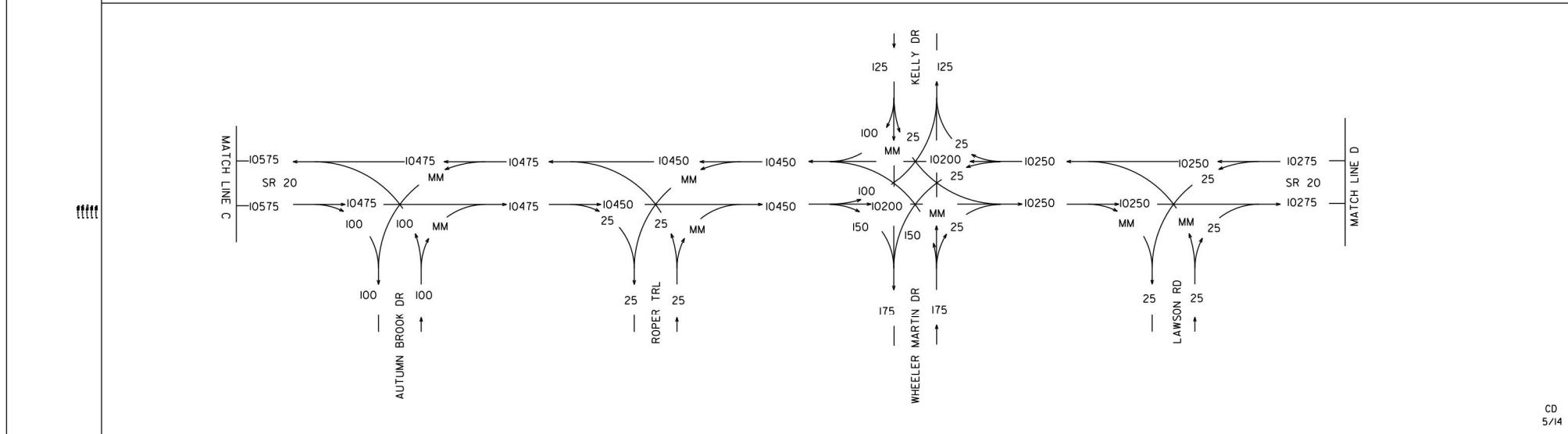
GDOT approved traffic forecasting methods were used to conduct a lane call capacity analysis, which indicated that six lanes are needed on the corridor by 2025. The results of an intersection capacity analysis for the entire corridor provide further support for widening; this alternative provides the highest number of intersections providing level of service of D or better in every year and time period (AM or PM) studied. Although some intersections along the project corridor, primarily small side roads, are expected to have unacceptable level of service in the design year Build scenario, the project team finds this to be an over-estimation due to software model limitations. Finally, planning-level signal warrant analysis indicates that no additional signals are warranted along the PI 0014131 project corridor.

## **Attachment 5**

### **Traffic Diagrams**



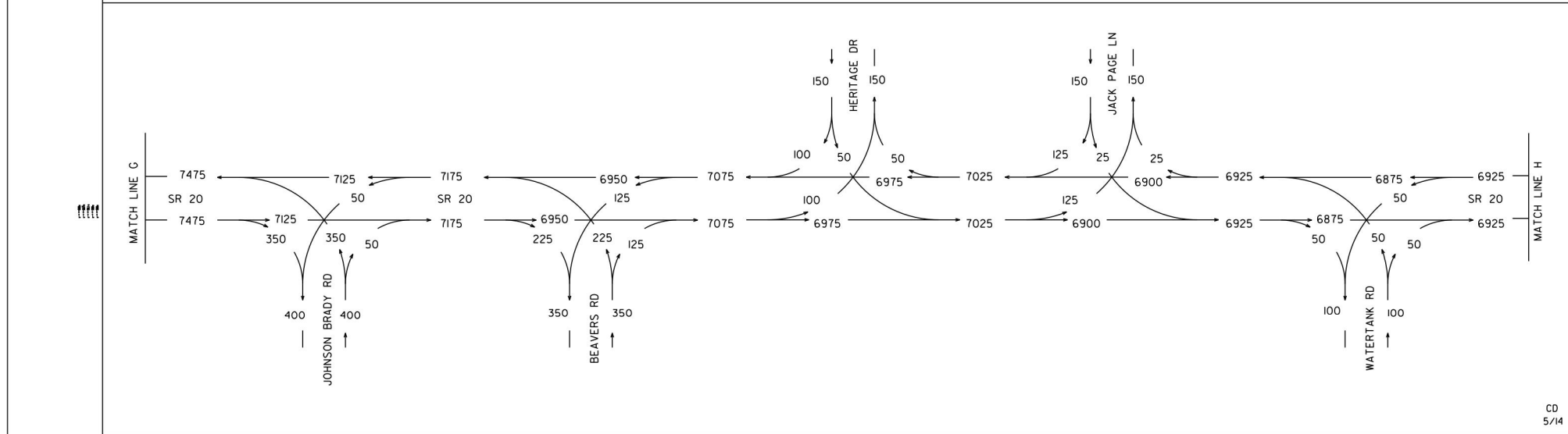
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10-1



DRAWING No.  
10-2





[illegible]

2011 ADT

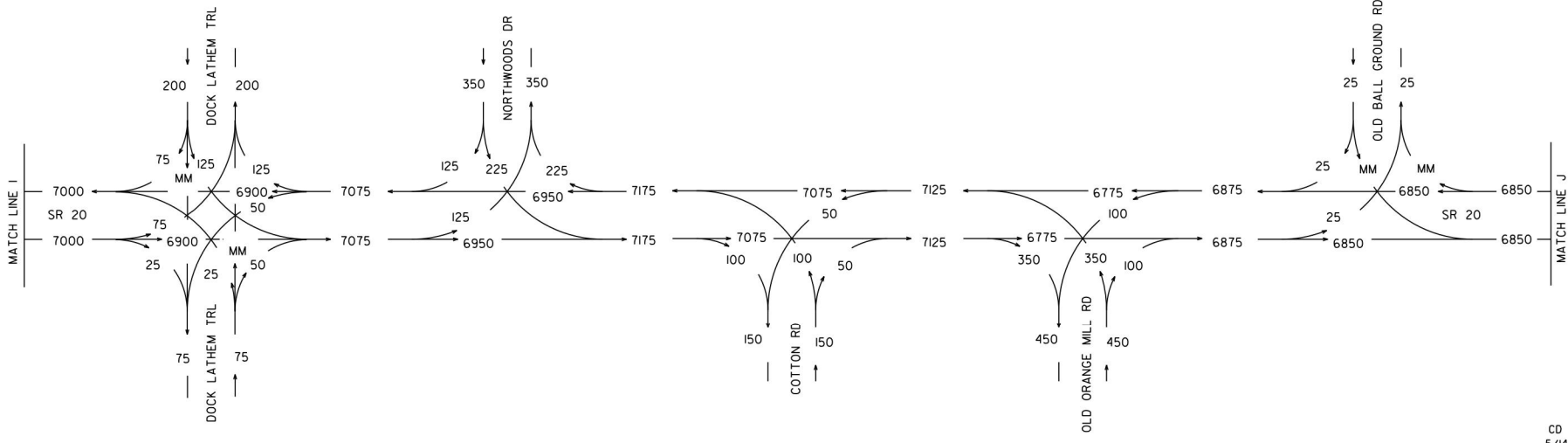
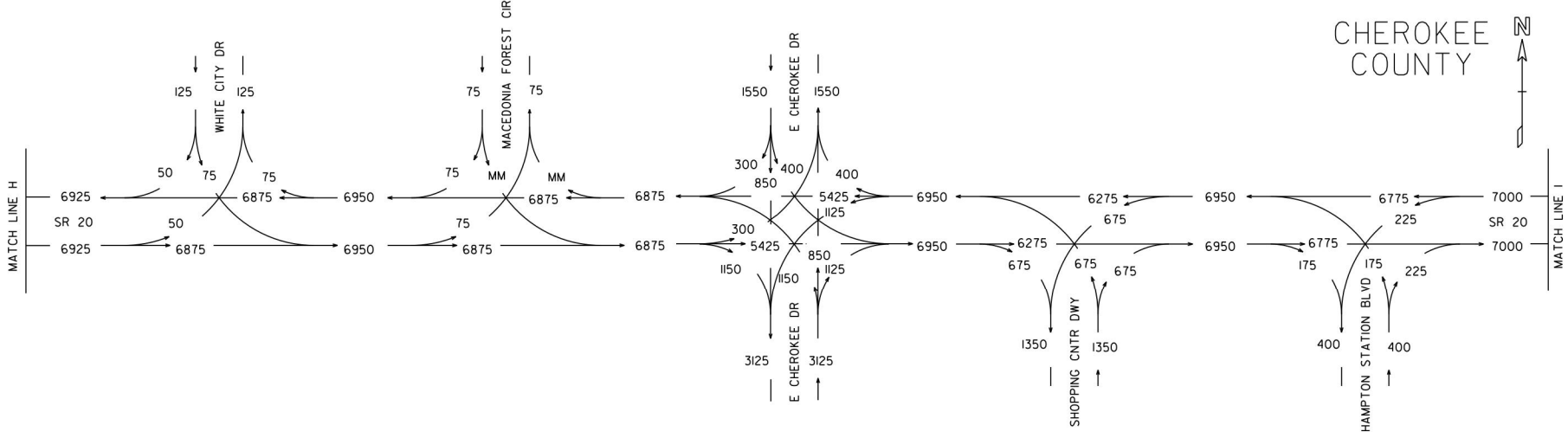
**GCA** GCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH. 404-355-4010 FAX 404-355-0604

24 HOURS T = 16%  
S.U. = 10%  
COMB. = 6%

[illegible]

STATE OF GEORGIA	
DEPARTMENT OF TRANSPORTATION	
OFFICE:	PLANNING
TRAFFIC DIAGRAM	
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DRAWING No  
10-4



STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2011 ADT

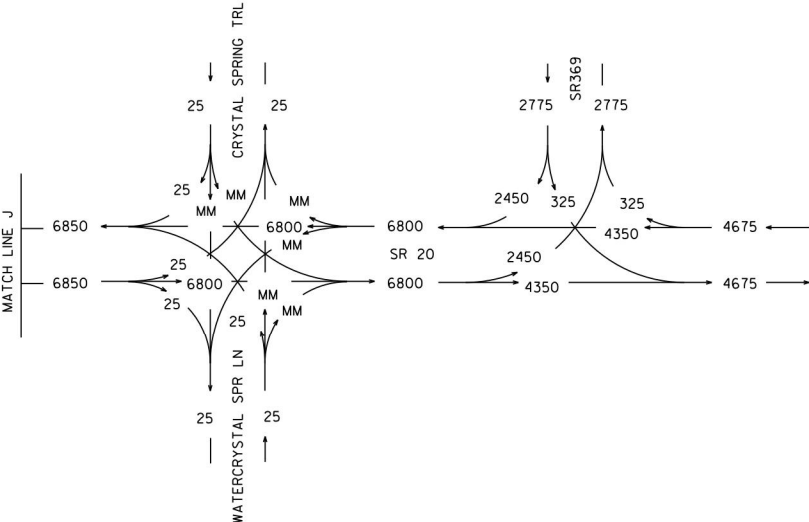
**GCA**  
GCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH 404-355-4010 FAX 404-355-0604

24 HOURS T = 16%  
S.U. = 10%  
COMB. = 6%

REVISION DATES		

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.  
**10-5**



CHEROKEE COUNTY

N

STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2011 ADT

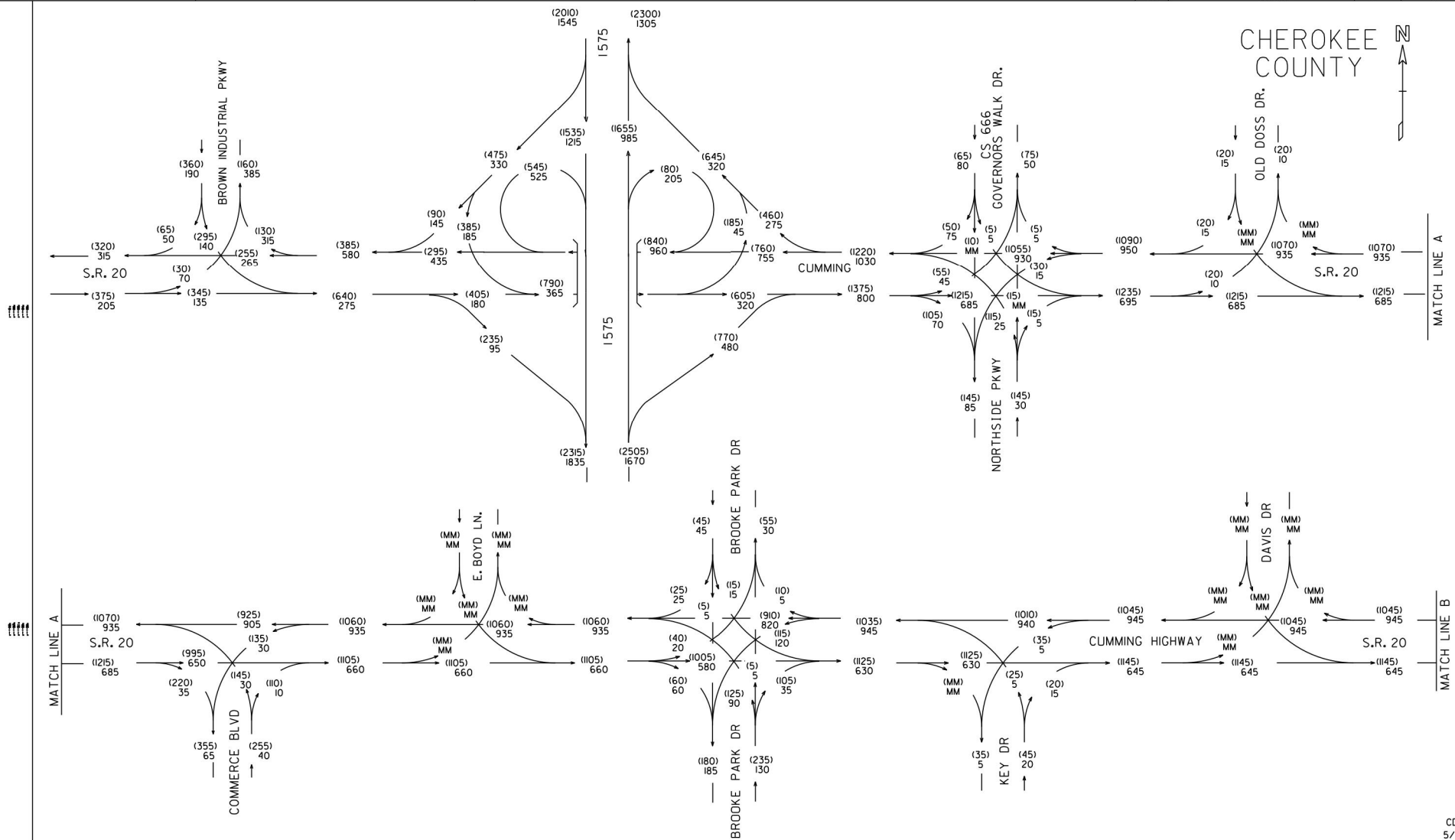
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1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH: 404-355-4010 FAX: 404-355-0604

24 HOURS T = 16%  
S.U. = 10%  
COMB. = 6%

REVISION DATES		

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.  
10-6



STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2011 PM DHV = (000)  
2011 AM DHV = 000

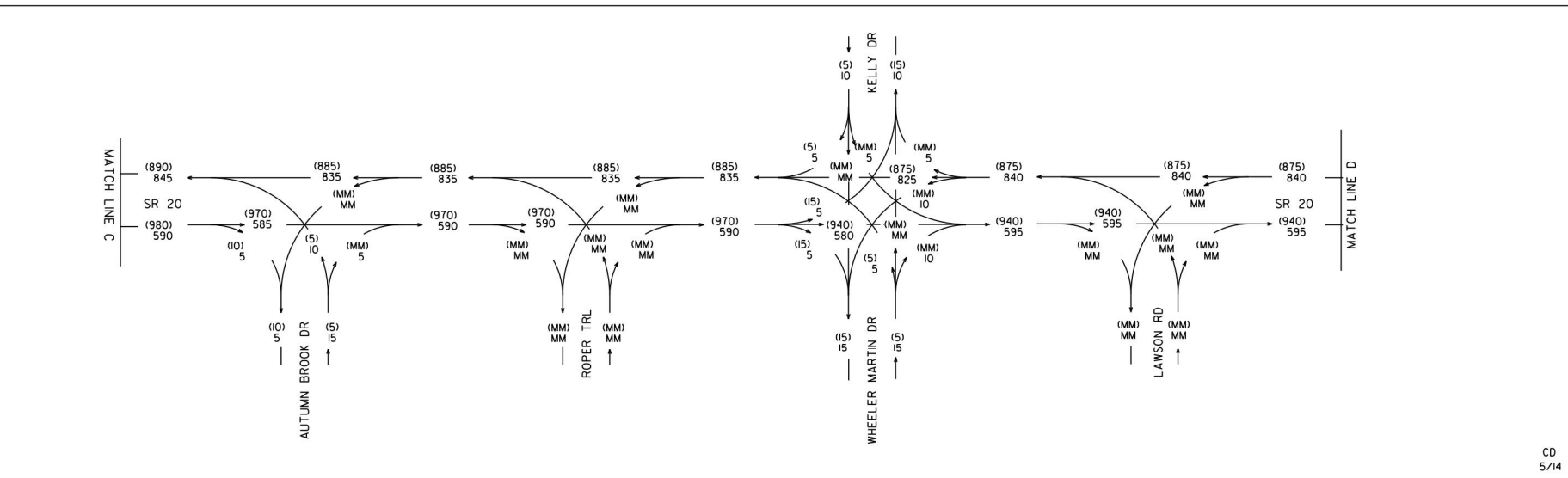
**GCA** INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH. 404-355-4010 FAX 404-355-0604

T = 12.5%  
S.U. = 7.5%  
COMB. = 5%

REVISION DATES

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

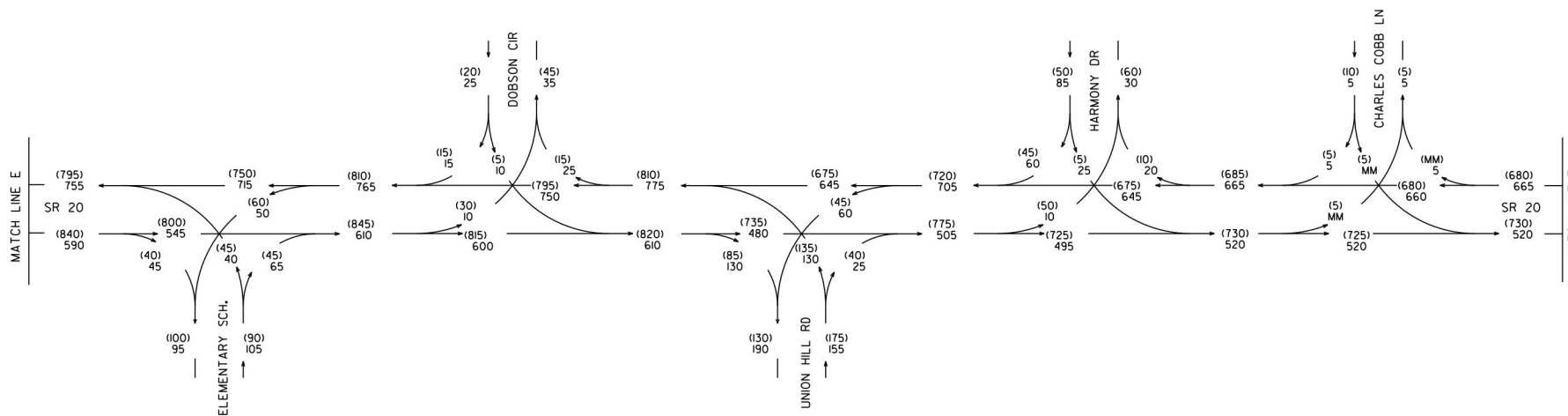
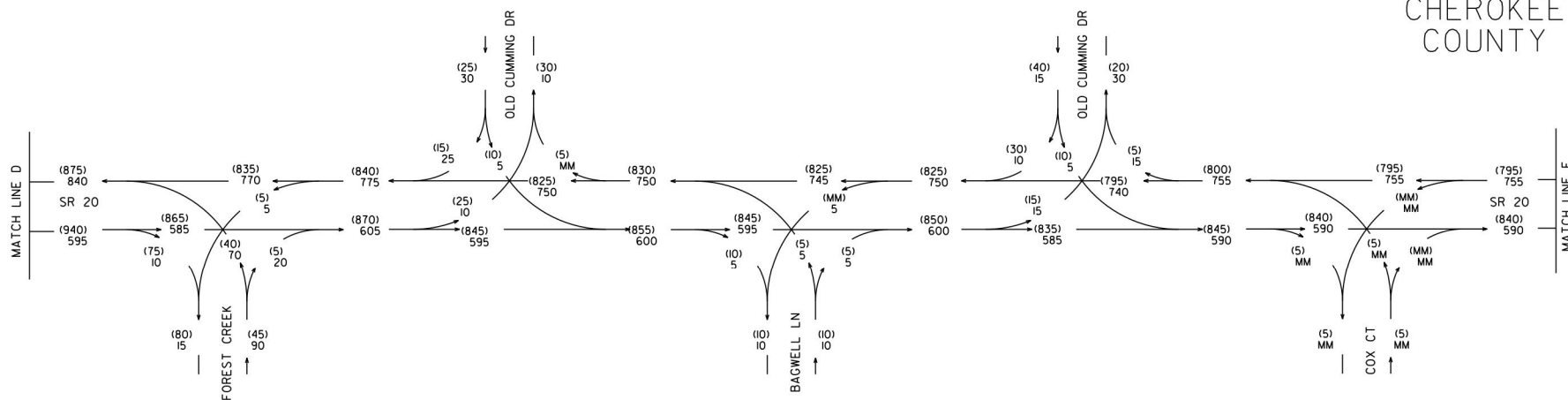
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REVISION DATES			STATE OF GEORGIA	
			DEPARTMENT OF TRANSPORTATION	
			OFFICE: PLANNING	
			TRAFFIC DIAGRAM	
			DRAWING No.	
			10-8	



DATE##	TIME##	PRF##	DOC##	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
USER#		SPENTABLE##		GA	0003681	3	6



STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2011 PM DHV = (000)  
2011 AM DHV = 000

**GCA**  
GCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH: 404-355-4000 FAX: 404-355-0604

T = 12.5%  
S.U. = 7.5%  
COMB. = 5%

REVISION DATES

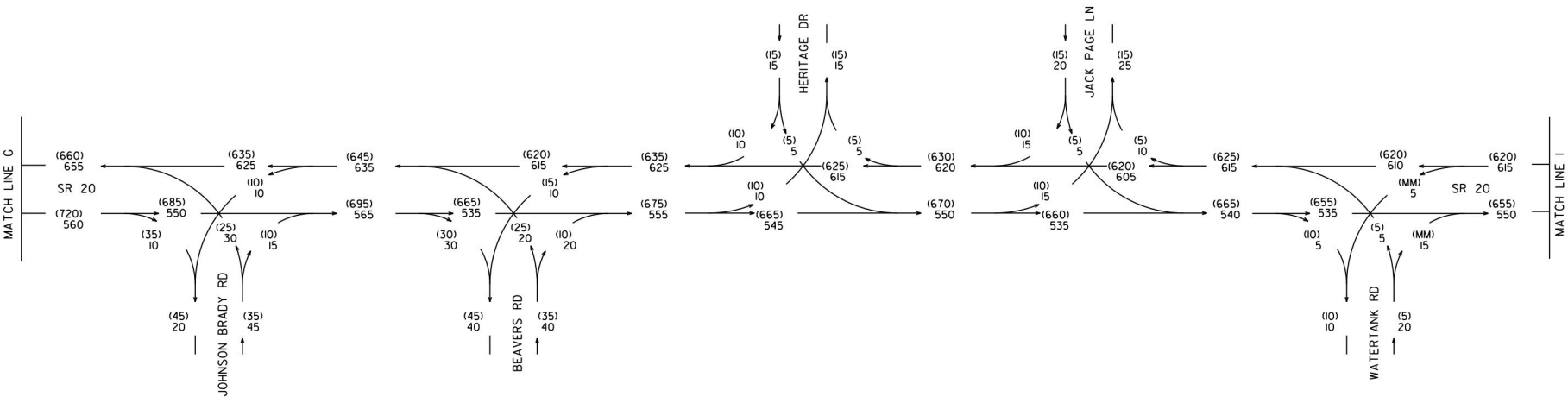
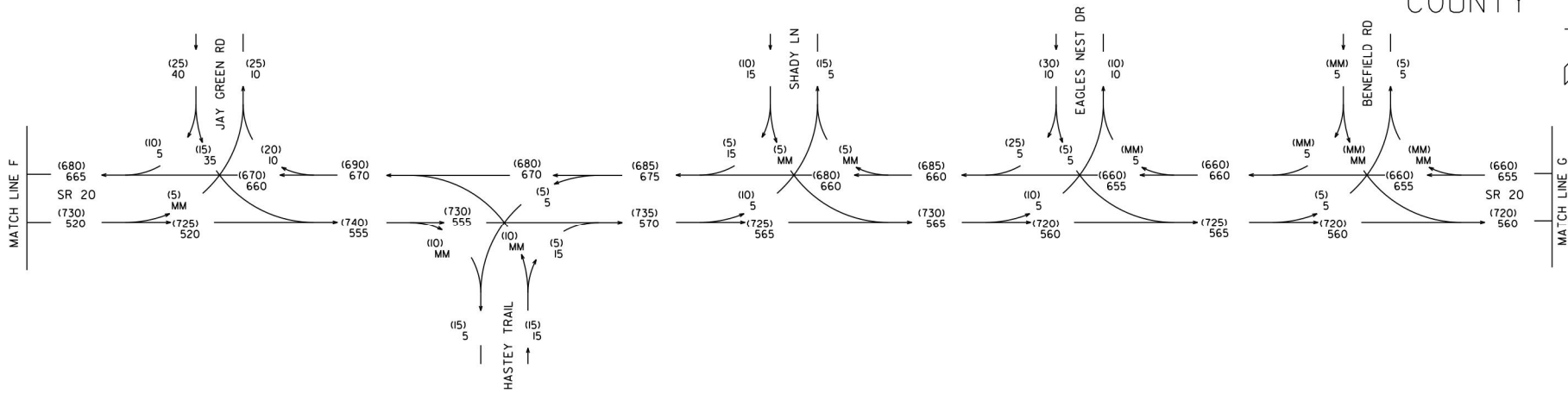
STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.  
10-9

CD  
5/14

CHEROKEE COUNTY

N



STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2011 PM DHV = (000)  
2011 AM DHV = 000

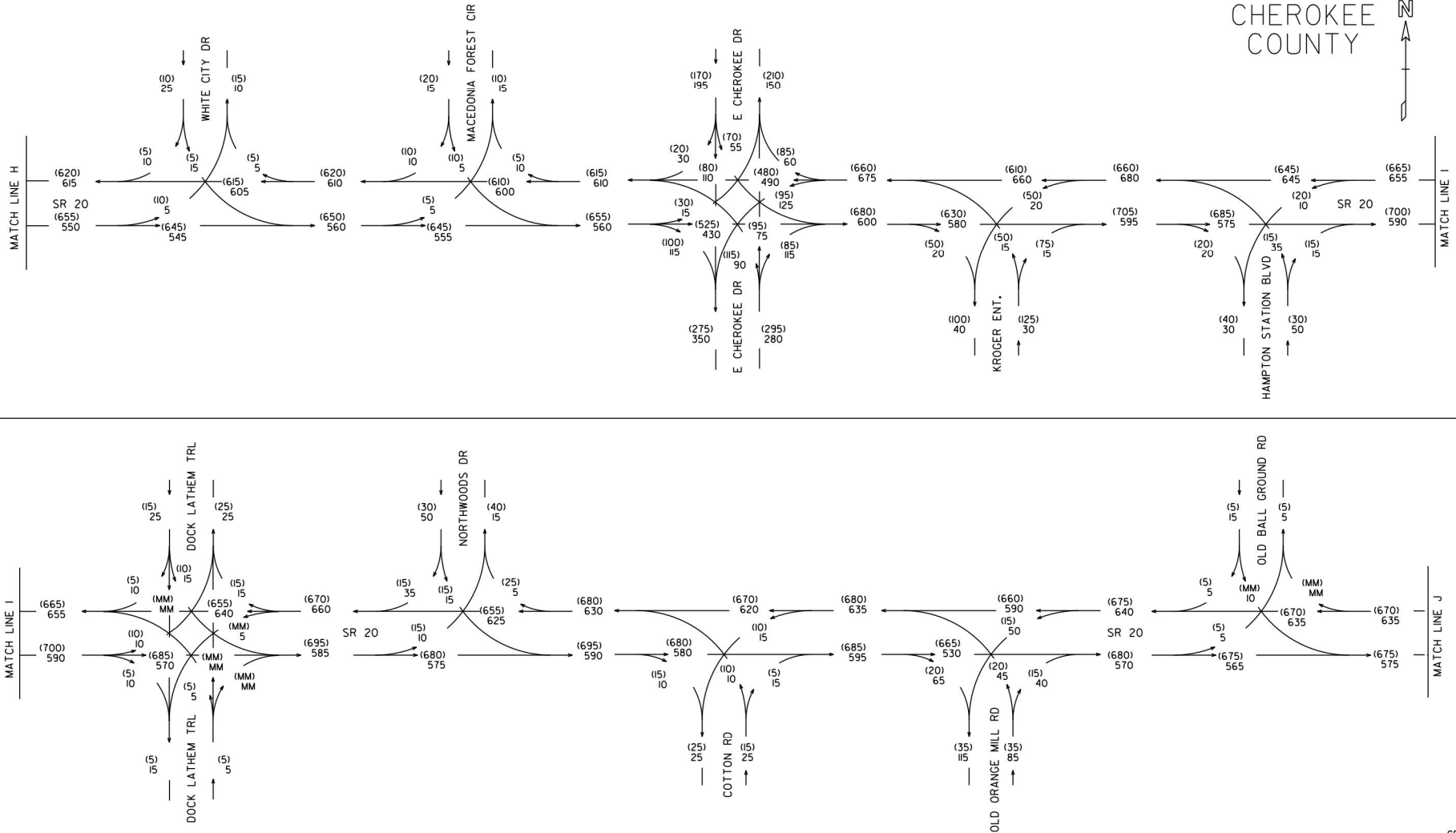
**GCA**  
GCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH: 404-355-4000 FAX: 404-355-0604

T = 12.5%  
S.U. = 7.5%  
COMB. = 5%

REVISION DATES	

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.  
10-10



STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2011 PM DHV = (000)  
2011 AM DHV = 000

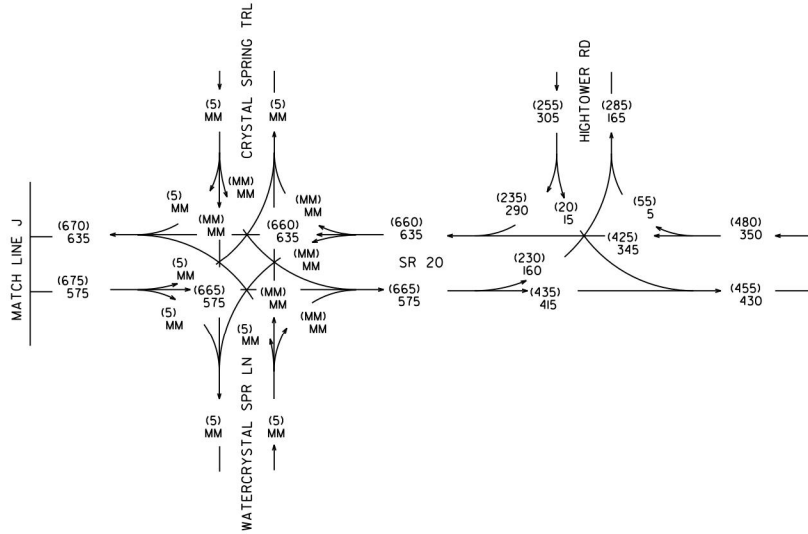
**GCA**  
CCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH 404-355-4000 FAX 404-355-0604

T = 12.5%  
S.U. = 7.5%  
COMB. = 5%

REVISION DATES

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING No.  
10-11



CHEROKEE COUNTY

N

CD  
5/14

STP00-0003-00(681)  
P.I.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2011 PM DHV = (000)  
2011 AM DHV = 000

**GCA** GCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH. 404-355-4000 FAX 404-355-0604

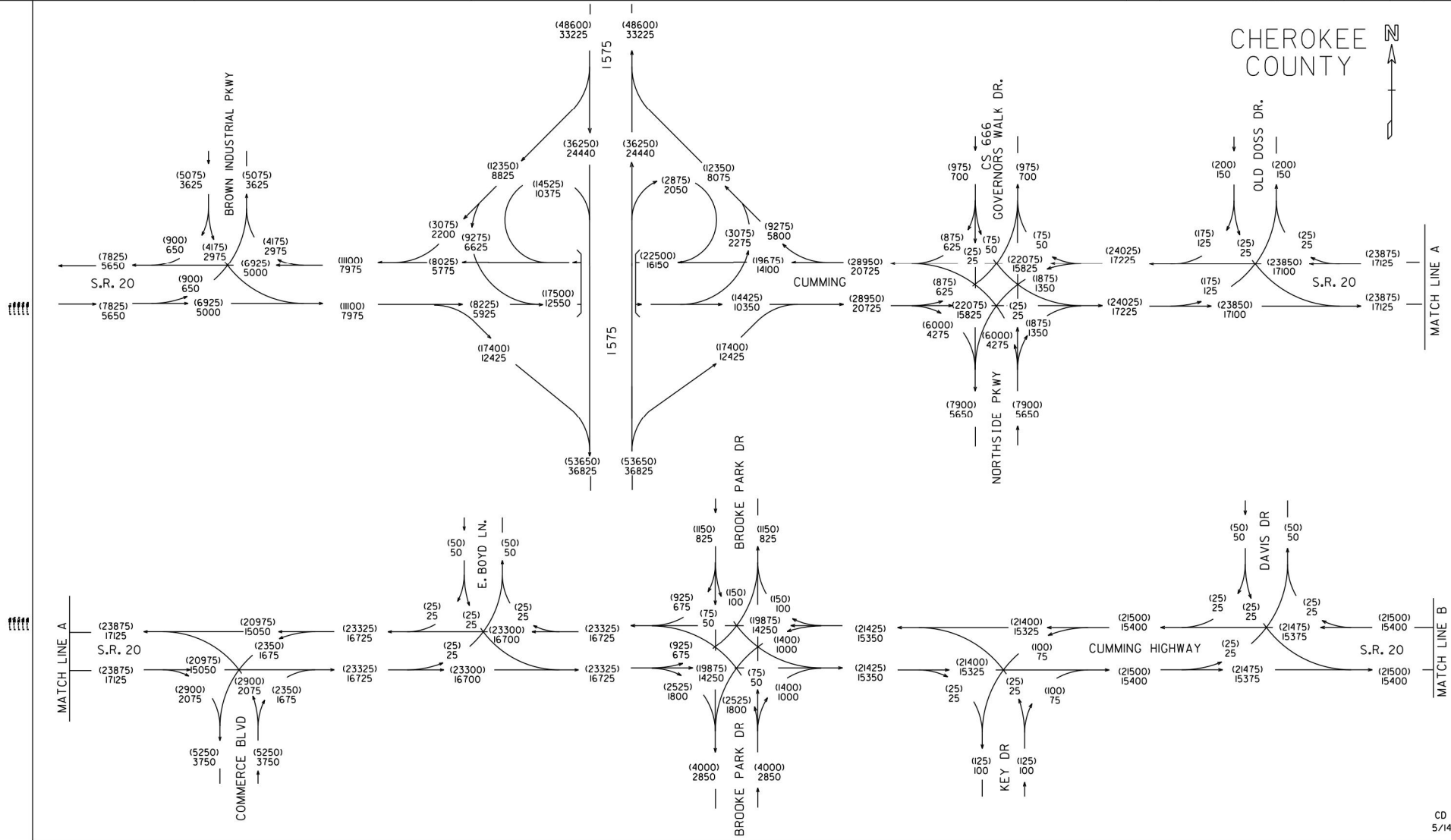
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COMB. = 5%

REVISION DATES

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING No.

10-12



STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2045 NO BUILD ADT = (000)  
2025 NO BUILD ADT = 000

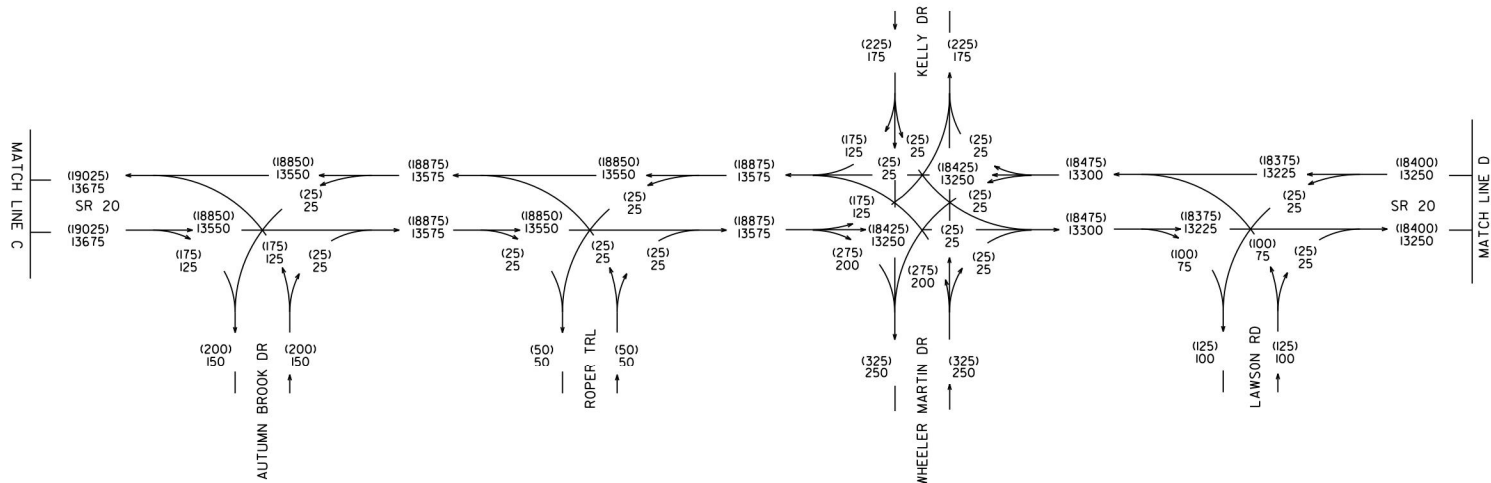
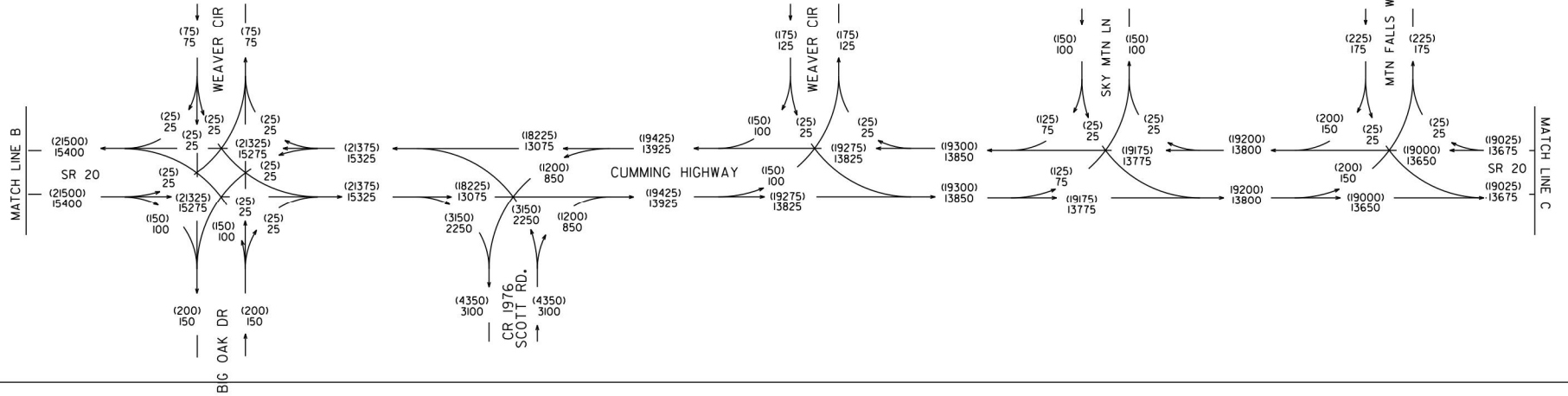
**GCA** GCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH. 404-355-4010 FAX 404-355-0604

24 HOURS T = 16%  
S.U. = 10%  
COMB. = 6%

REVISION DATES			STATE OF GEORGIA DEPARTMENT OF TRANSPORTATION	
			OFFICE: PLANNING	
			TRAFFIC DIAGRAM	
			DRAWING NO. 10-13	



# CHEROKEE COUNTY



CD  
5/14

STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

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2025 NO BUILD ADT = 000

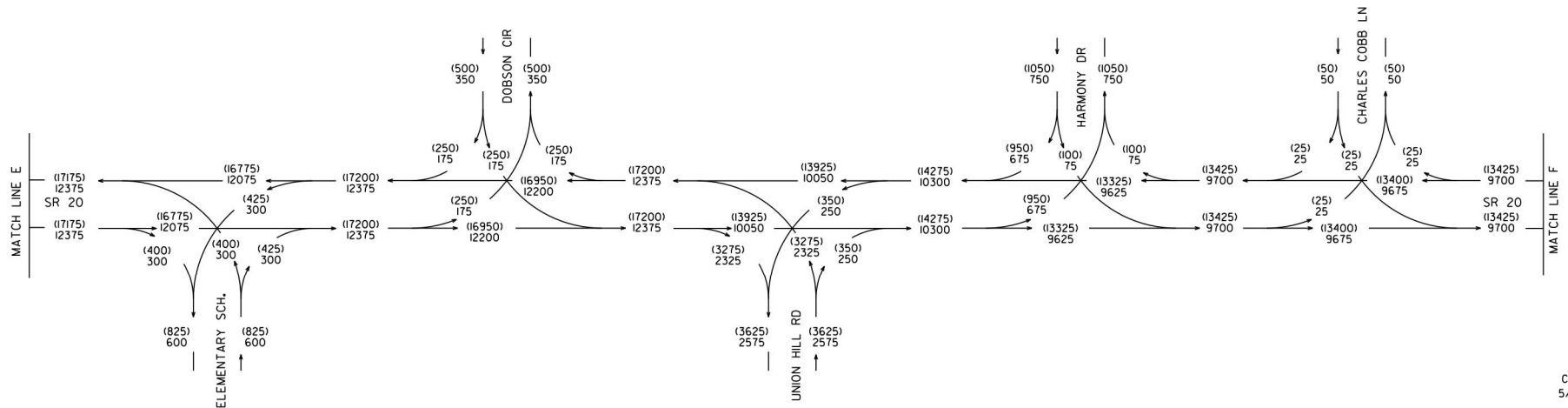
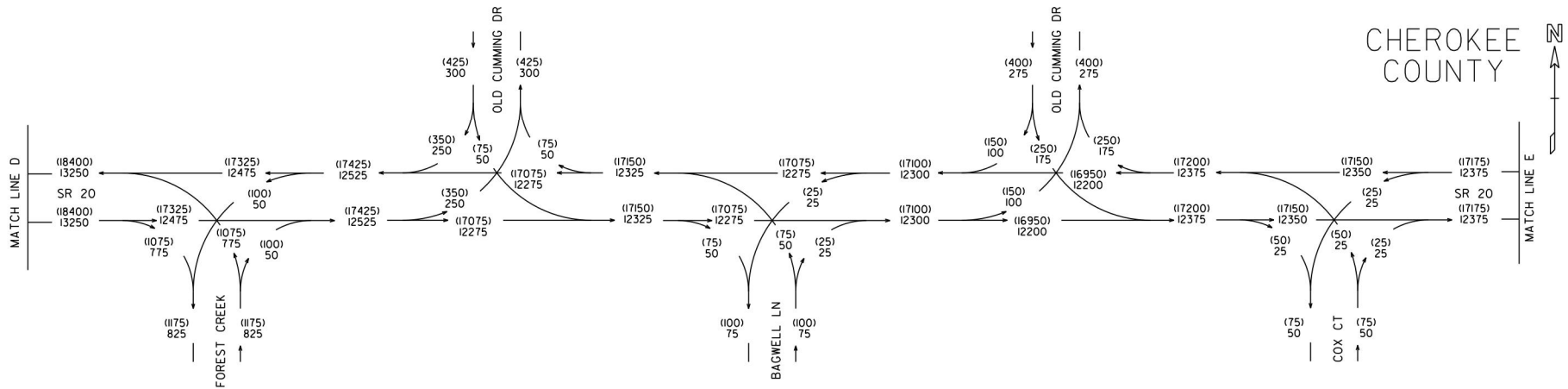
**GCA**  
GCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH: 404-355-4000 FAX: 404-355-0604

24 HOURS T = 16%  
S.U. = 10%  
COMB. = 6%

## REVISION DATES


STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.  
10-14



STP00-0003-00(681)  
P.L.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2045 NO BUILD ADT = (000)  
2025 NO BUILD ADT = 000

**GCA**  
GCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH: 404-355-4010 FAX: 404-355-0604

24 HOURS T = 16%  
S.U. = 10%  
COMB. = 6%

REVISION DATES

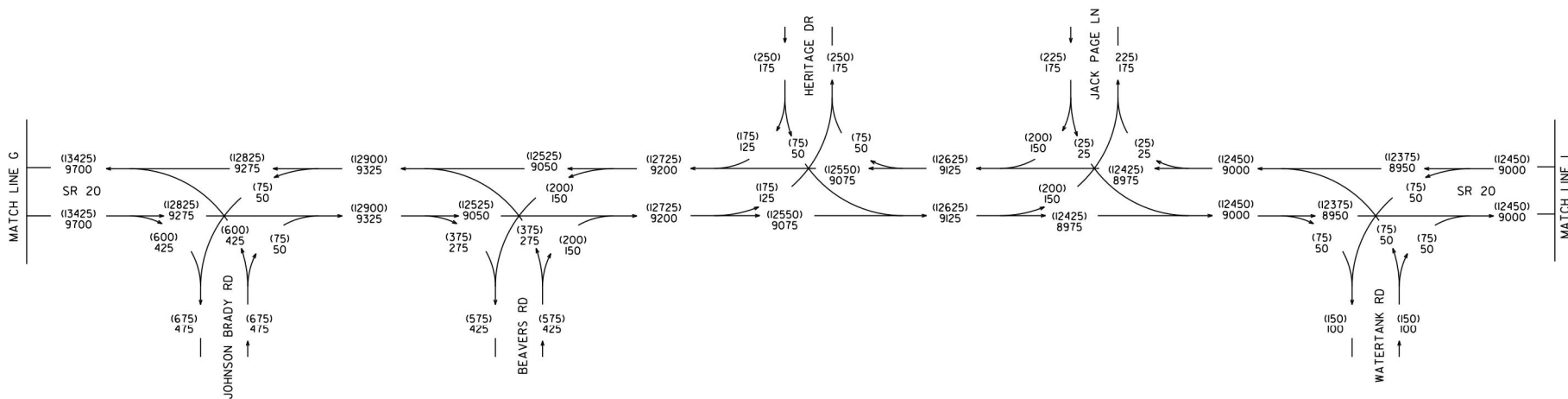
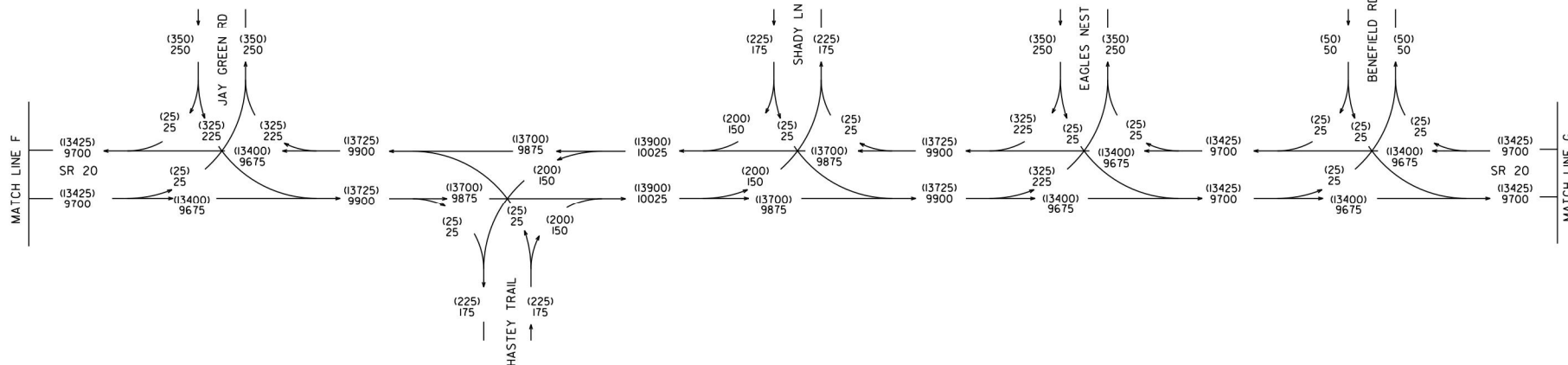
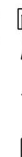
STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING No.

10-15

DATE##	TIME##	#PRF##	#DCW#	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
#USER#		#PENTABLE##		GA	0003681	4	6

CHEROKEE COUNTY



CD  
5/14

STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2045 NO BUILD ADT = (000)  
2025 NO BUILD ADT = 000

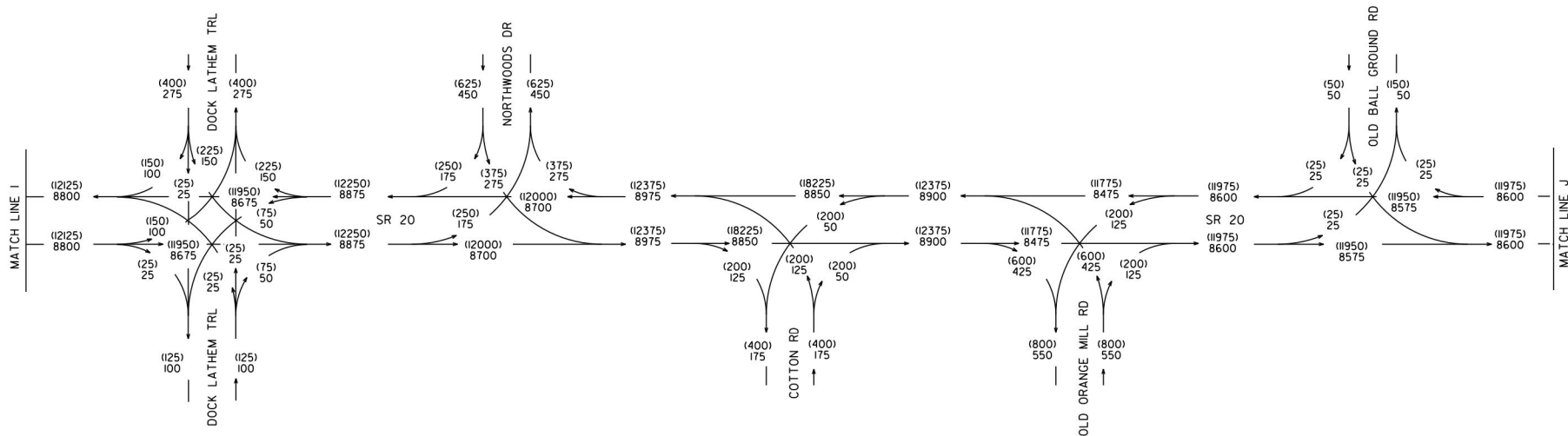
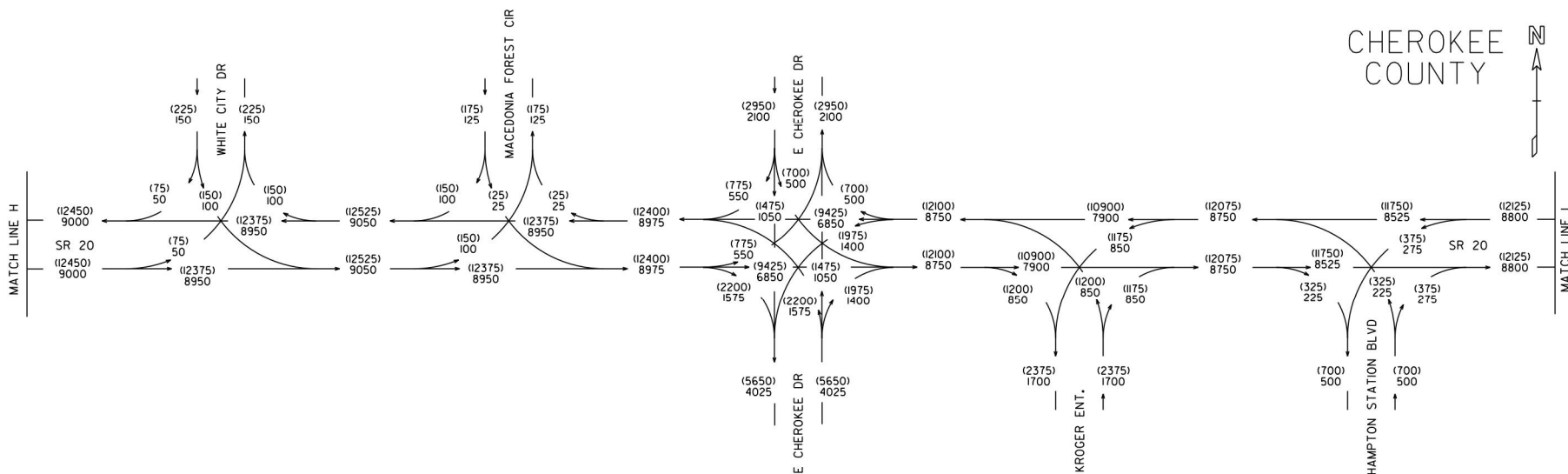
**GCA**  
GCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH 404-355-4000 FAX 404-355-0604

24 HOURS T = 16%  
S.U. = 10%  
COMB. = 6%

REVISION DATES


STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.  
**10-16**



STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2045 NO BUILD ADT = (000)  
2025 NO BUILD ADT = 000

**GCA**  
GCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH 404-355-4000 FAX 404-355-0604

24 HOURS T = 16%  
S.U. = 10%  
COMB. = 6%

REVISION DATES


STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.  
10-17

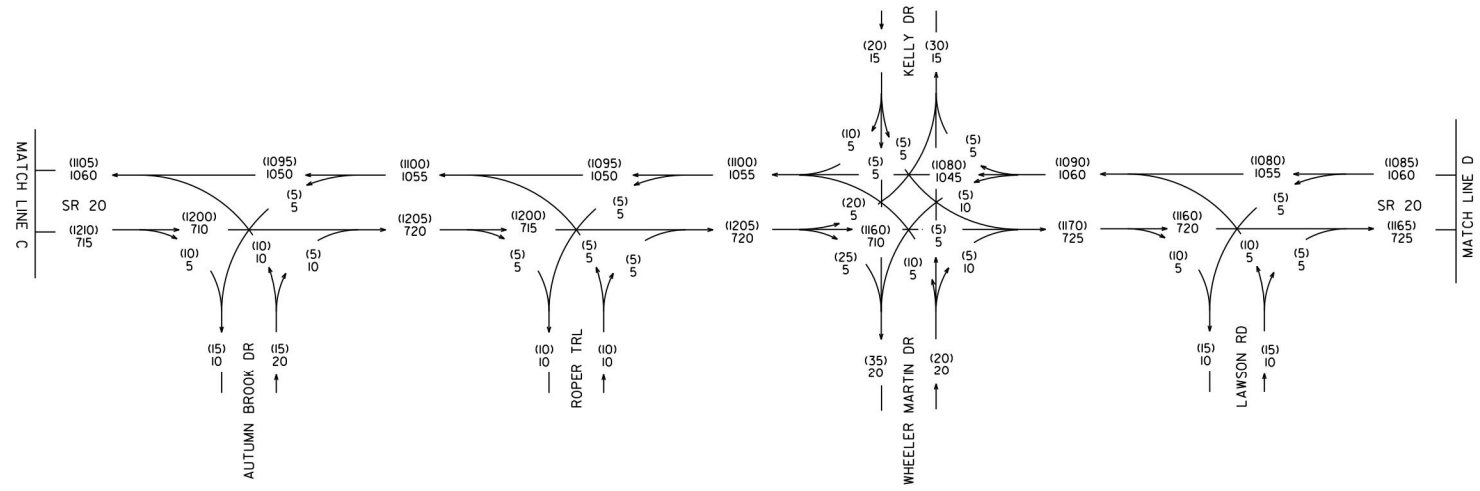
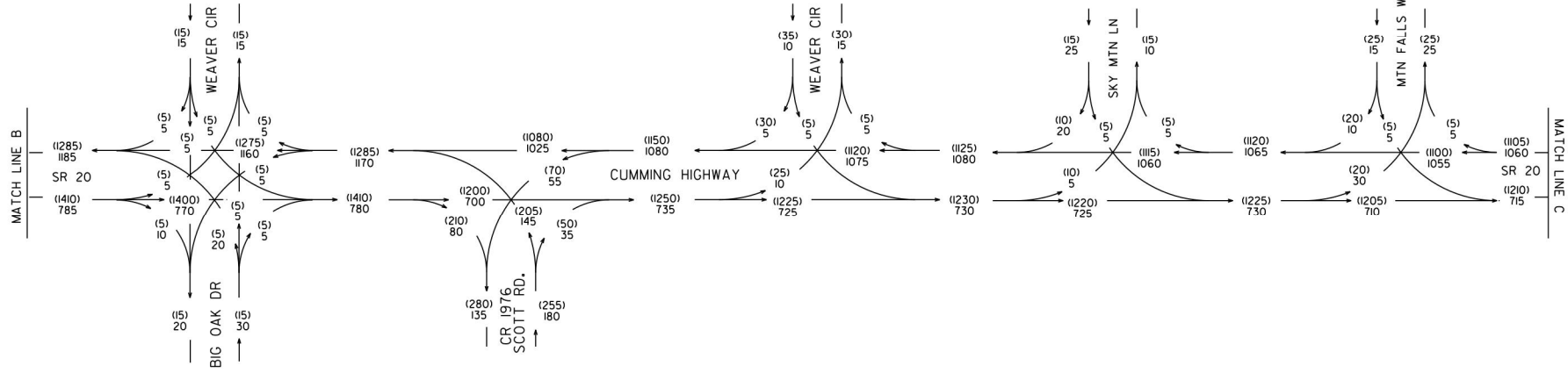






DRAWING No.  
10-19

CHEROKEE COUNTY



CD  
5/14

STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2025 NO BUILD PM DHV = (000)  
2025 NO BUILD AM DHV = 000

**GCA**  
GCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH 404-355-4010 FAX 404-355-0604

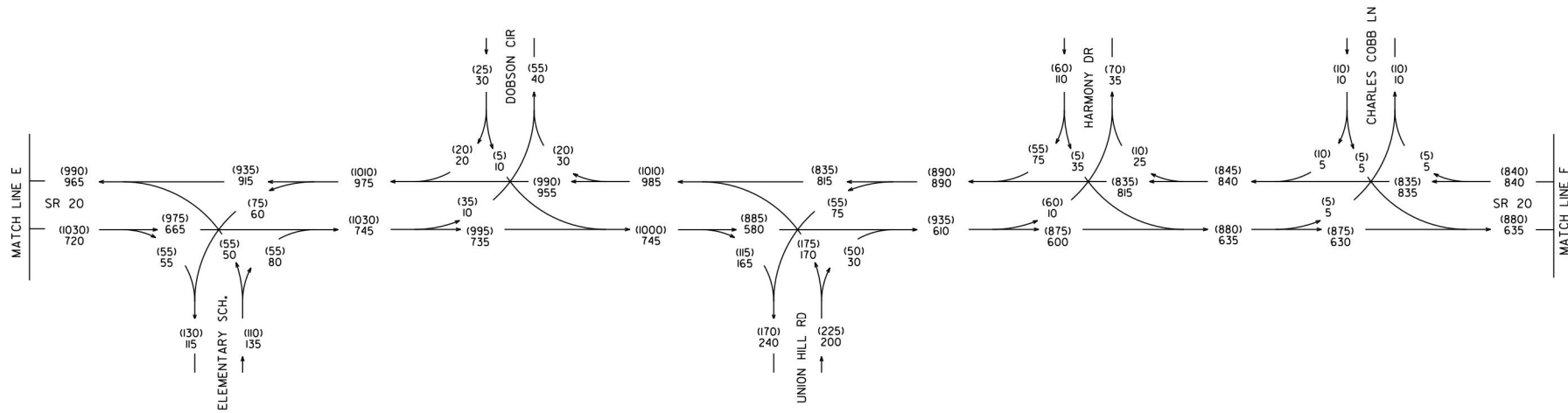
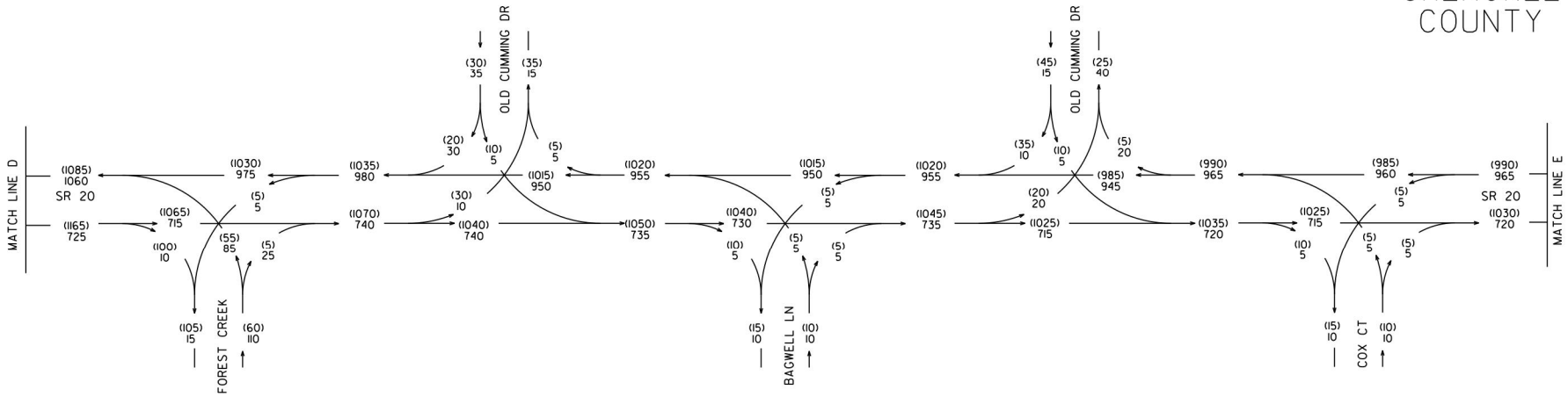
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COMB. = 5%

REVISION DATES	

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.  
10-20

CHEROKEE COUNTY



CD  
5/14

STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2025 NO BUILD PM DHV = (000)  
2025 NO BUILD AM DHV = 000

**GCA**  
CCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH 404-355-4000 FAX 404-355-0604

T = 12.5%  
S.U. = 7.5%  
COMB. = 5%

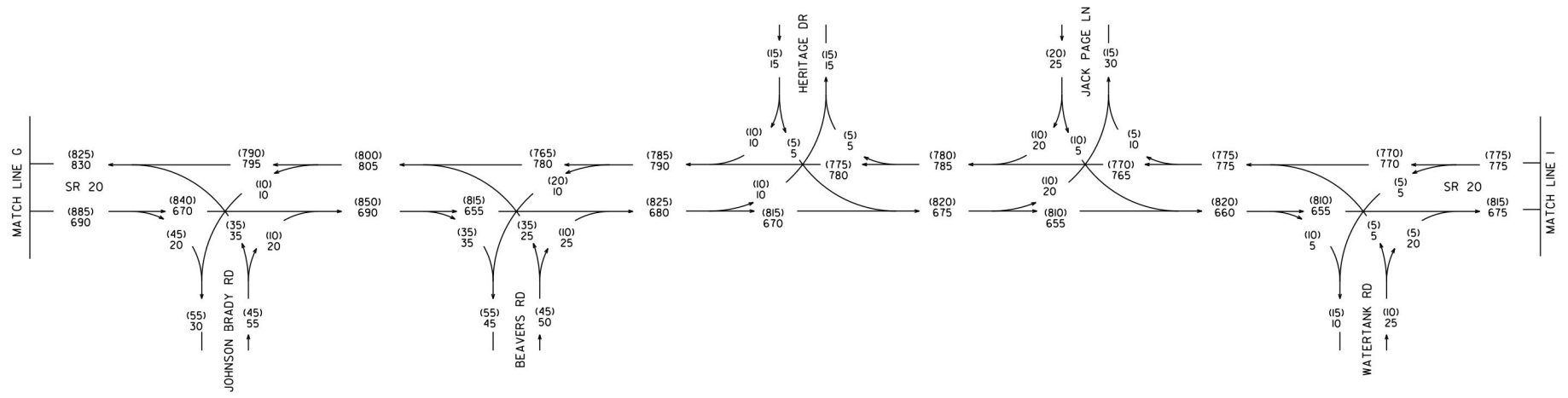
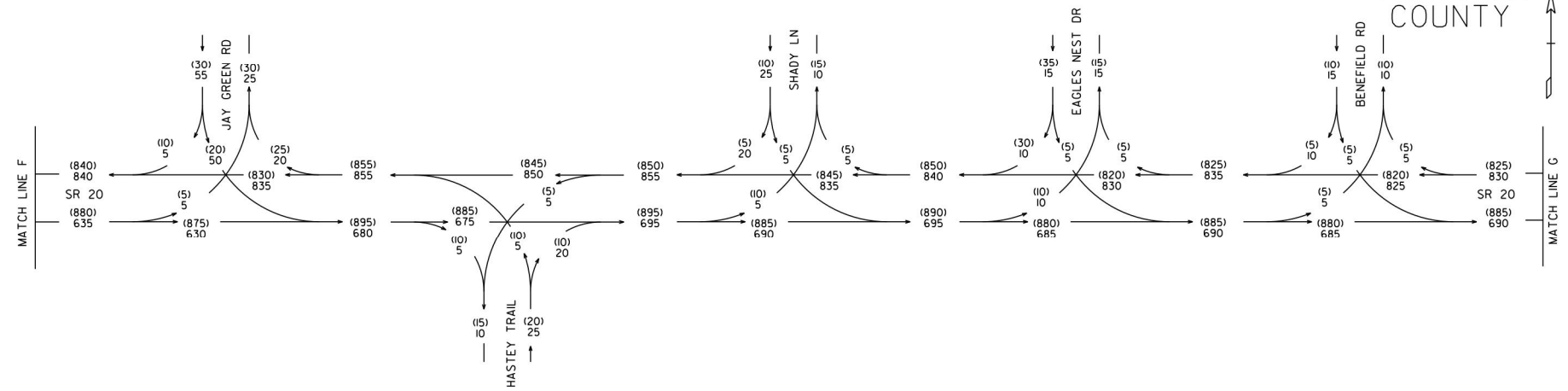
REVISION DATES

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.

10-21

CHEROKEE COUNTY



CD  
5/14

STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2025 NO BUILD PM DHV = (000)  
2025 NO BUILD AM DHV = 000

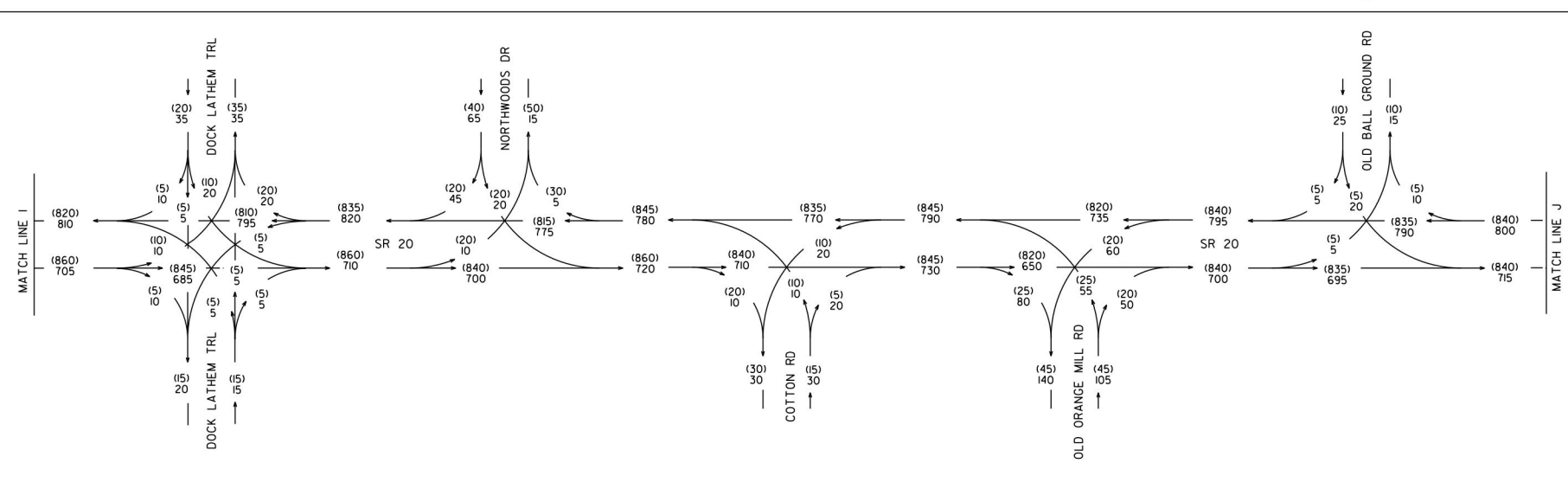
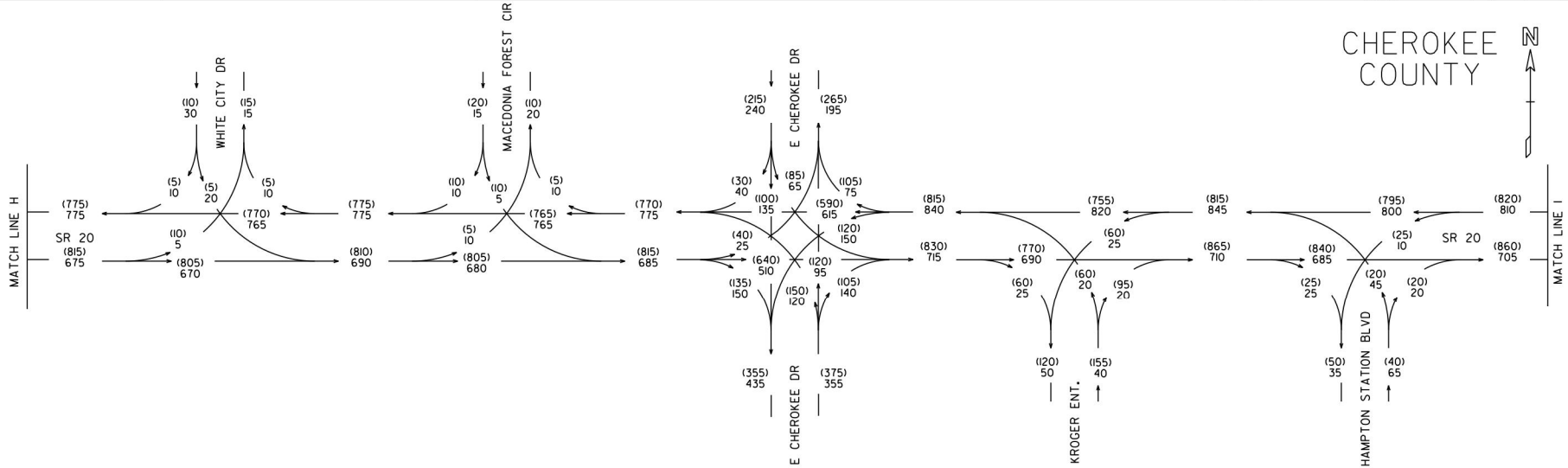
**GCA**  
GCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH: 404-355-4000 FAX: 404-355-0604

T = 12.5%  
S.U. = 7.5%  
COMB. = 5%

REVISION DATES	

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.  
10-22



STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2025 NO BUILD PM DHV = (000)  
2025 NO BUILD AM DHV = 000

**GCA** GCA, INC.  
800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH: 404-355-4000 FAX: 404-355-0604

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S.U. = 7.5%  
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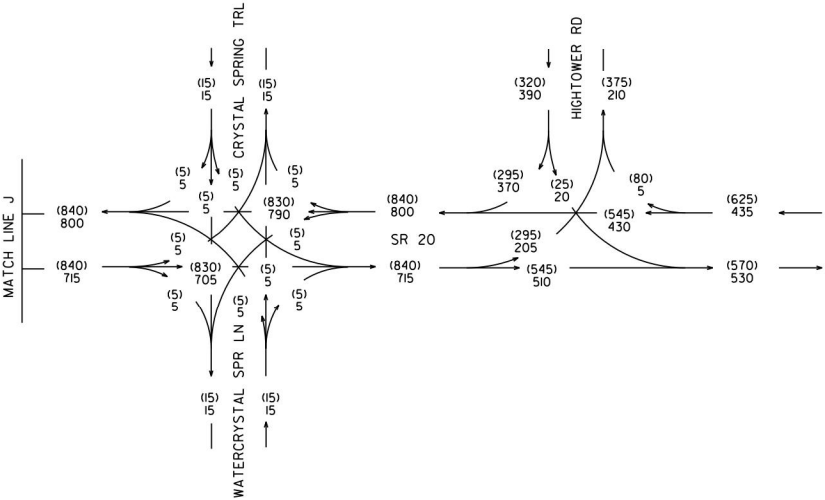
REVISION DATES

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

CD  
5/14

DRAWING NO.  
10-23





CHEROKEE  
COUNTY

CD  
5/14

STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2025 NO BUILD PM DHV = (000)  
2025 NO BUILD AM DHV = 000

**GCA**  
GCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH: 404-355-4000 FAX: 404-355-0604

T = 12.5%  
S.U. = 7.5%  
COMB. = 5%

REVISION DATES			STATE OF GEORGIA DEPARTMENT OF TRANSPORTATION	
			OFFICE: PLANNING	
			TRAFFIC DIAGRAM	
			DRAWING NO. 10-24	

CHEROKEE COUNTY

N

MATCH LINE A

MATCH LINE B

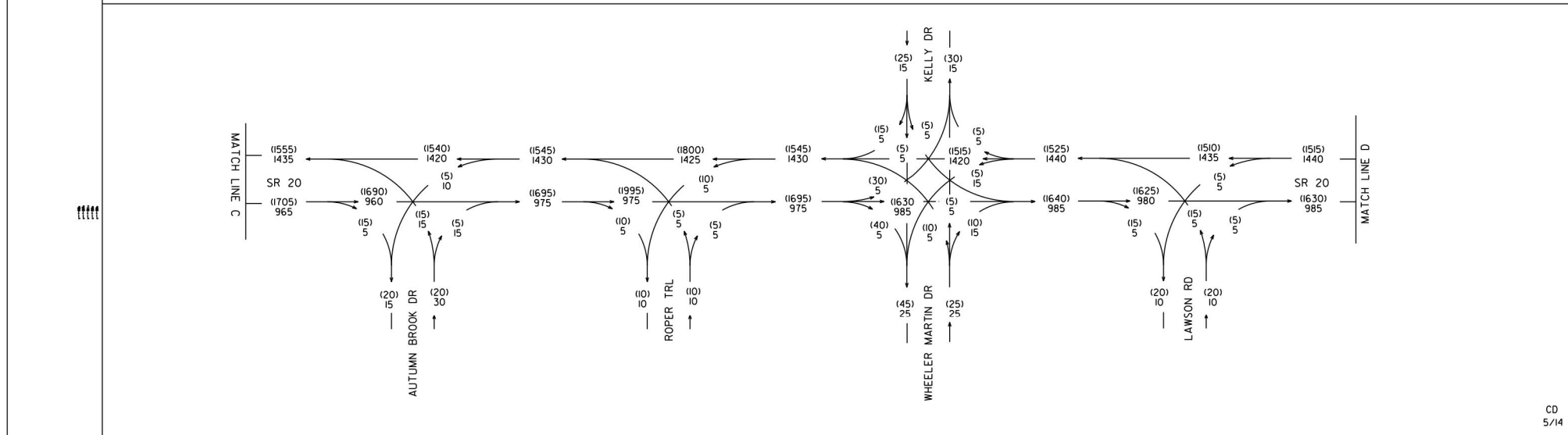
Diagram illustrating a road network in Cherokee County, Georgia, showing various intersections and traffic volumes.

Key roads and intersections include:

- Top Section:**
  - Intersections with S.R. 20 and Brown Industrial Pkwy.
  - Intersections with S.R. 20 and Commerce Blvd.
  - Intersections with S.R. 20 and E. Boyd Ln.
  - Intersections with S.R. 20 and Brooke Park Dr.
  - Intersections with S.R. 20 and Cumming Highway.
  - Intersections with S.R. 20 and Key Dr.
- Bottom Section:**
  - Intersections with S.R. 20 and Brooke Park Dr.
  - Intersections with S.R. 20 and Cumming Highway.
  - Intersections with S.R. 20 and Key Dr.

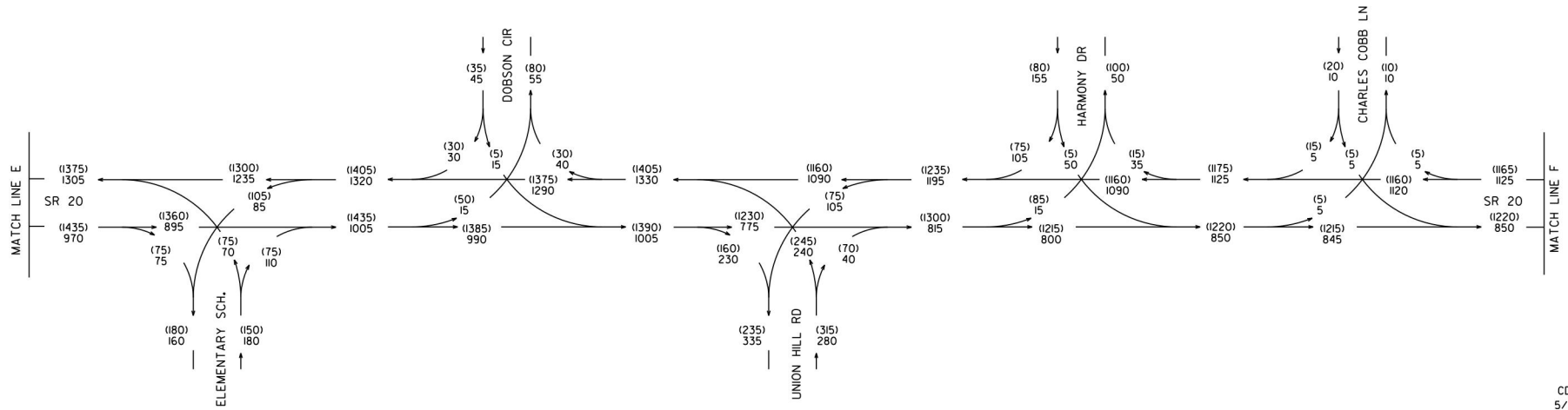
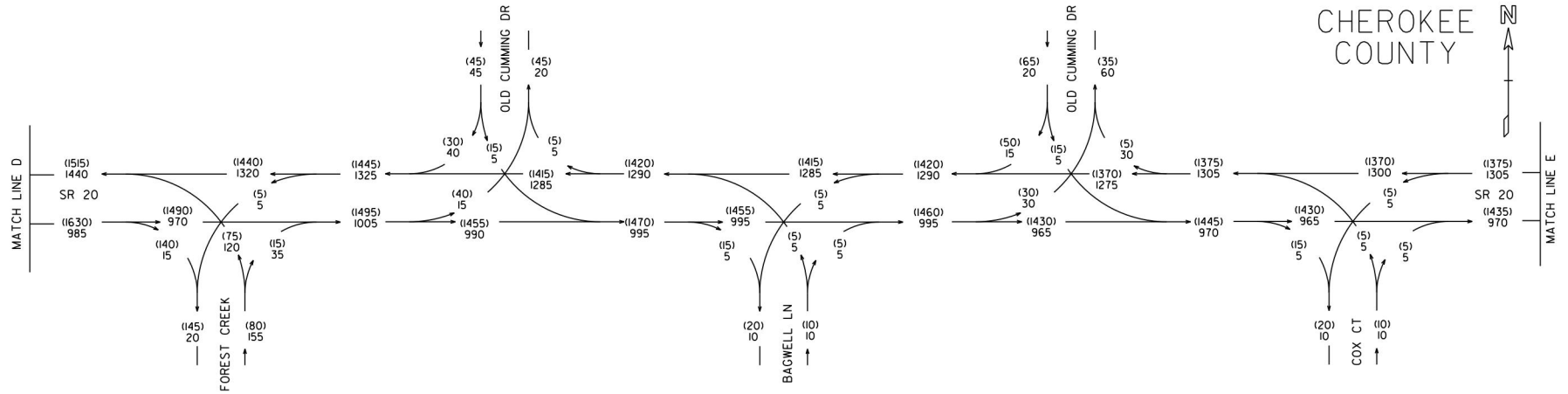
Traffic volumes are indicated by numbers in parentheses at each intersection. The diagram also shows Match Line A and Match Line B, indicating the extent of the study area.

DRAWING No  
10-25



DRAWING No  
10-26

DATE**	TIME**	PRF**	DCG**	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
000000	000000	000000	000000	GA	0003681	3	6



STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2045 NO BUILD PM DHV = (000)  
2045 NO BUILD AM DHV = 000

**GCA** GCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH 404-355-4000 FAX 404-355-0604

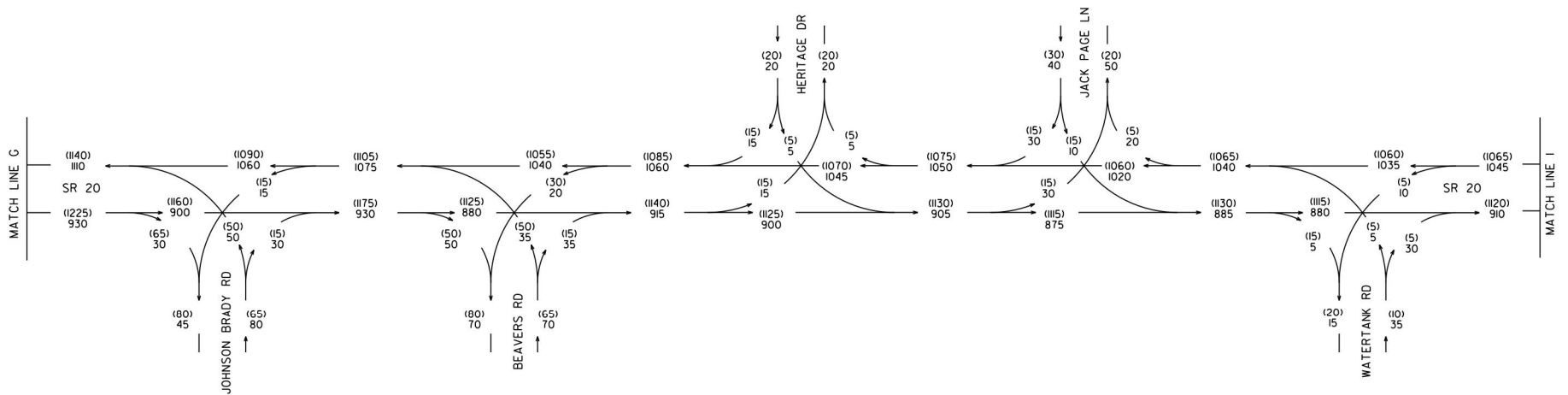
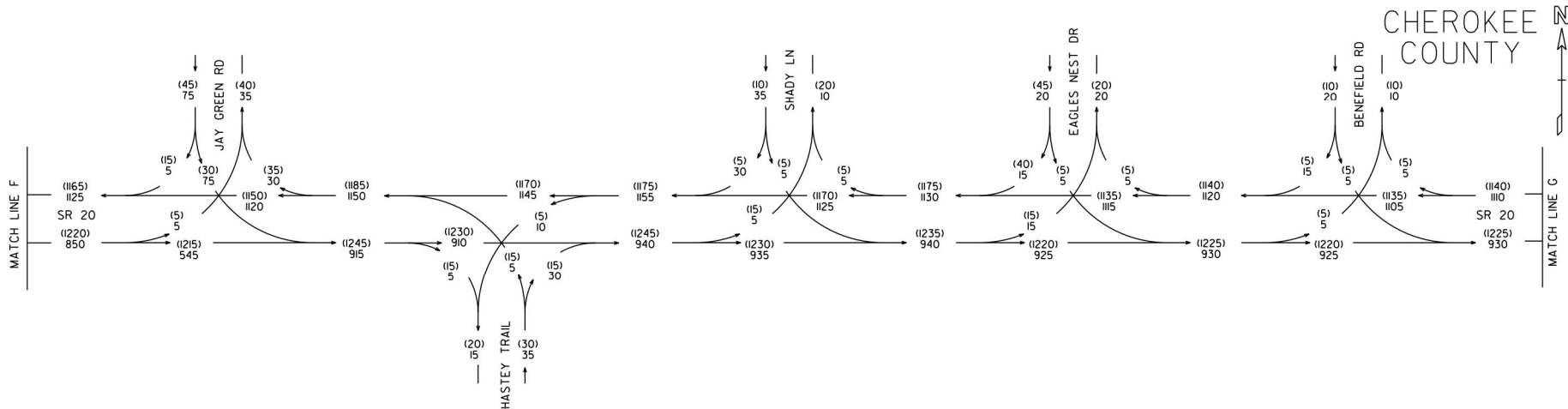
T = 12.5%  
S.U. = 7.5%  
COMB. = 5%

REVISION DATES

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.  
10-27

DATE##	TIME##	PRF##	DCW#	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
USER#		SPENTABLE##		GA	0003681	4	6



STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2045 NO BUILD PM DHV = (000)  
2045 NO BUILD AM DHV = 000

**GCA**  
GCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH: 404-355-4000 FAX: 404-355-0604

T = 12.5%  
S.U. = 7.5%  
COMB. = 5%

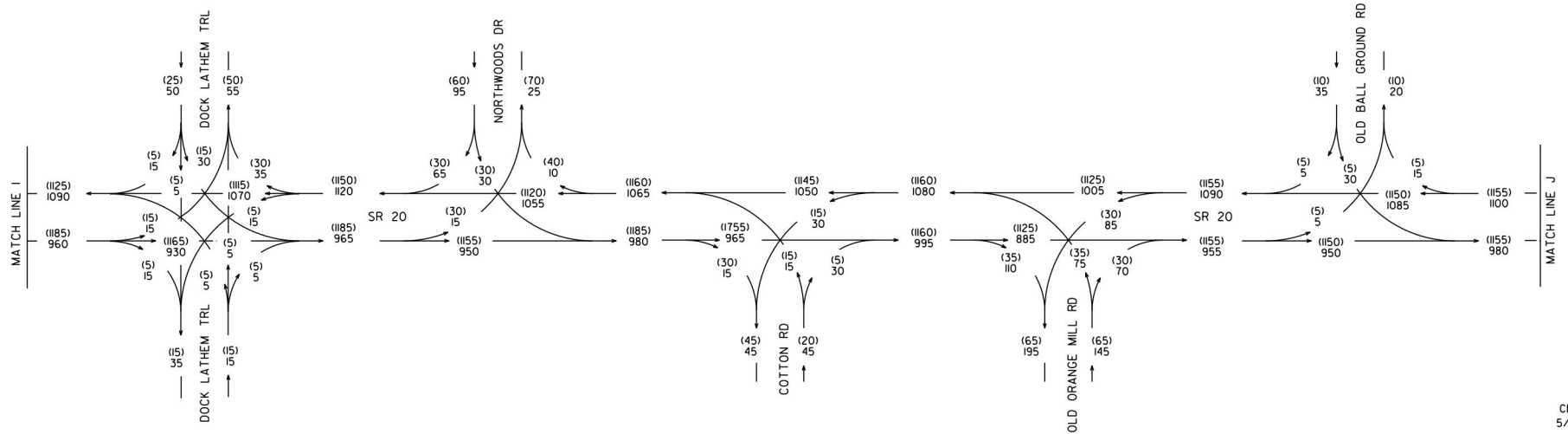
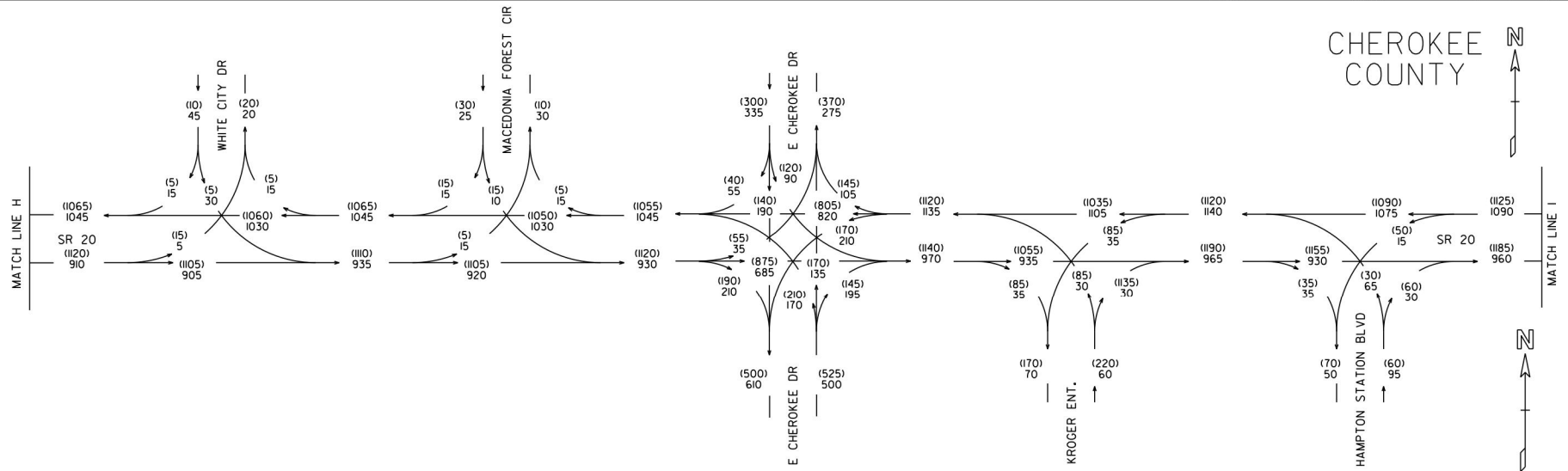
REVISION DATES


STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.  
10-28

CD  
5/14





STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2045 NO BUILD PM DHV = (000)  
2045 NO BUILD AM DHV = 000

**GCA** GCA, INC.  
800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH. 404-355-4000 FAX 404-355-0604

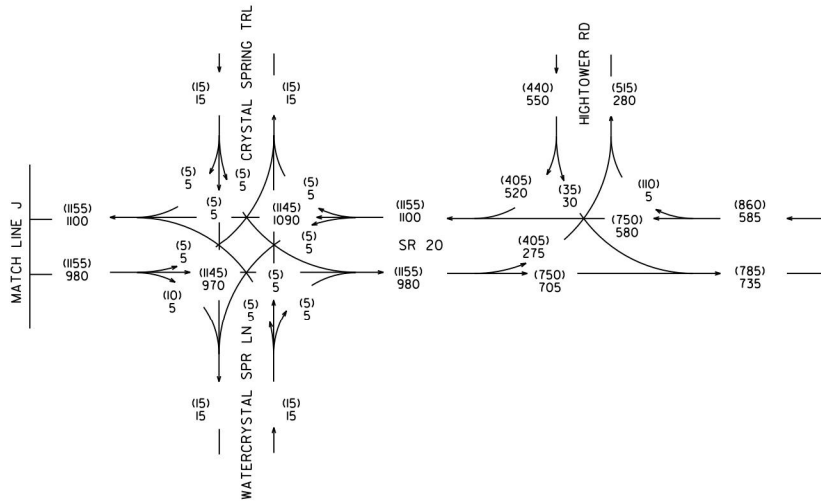
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S.U. = 7.5%  
COMB. = 5%

REVISION DATES


STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.  
10-29

CD  
5/14



CHEROKEE COUNTY

N

CD  
5/14

STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2045 NO BUILD PM DHV = (000)  
2045 NO BUILD AM DHV = 000

**GCA** GCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH 404-355-4000 FAX 404-355-0604

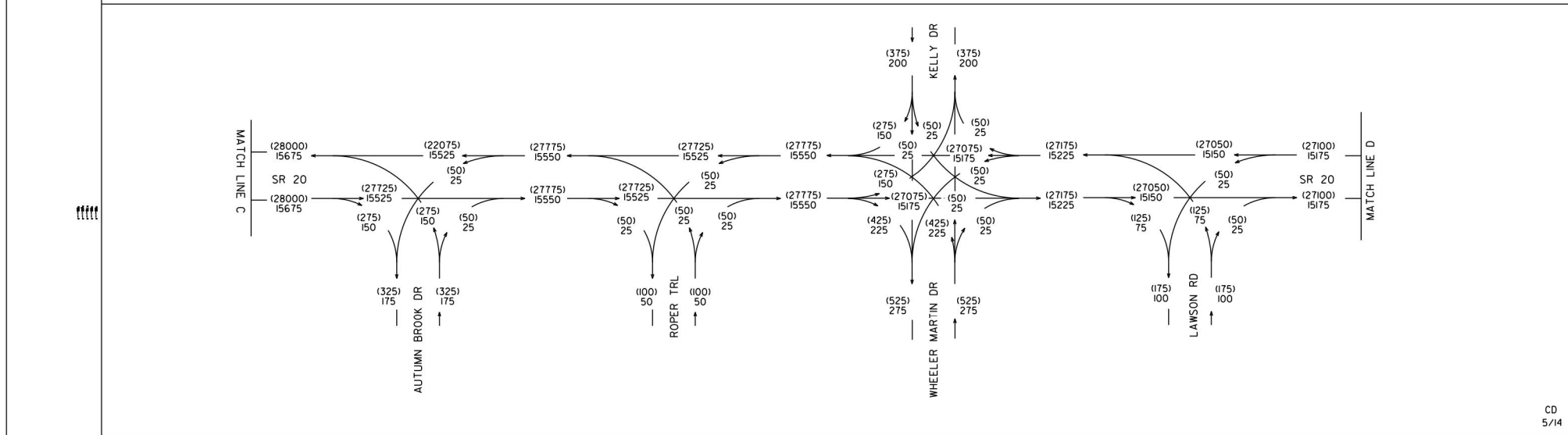
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COMB. = 5%

REVISION DATES

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.  
10-30

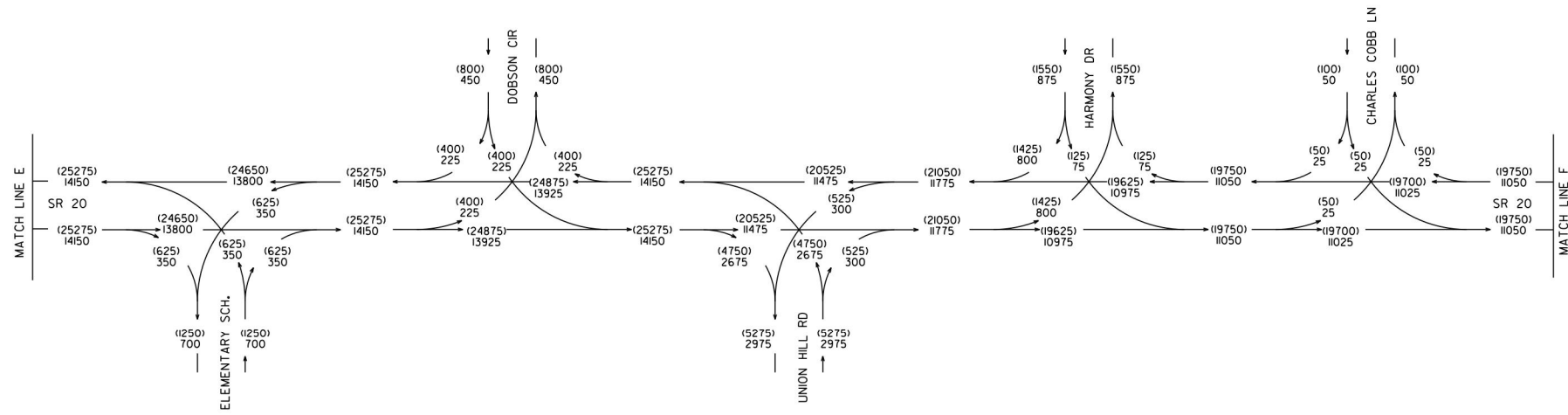
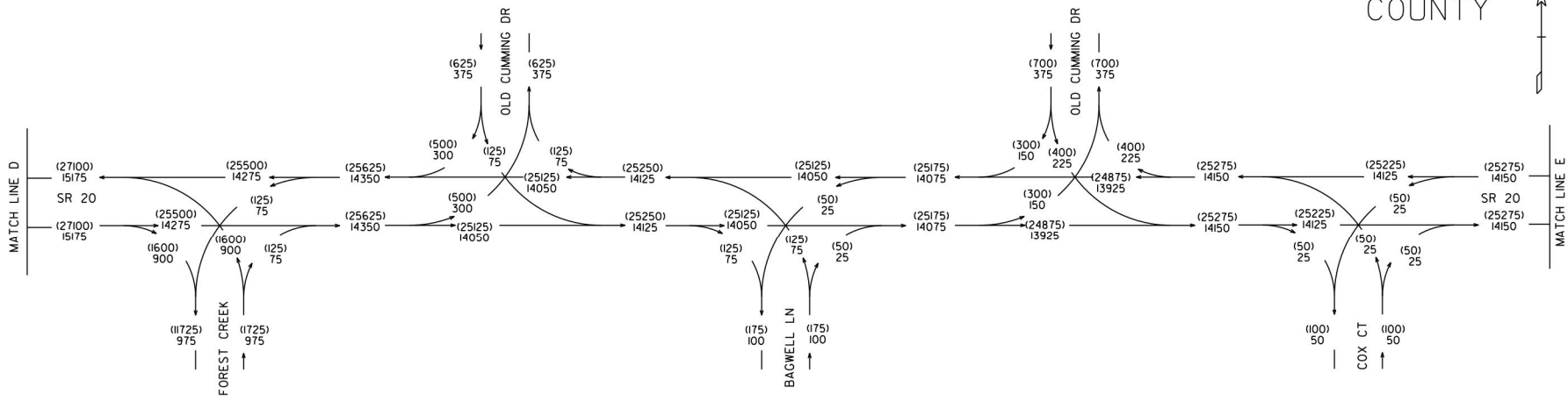




DRAWING No.  
10-32

CHEROKEE COUNTY

N



STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2045 BUILD ADT = (000)  
2025 BUILD ADT = 000

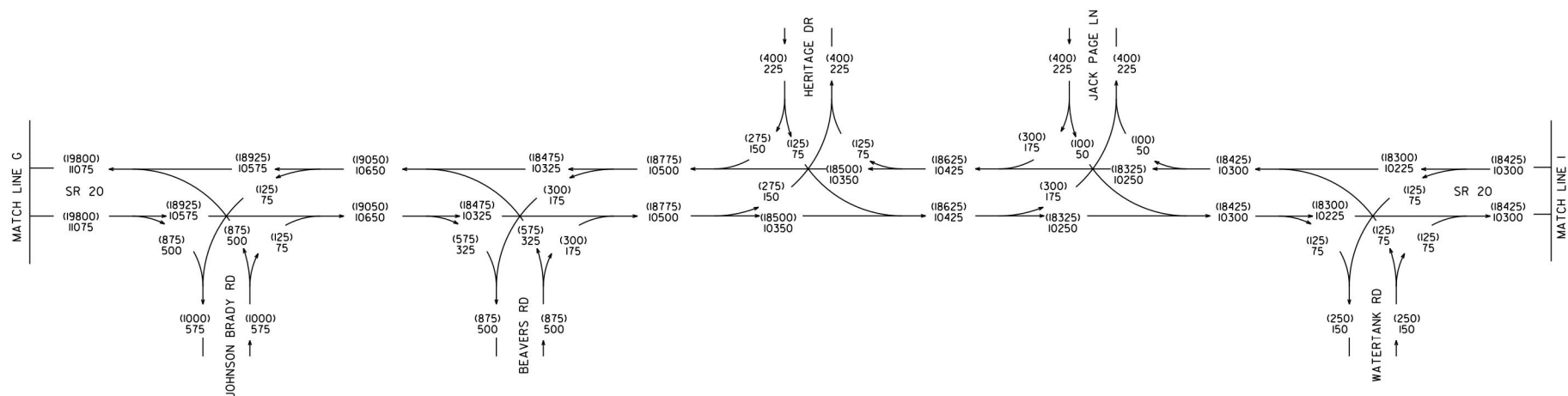
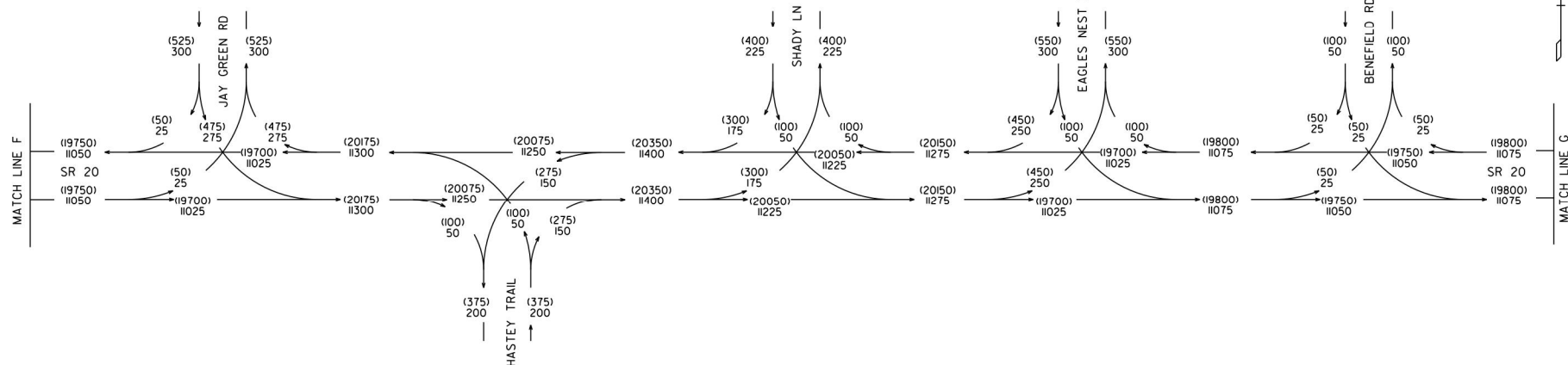
**GCA** INC.  
800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH. 404-355-4000 FAX 404-355-0604

24 HOURS T = 16%  
S.U. = 10%  
COMB. = 6%

REVISION DATES				STATE OF GEORGIA DEPARTMENT OF TRANSPORTATION	
				OFFICE: PLANNING	
				TRAFFIC DIAGRAM	
				DRAWING NO. 10-33	

DATE##	TIME##	#PRF##	#DCN##	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
#USER#		##PENTABLE##		GA	0003681	4	6

CHEROKEE COUNTY



CD  
5/14

STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2045 BUILD ADT = (000)  
2025 BUILD ADT = 000

**GCA** GCA, INC.  
800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH: 404-355-4000 FAX: 404-355-0604

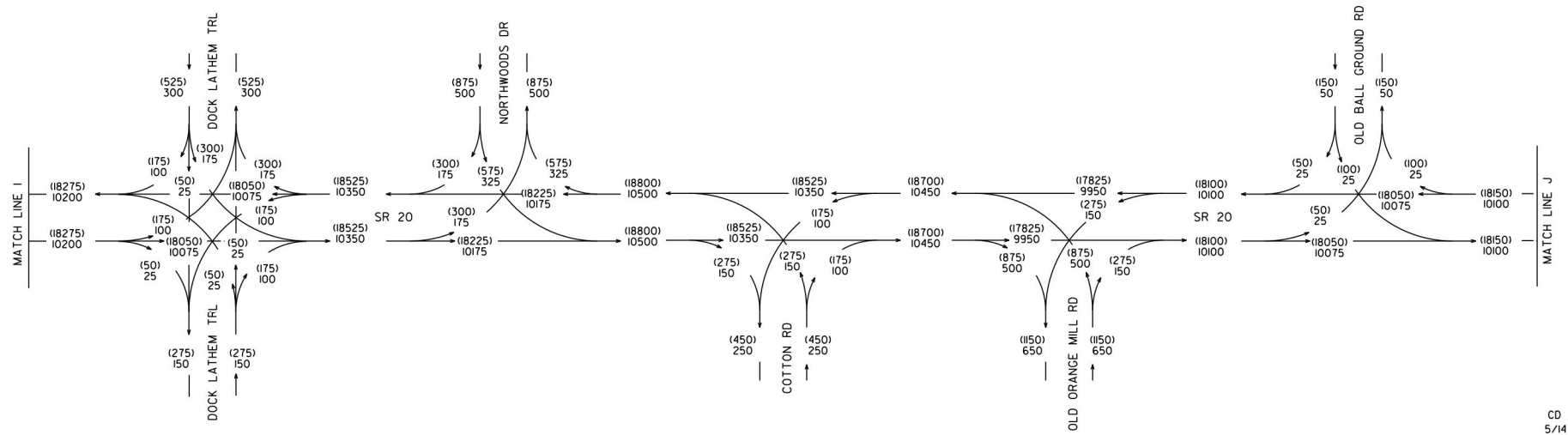
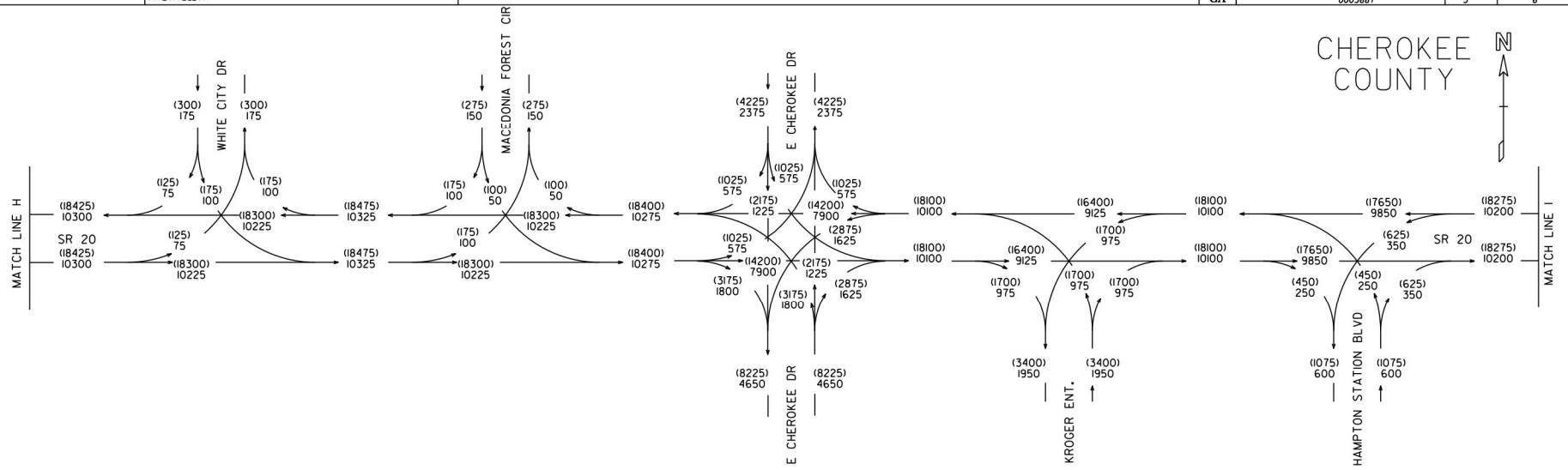
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S.U. = 10%  
COMB. = 6%

REVISION DATES


STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.  
10-34





STP00-0003-00(681)  
P.I.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

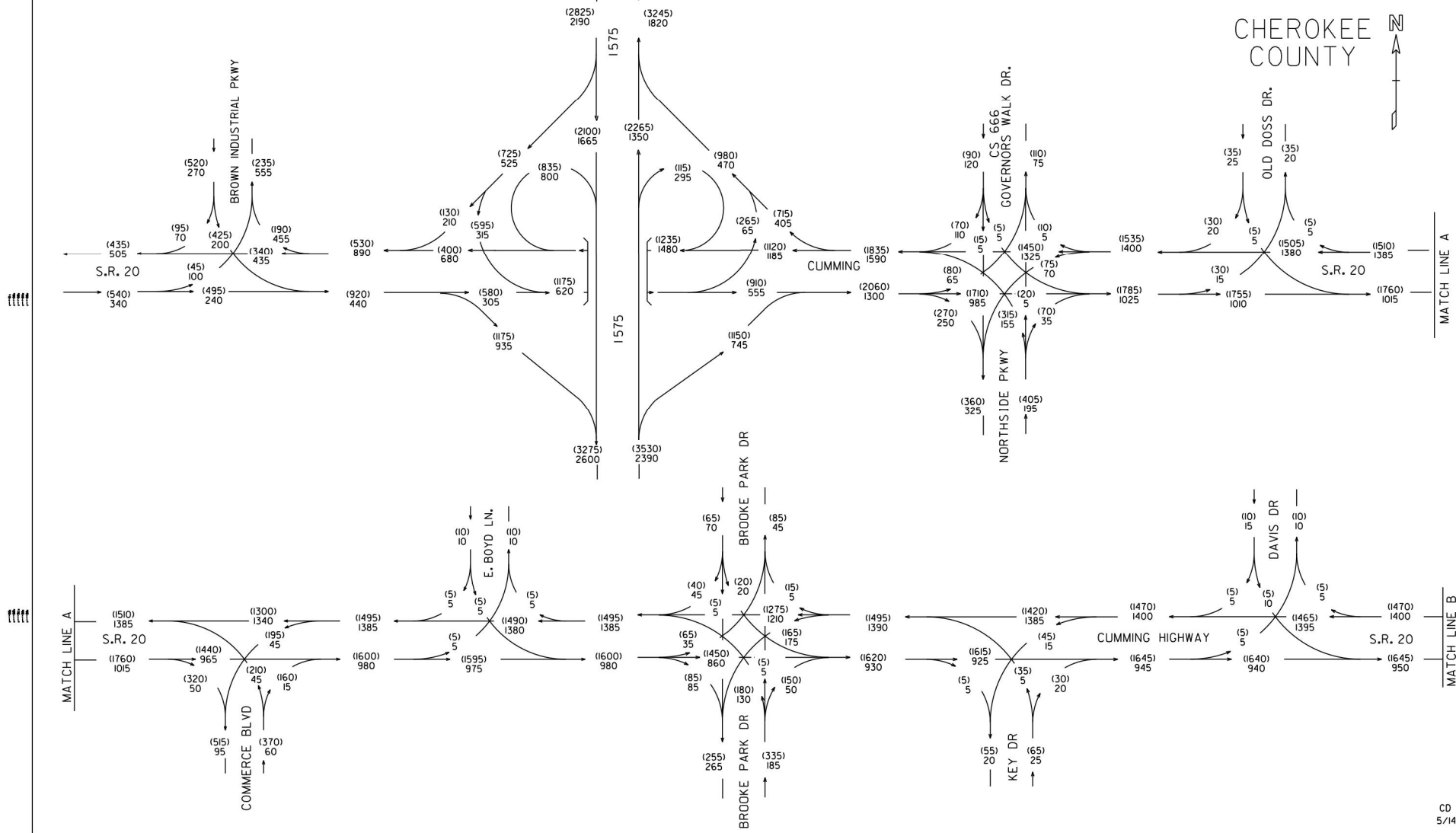
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2025 BUILD ADT = 000

**GCA** GCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH. 404-355-4000 FAX 404-355-0604

24 HOURS T = 16%  
S.U. = 10%  
COMB. = 6%

REVISION DATES				STATE OF GEORGIA DEPARTMENT OF TRANSPORTATION	
				OFFICE: PLANNING	
				TRAFFIC DIAGRAM	
				DRAWING No. 10-35	





CD  
5/14

STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2025 BUILD PM DHV = (000)  
2025 BUILD AM DHV = 000

**GCA** INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH. 404-355-4010 FAX 404-355-0604

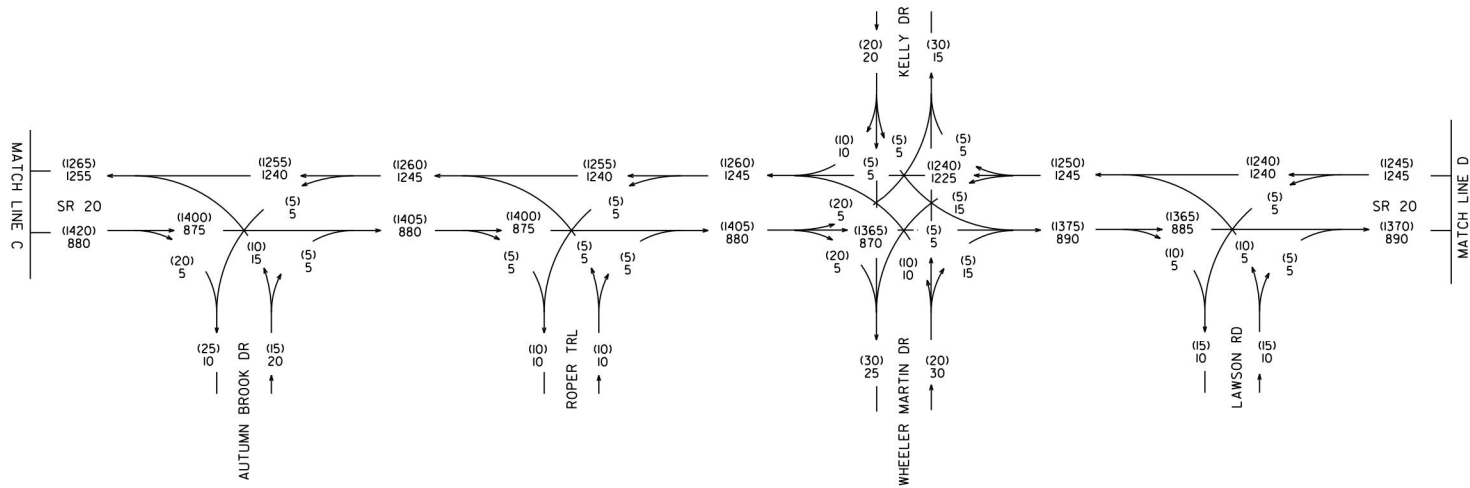
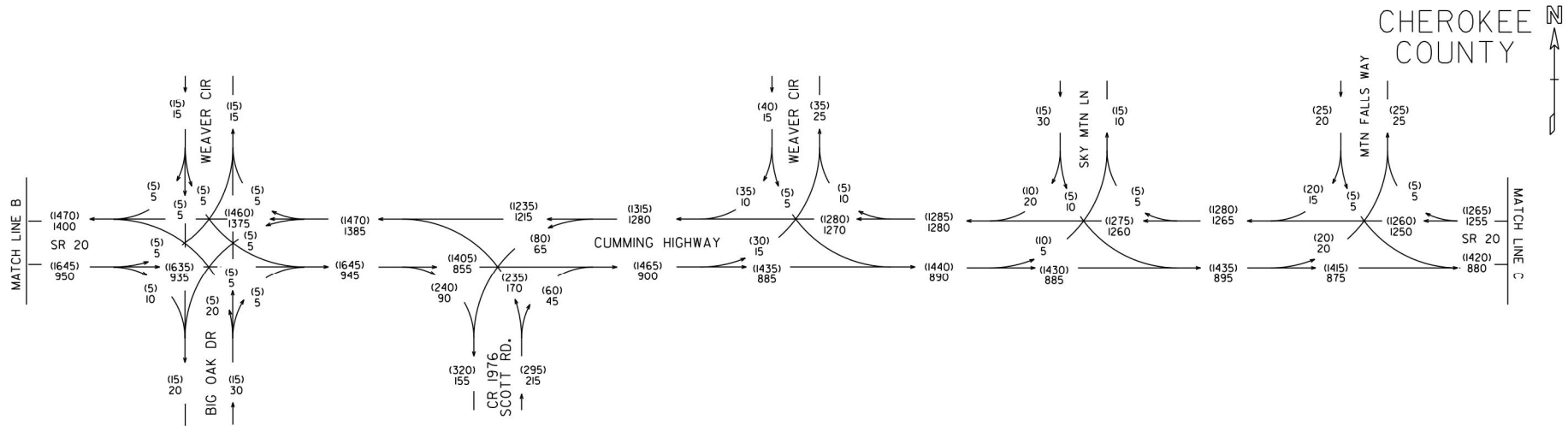
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COMB. = 5%

REVISION DATES		

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING No.  
**10-37**

DATE### #USER#	TIME### #PRF## ##PENTABLE##	#DCN#	STATE GA	PROJECT NUMBER 0003681	SHEET NO. 2	TOTAL SHEETS 6
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STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2025 BUILD PM DHV = (000)  
2025 BUILD AM DHV = 000

**GCA**  
GCA, INC.  
800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH: 404-355-4000 FAX: 404-355-0604

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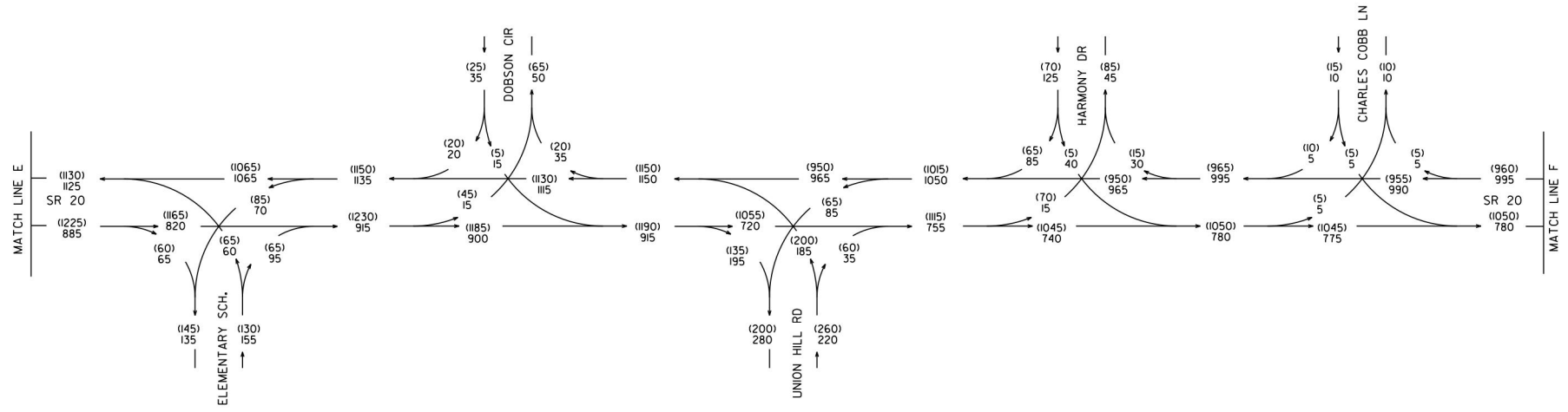
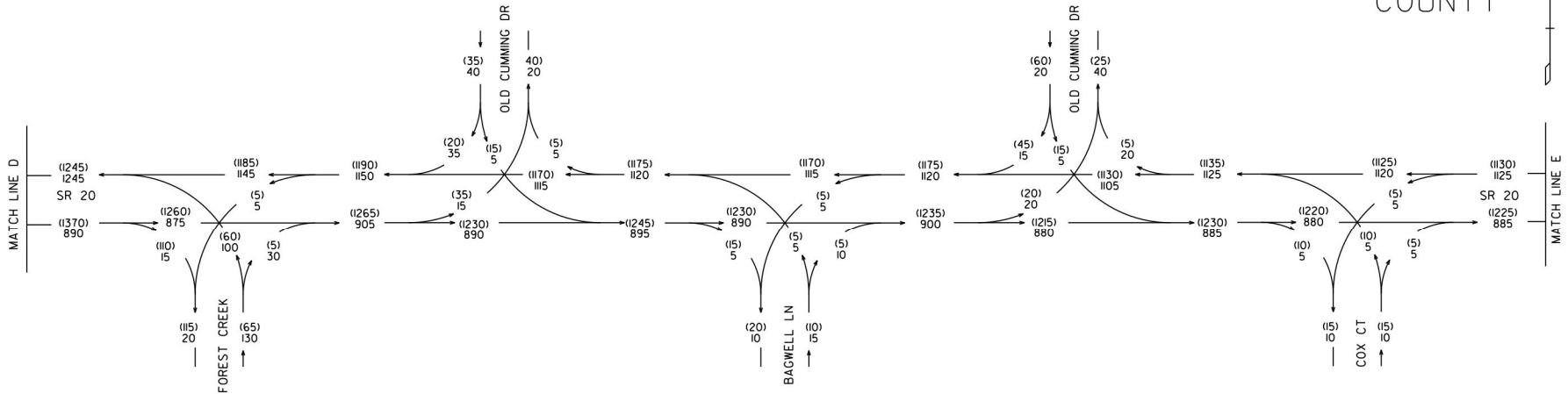
REVISION DATES

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

CD  
5/14

DRAWING NO.  
10-38

CHEROKEE COUNTY



CD  
5/14

STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2025 BUILD PM DHV = (000)  
2025 BUILD AM DHV = 000

**GCA** INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH: 404-355-4000 FAX: 404-355-0604

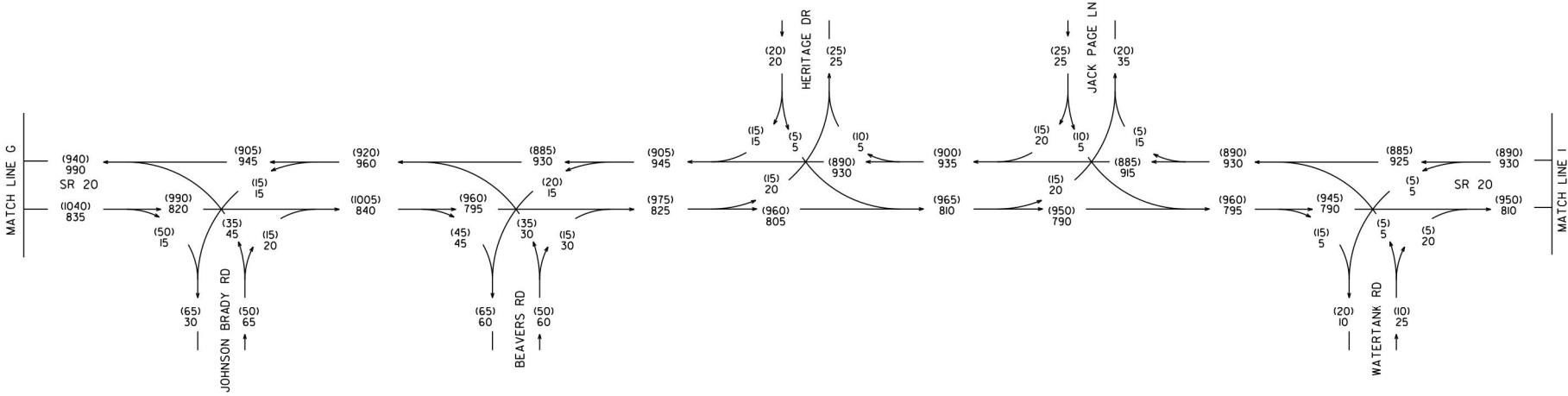
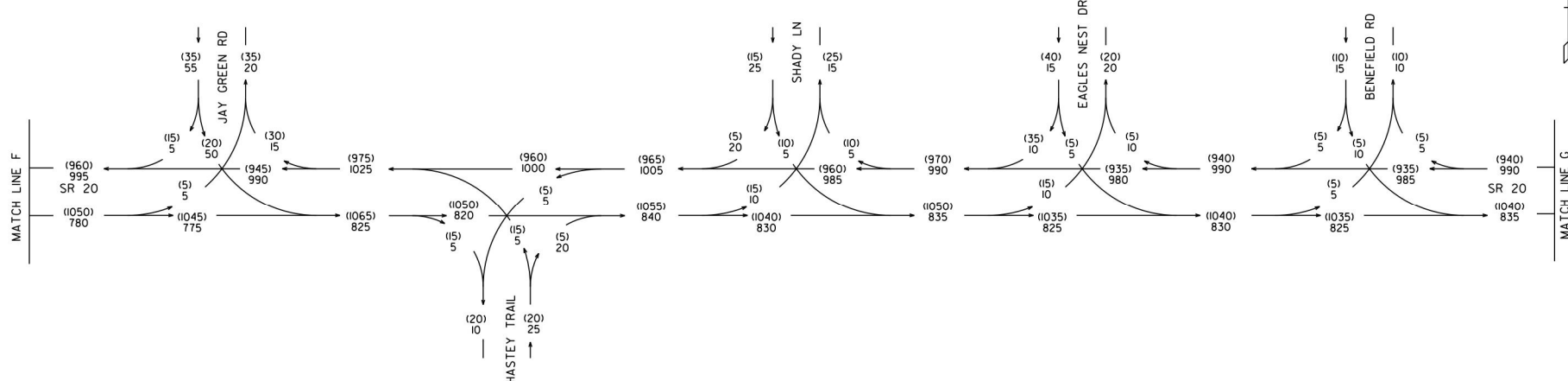
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S.U. = 7.5%  
COMB. = 5%

REVISION DATES	

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING No.  
10-39

CHEROKEE COUNTY



CD  
5/14

STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2025 BUILD PM DHV = (000)  
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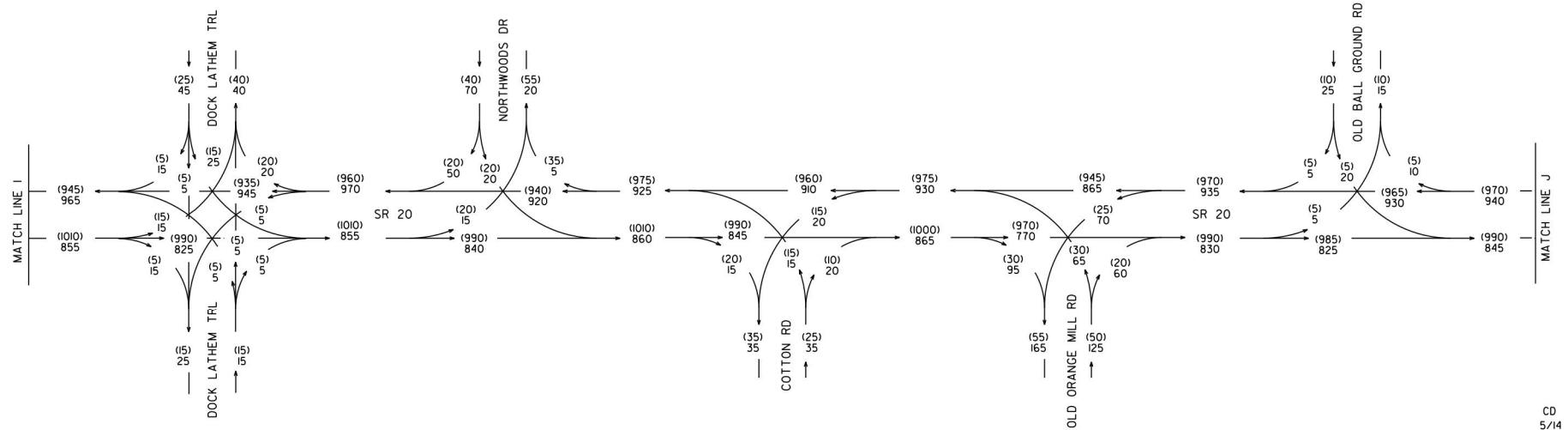
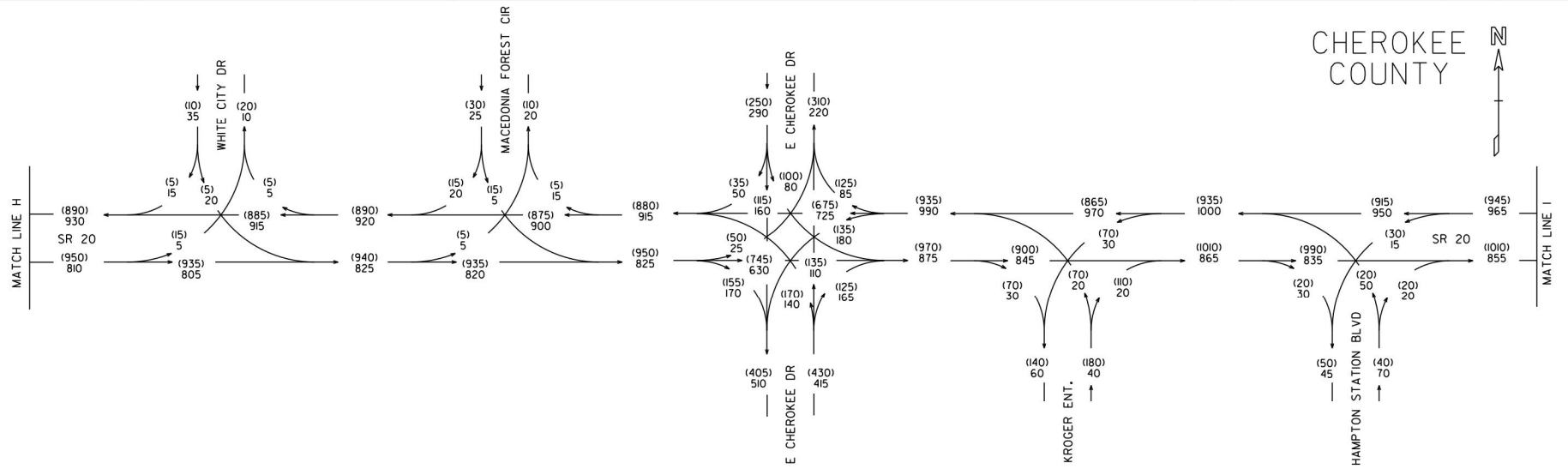
**GCA**  
GCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH 404-355-4000 FAX 404-355-0604

T = 12.5%  
S.U. = 7.5%  
COMB. = 5%

REVISION DATES	

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.  
**10-40**



STP00-0003-00(681)  
P.I.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2025 BUILD PM DHV = (000)  
2025 BUILD AM DHV = 000

**GCA** INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH. 404-355-4000 FAX 404-355-0604

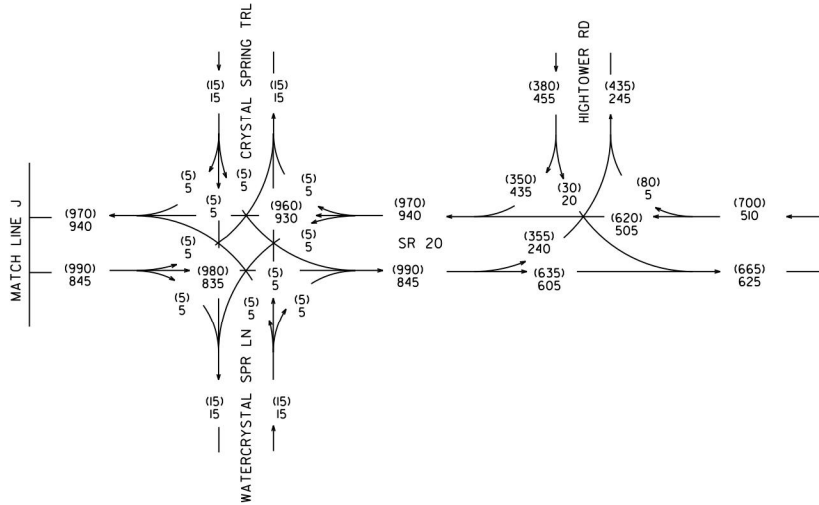
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COMB. = 5%

REVISION DATES


STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING No.  
10-41





CHEROKEE COUNTY

N

CD  
5/14

STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2025 BUILD PM DHV = (000)  
2025 BUILD AM DHV = 000

**GCA**  
GCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH: 404-355-4000 FAX: 404-355-0604

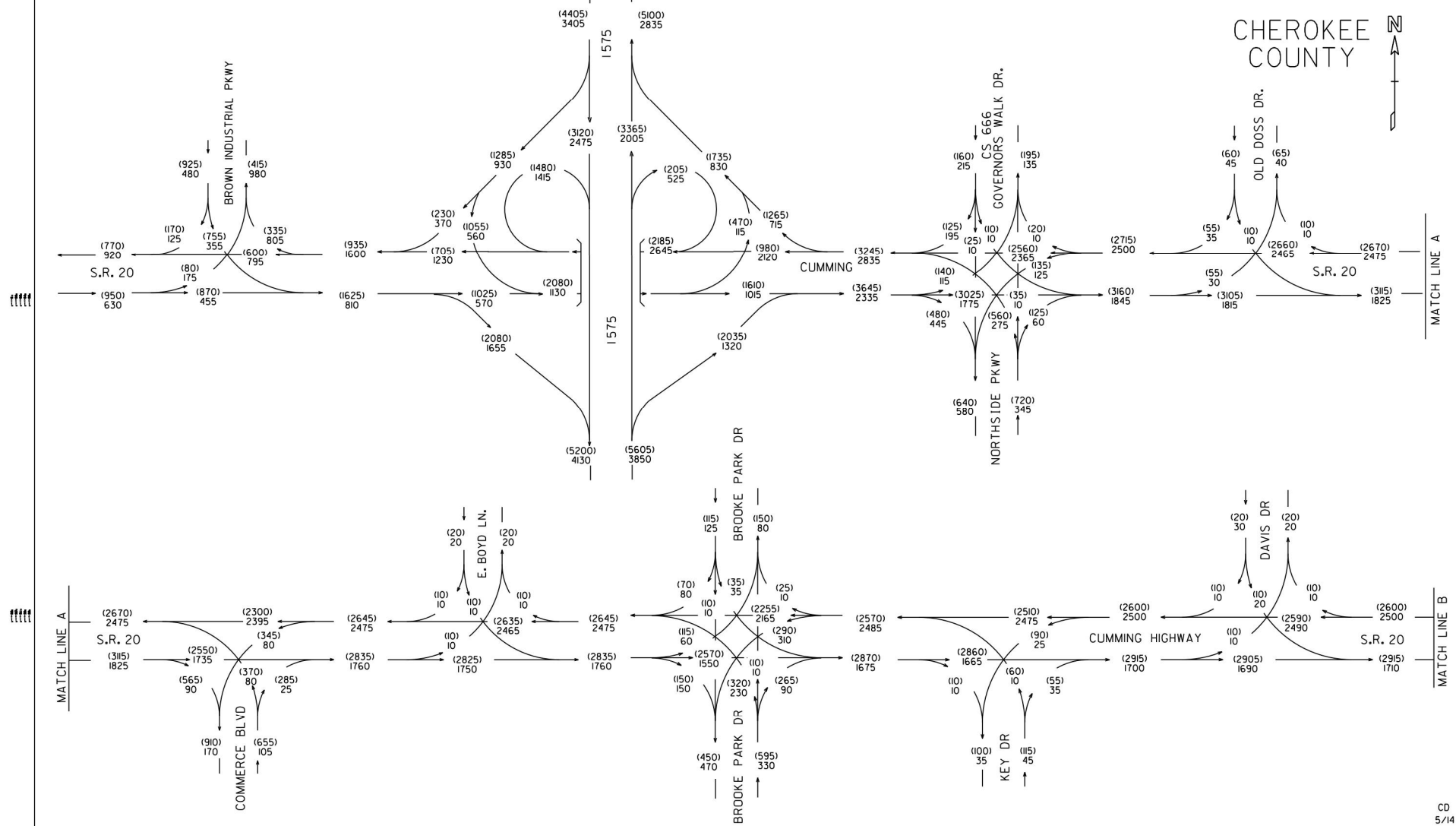
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REVISION DATES

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.

10-42



STP00-0003-00(681)  
P.I.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2045 BUILD PM DHV = (000)  
2045 BUILD AM DHV = 000

**GCA** GCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH. 404-355-4000 FAX 404-355-0604

T = 12.5%  
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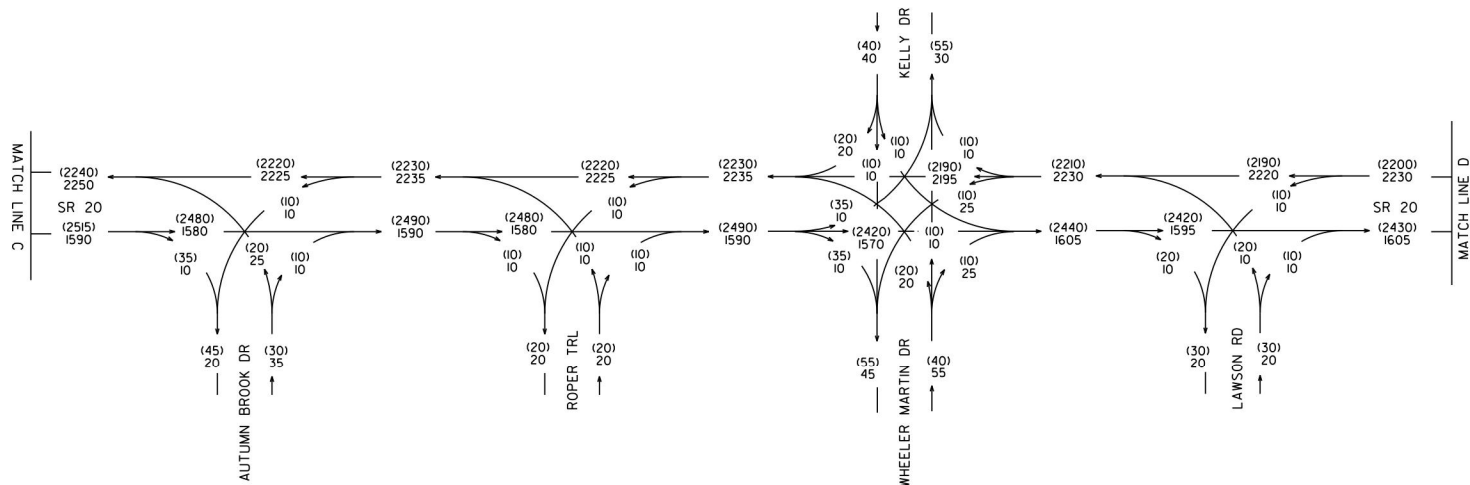
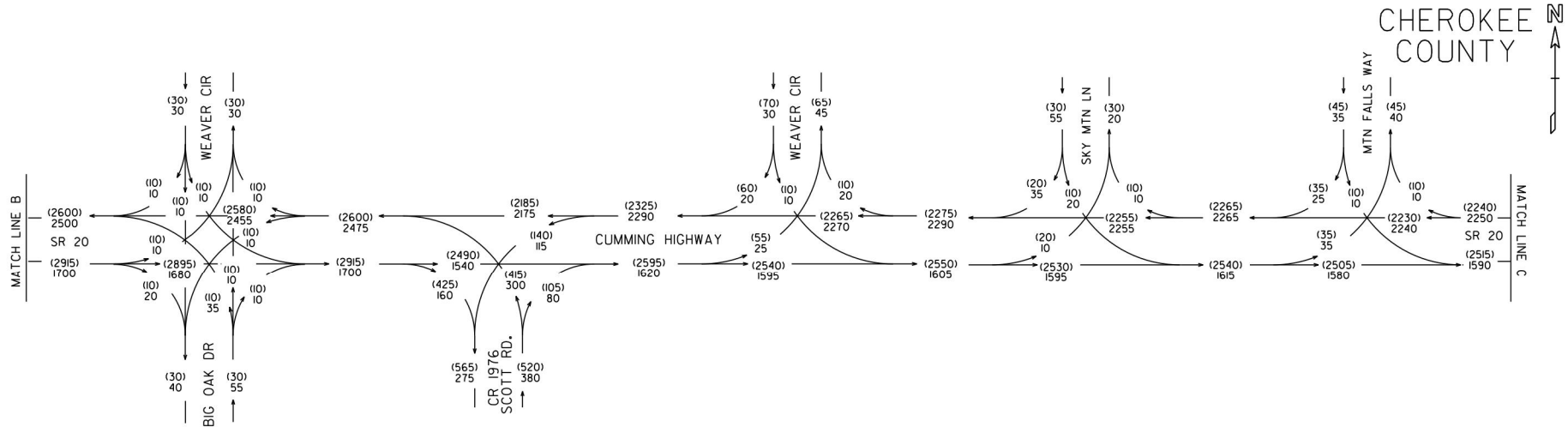
REVISION DATES


STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING No.  
10-43

CD  
5/14

DATE##	TIME##	SPRFS	DOG#	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
#USER#		##PENTABLE##		GA	0003681	2	6



CD  
5/14

STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2045 BUILD PM DHV = (000)  
2045 BUILD AM DHV = 000

**GCA**  
GCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH: 404-355-4000 FAX: 404-355-0604

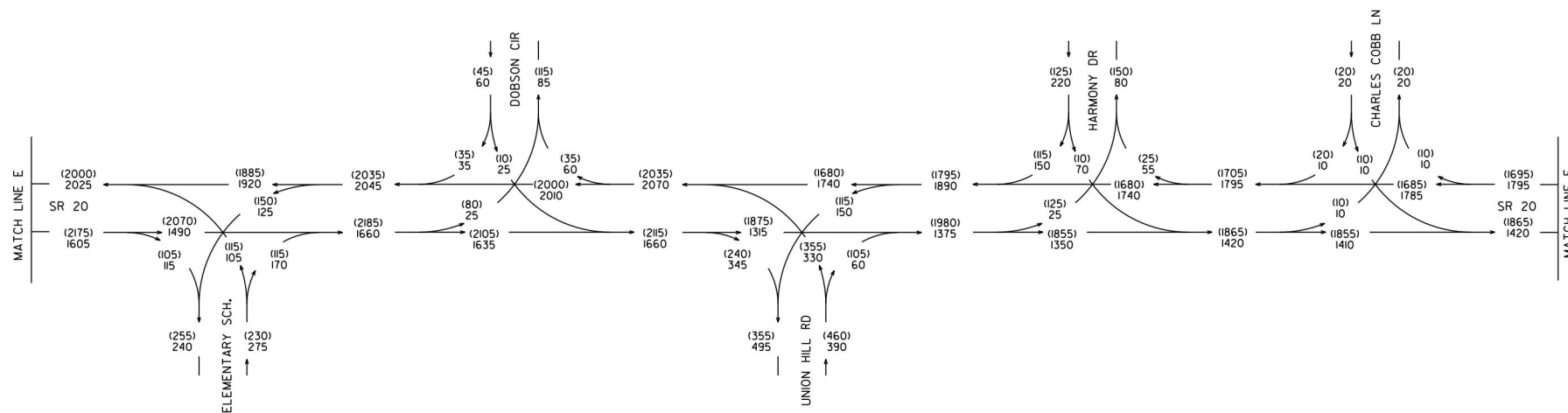
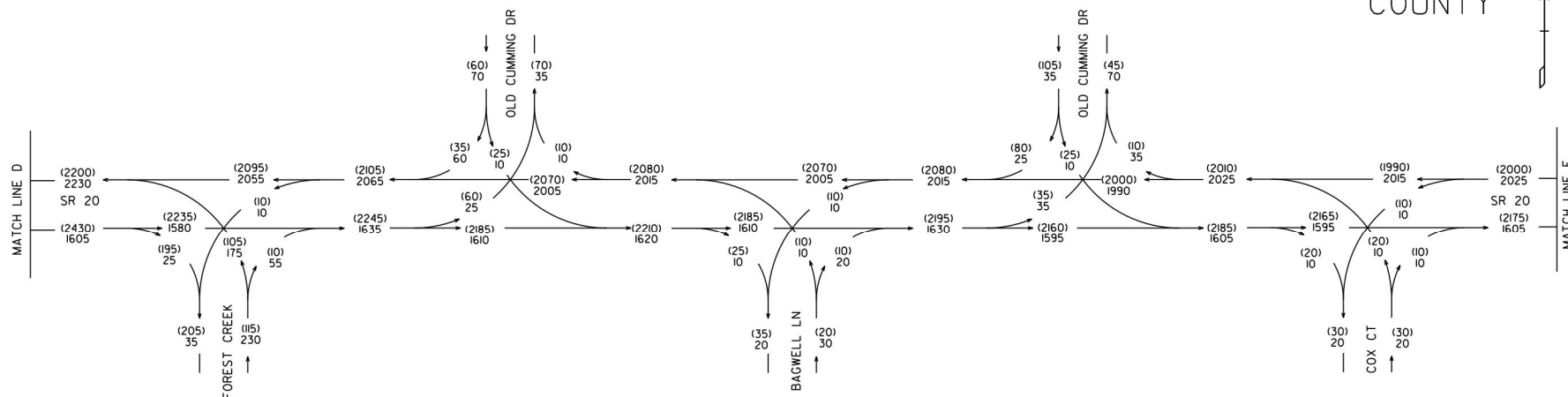
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REVISION DATES

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.  
10-44

CHEROKEE COUNTY



CD  
5/14

STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2045 BUILD PM DHV = (000)  
2045 BUILD AM DHV = 000

**GCA**  
GCA, INC.  
800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH 404-355-4000 FAX 404-355-0604

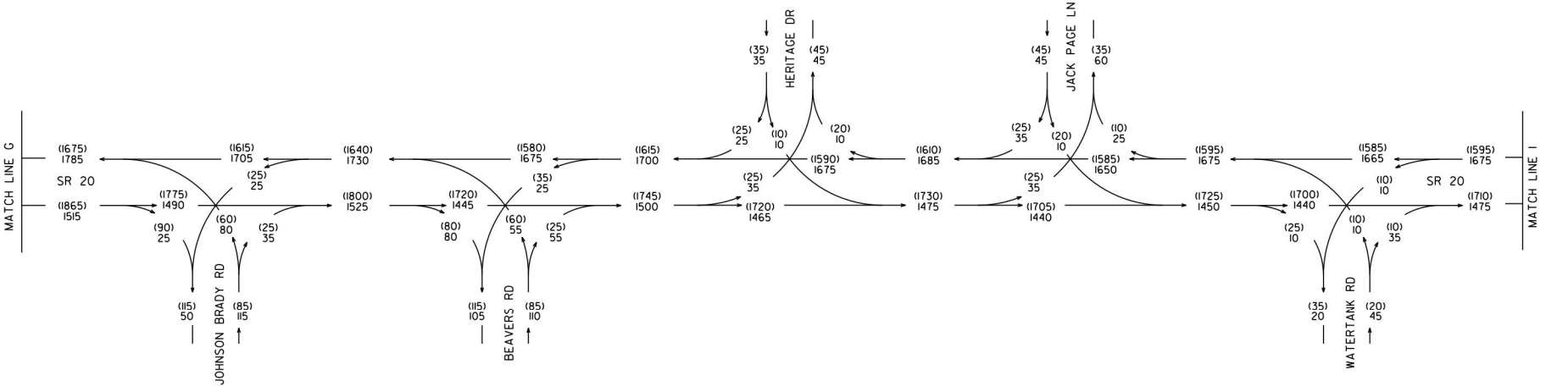
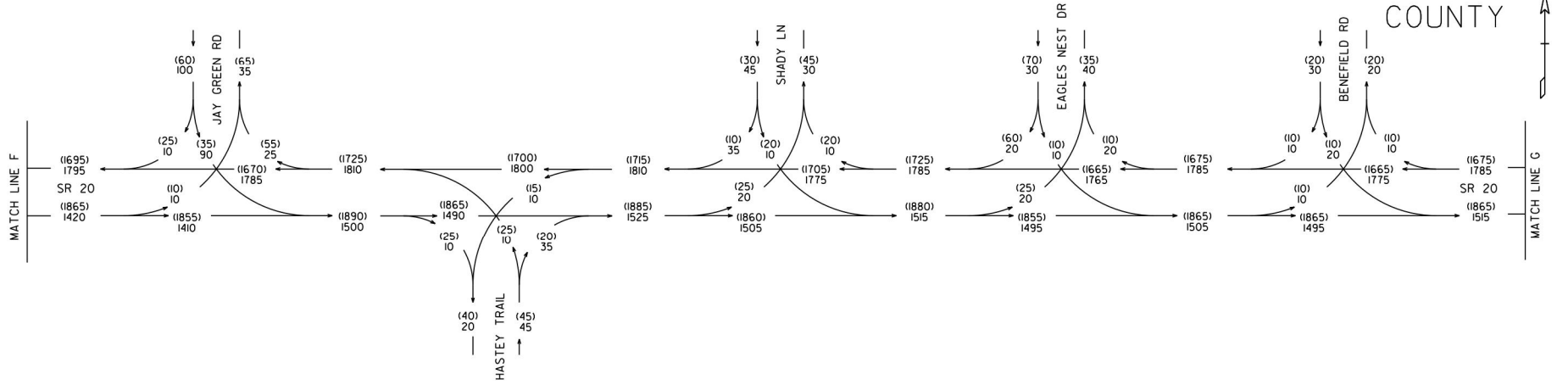
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COMB. = 5%

REVISION DATES

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.  
10-45

CHEROKEE COUNTY



CD  
5/14

STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2045 BUILD PM DHV = (000)  
2045 BUILD AM DHV = 000

**GCA**  
GCA, INC.  
800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH 404-355-4000 FAX 404-355-0604

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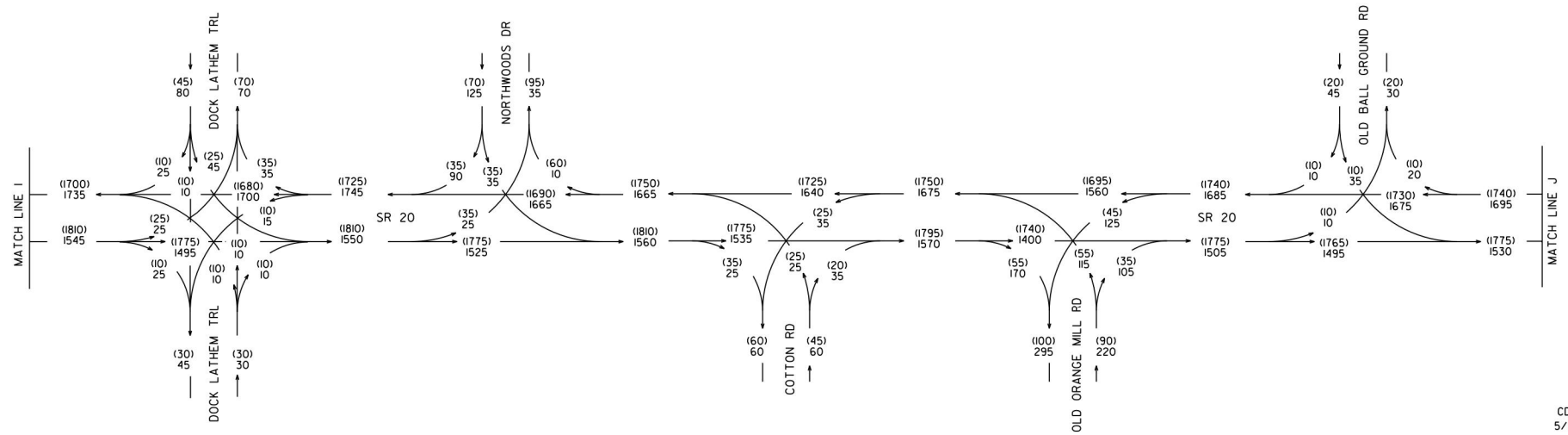
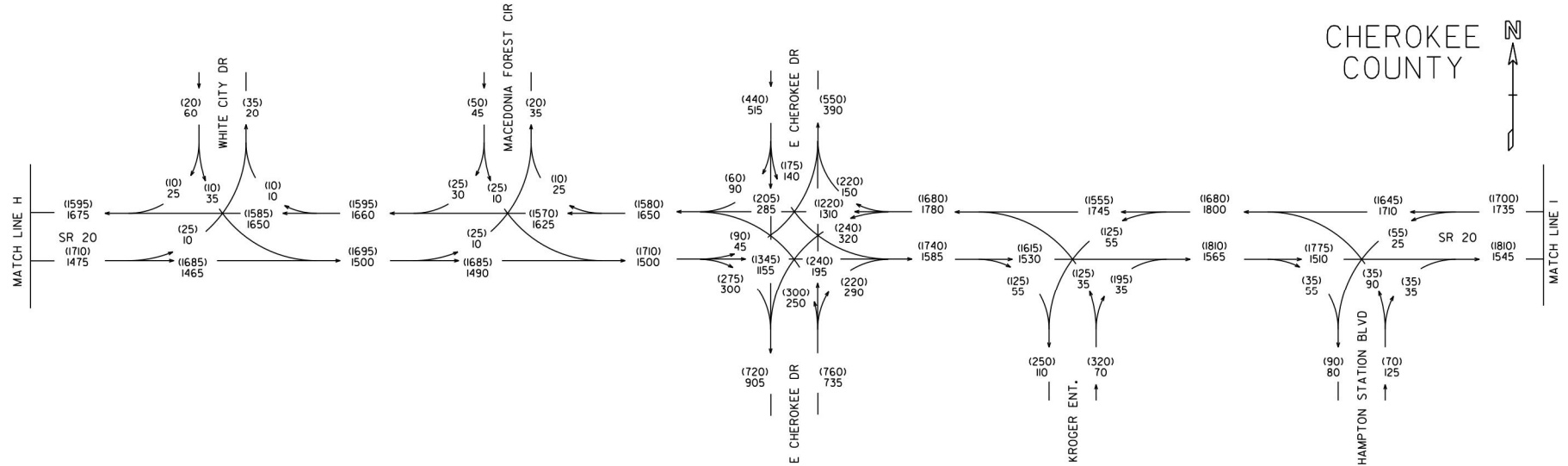
REVISION DATES

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.

10-46

DATE##	TIME##	PRF##	DOC##	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
USER#		SPENTABLE##		GA	0003681	5	6



STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2045 BUILD PM DHV = (000)  
2045 BUILD AM DHV = 000

**GCA**  
CCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH: 404-355-4000 FAX: 404-355-0604

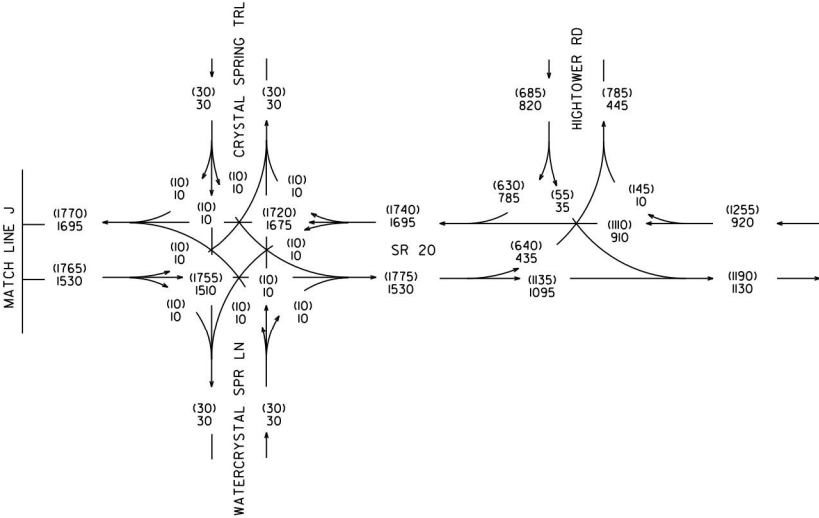
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REVISION DATES

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.  
10-47

CD  
5/14



CHEROKEE  
COUNTY

N  
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CD  
5/14

STP00-0003-00(681)  
P.J.# 0003681  
CHEROKEE COUNTY  
S.R. 20  
FM I-575 TO SR369

2045 BUILD PM DHV = (000)  
2045 BUILD AM DHV = 000

**GCA** CCA, INC.  
1800 PEACHTREE STREET, N.W.  
SUITE 825  
ATLANTA, GEORGIA 30309  
PH 404-355-4000 FAX 404-355-0604

T = 12.5%  
S.U. = 7.5%  
COMB. = 5%

REVISION DATES		

STATE OF GEORGIA  
DEPARTMENT OF TRANSPORTATION  
OFFICE: PLANNING  
TRAFFIC DIAGRAM

DRAWING NO.  
10-48



## **Attachment 6**

### **Roundabout Data**

## Dunnahoo, Lindsey

---

**From:** Law, Nicole <nlaw@dot.ga.gov>  
**Sent:** Tuesday, August 30, 2016 5:55 PM  
**To:** Gero, Scott  
**Cc:** Dunnahoo, Lindsey; Wood, Jeff  
**Subject:** FW: Request confirmation that GDOT does not consider roundabouts on 6-lane arterials - SR 20 Corridor

Scott,  
Please see the recommendation below in regards to your concerns about considering a roundabout in the 6-lane sections.

*Thanks,*

*Nicole S Law*

Phone: (404) 631-1723

Mobile: (404) 807-7424

---

**From:** Barry, Christina  
**Sent:** Tuesday, August 30, 2016 5:14 PM  
**To:** Law, Nicole  
**Cc:** Zehngraff, Scott E.  
**Subject:** RE: Request confirmation that GDOT does not consider roundabouts on 6-lane arterials - SR 20 Corridor

Hi Nicole,

We agree that the consultant does not need to consider roundabouts in the six lane section. However, we would recommend that they consider restricted crossing u-turns or median u turns as possible alternatives for these intersections. Please let me know if you have any additional questions.

Thanks!

**Christina D. Barry, PE**  
Traffic Operations Supervisor  
Office of Traffic Operations  
Georgia Department of Transportation  
935 E. Confederate Avenue, Bldg. 24  
Atlanta, GA 30316  
[cbarry@dot.ga.gov](mailto:cbarry@dot.ga.gov)  
Phone: (404) 635-2922

---

**From:** Law, Nicole  
**Sent:** Wednesday, August 24, 2016 5:25 PM  
**To:** Barry, Christina  
**Subject:** FW: Request confirmation that GDOT does not consider roundabouts on 6-lane arterials - SR 20 Corridor

Christina,  
Is this your area of expertise or does it go to Design Policy & Support? Do you mind pointing me in the right direction to answer the concerns of my consultant below?

*Thanks,*

*Nicole S Law*

Phone: (404) 631-1723

Mobile: (404) 807-7424

---

**From:** Gero, Scott [<mailto:Scott.Gero@aecom.com>]

**Sent:** Wednesday, August 24, 2016 3:59 PM

**To:** Law, Nicole

**Cc:** Dunnahoo, Lindsey; Wood, Jeff

**Subject:** Request confirmation that GDOT does not consider roundabouts on 6-lane arterials - SR 20 Corridor

Nicole,

Can you reach out to Scott Zehngraft or whoever is appropriate to confirm that we do not need to evaluate or consider roundabouts on a 6-lane arterial? I understand according to Chapter 8 of the DPM that if we have an ADT that exceeds 45,000 vehicles, then we no longer need to consider a multi-lane roundabout. This is the case from Scott Rd to Union Hill Road. However, east of Union Hill Rd to SR 369, we are proposing 6 lanes with an ADT less than 45,000 in our design year. Therefore, before we advance the concept report and not address or evaluate roundabouts in our 6-lane section, I just want to make sure that GDOT Traffic agrees that roundabouts do not need to be considered for 6-lane sections.

For a roundabout to be a reasonable solution, the opening and design year volumes for traffic entering the roundabout from the major road should be less than 90% of the total volume entering the roundabout.

**Table 8.1. Planning-level Thresholds for Single-Lane and Two-Lane Roundabouts**

No. of Circulatory Lanes	ADT <sup>1</sup> (design year)	% Traffic on Major Road (opening & design year)
Single-lane	< 25,000	< 90
Two-lane	< 45,000	< 90

<sup>1</sup>Based on traffic entering the circulatory roadway for a four-leg roundabout. A reasonable approximation for a three-leg roundabout is 75% of the values shown above.

<sup>2</sup>The volume of traffic entering the roundabout from the major road divided by the total traffic volume entering the roundabout, as a percentage.

If traffic volumes exceed the maximum ADT thresholds shown in Table 2.1 (i.e., 45,000 or more) or if site conditions are unfavorable to a roundabout, an acceptable conventional intersection may be selected without further evaluation. Nevertheless, a roundabout may still operate better than a conventional intersection and may be carried forward for more detailed consideration of a roundabout feasibility study.

Thank you,

**Scott A. Gero, P.E.**

Project Manager

**SR 20 Improvements from Canton to Cumming**

PI No's: 0014131, 0014132, 0014133, 0002862, 0003682

<http://www.dot.ga.gov/BuildSmart/Projects/Pages/1575SR400.aspx>

**AECOM**

400 Northpark Town Center  
1000 Abernathy Rd. NE, Suite 900  
Atlanta, GA 30328  
T 678.808.8800 F 678.808.8400  
[www.aecom.com](http://www.aecom.com)

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## **Attachment 8**

### **Minutes of Concept Meeting**





## SR 20 Improvements from Canton to Cumming

PI No's: 0014131, 0014132, 0014133, 0002862, 0003682

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AECOM Proj.: 60507210 (File 60267130)

## Meeting Agenda

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**Subject:** Concept Team Meeting for SR 20 Corridor Improvements (Canton to Cumming)

**Date:** March 10, 2017

**Location:** GDOT – Rm 409

**Attendees:**

Cynthia Burney – GDOT  
Nicole Law – GDOT Outgoing PM  
Cleopatra James – GDOT Incoming PM  
Scott Gero – AECOM PM  
Laura Dawood – AECOM Environmental  
Lindsey Dunnahoo – AECOM Engineer  
Paola Rojas – AECOM Engineering  
Chad Bishop – AECOM Engineer  
Chandria Brown – GDOT  
R Lawrence – GDOT Planning  
Angela Turner – GDOT Design Policy  
Aaron Burgess – GDOT NEPA  
Chris Raymond – GDOT TMC  
Jim Pomfret – GDOT OES  
Walt Taylor – GDOT Engineering Services  
Erik Rohde – GDOT Engineering Services  
Chuck Hasty – GDOT Engineering Services  
Chesleion Charles – Southern Company Gas

**District 1:**

Tina Apperson – GDOT  
Lynn Palmer – GDOT Utilities  
Kevin York – GDOT R/W  
Harold D. Mull – GDOT DCE  
Pete Hughes – SEMC  
Ted Brown – SEMC  
Mike Souther - Windstream

**District 6:**

Barry Hensley – Assistant Construction Manager  
Bethany Watson – Assistant City Engineer (Canton)  
David Hatabian – City Engineer (Canton)  
Geoff Morton – Cherokee County  
Jennifer Deems – GDOT Utilities  
Duane Fant – District 6 R/W  
Dee Carson – District 6 Traffic Ops  
David Acree – District 6 Pre-Construction  
Keith Day – District 6 Area Mngr  
Brian Whelchel – District 6 Asst Area Mngr  
Grant Waldrop – GDOT Traffic Ops  
John Gay – Engineer (Georgia Power)  
Drace Farrell – Engineer (Windstream)

- 
- Introduction of SR 20
    - Map – See Attachment 1
    - History
    - Screen 2 Alternatives => Widen Existing – See Attachment 2.
    - Accelerated Schedule
      - Streamlined PFPR in April
      - Right of Way in June
      - Standard PFPR in late fall/early winter
    - Modified PDP
  - Concept Report
    - Proj Justification
    - Need & Purpose
    - Traffic / Lane Call
      - See Attachment 3 for laneage demand determination
      - Goal: LOS D for entire corridor
    - Functional Classification – See attachment, Urban/Rural Arterial.
    - Typical Section:
      - Urban vs. Rural – Urban typical to minimize impacts, to stay consistent with the development patterns in the area, and help with MS4 design.
      - Drainage/MS4 – project approach – Approach is to catch, treat, and detain all water that falls on the road. Offsite runoff will be conveyed in a separate system from the onsite runoff.
      - 11' & 12' lanes
        - Per VE study recommendation, the typical section is being revised to 11' inside lanes and a 12' outside lane.
        - Harold recommended one 11' inside lane, with a 12' middle and outside lane for truck accommodations.
      - Sidewalk and multi-purpose paths – Cherokee County has a planned trail from Cherokee Veterans Park to Smithwick Creek. Forsyth County has a planned trail from Spot Rd to Sawnee Mountain as well as on the east side of Post Rd.
      - Pavement Design – Rigid vs. Flexible – PES & PTS requested in August, 2016.
    - Design Speed: 45 vs 55 mph => Escalation Memo or Concept Report
      - AECOM will put together an escalation memo for 45 mph
        - Cynthia went to the public meeting hosted by Senator Brandon Beach. A question was raised about the speed limit on the road and the public seemed OK with 45 mph.
    - Draft VE Study Recommendations – See Attachment 4 for draft responses to the draft recommendations. The team is waiting for the final VE Study Report.
    - Utilities – SUE
      - How can we accelerate utility conflict resolution (relocation determination) to achieve comfort with R/W needs by June?
        - Dictate where utilities can go on a typical section
        - Hold workshops for utility coordination with each district and invite utility owners
    - R/W: (120' – 250'+)



- Encompass all needs as R/W or only to Shoulder Breaks and easement beyond? Both District 1 and District 6 prefer to have right of way everywhere to make it easier for utility relocations. Right of way should be evaluated on a case by case basis (i.e. use easements to save a parking lot).
  - Kevin York will be coordinating right of way for the entire corridor
- Access Control/Innovative intersections:
  - R-Cuts
  - Median U-turns (Michigan Lefts) – SR 371/Post Rd, Bethelview Rd
- Context Sensitive => NEPA => Avoid, Minimize & Mitigate
  - Meeting with USACE 3/16 to review alignment (USACE is lead federal agency due to need for permit to impact Waters of the US)
- ROLL PLOTS of Concept Layout
  - School Drwy Access at Freehome Elementary
    - Geoff will coordinate with Freehome Elementary about access - AECOM to send PDF.
  - Drwy at McDonald's
    - All agreed with closing the Dec 2016 PIOH proposed right in/right out driveway at McDonalds. There is not enough room to add a deceleration lane without significant displacement of parking spots adjacent to the road. Three access points will still be viable from E Cherokee Drive as well as from two location from SR 20 (at light to Kroger and one right in/right out drwy at east end of strip plaza. There is interparcel access currently available and it will remain with the proposed improvements.
- Environmental Permitting:
  - USACE (Lead Agency) – PAR - Submitted 3/1
  - Public Involvement – Next PIOH after Streamlined PFPR and before R/W (May). The intent is to show the actual propose R/W and easements to allow for one final look and comment to tweak before finalizing R/W Plans.
- Construction: (Constructability to be combined w Streamlined PFPR)
  - The majority of the project should be pretty straightforward to construct as we are widening a 2-lane to a 6-lane divided. This large widening will create plenty of space and opportunity to maintain traffic on one side while constructing the other side. There are not many changes proposed to the mainline profile which further simplifies staging.
  - Show cross sections with retaining walls and staging cross sections at critical stations.
  - Detours will not be needed for mainline construction. Some side roads may need detours (TBD) running traffic on temporary gravel surfaces.
- Other
  - Concern that the signals at East Cherokee and Kroger are too close. Per district traffic, the signals are close enough to be co-ordinated and are not an issue. These are existing signal locations.
  - Evaluate the pond in the southwest corner of the Union Hill intersection to see if it can be moved closer to SR 20 (there is a planned development in this parcel)
  - Add the multi-use trails to the typical sections in the Concept Reports

- OK to cut off Franklin Goldmine from SR 20 (cul-de-sac near SR 20)
- Angela asked about the intersections that are shown to fail in the design year. AECOM to determine what year these intersections will fail.
- Pipe Clearance – Need a variance to reduce clearance requirements
  - Up-class the pipe
  - Steel en-case the pipe
  - Switch to an elliptical pipe
- Add TIP #FT-313 to concept report for 0003682

## MEETING SIGN-IN SHEET

Project: SR 20

Meeting Date:

March 10, 2017

Facilitator: Nicole Law/Scott Gero

Place/Room:

OGC 409

Name	Company	Phone	E-Mail
Laura Dawood	AECOM	770.548.9904	Laura.dawood@aecom.com
Chad Bishop	AECOM	404-965-7050	chad.bishop@aecom.com
Cynthia Burney	GDOT	4-631-1851	cburney@dot.ga.gov
Nicole Law	GDOT	4-631-1721	nlaw@dot.ga.gov
Cleopatra James	GDOT	4-631-1546	cjames@dot.ga.gov
CHESLEIGH CHARLES	SOUTHERN COMPANY GAS	4-584-3257	ccharles@southernco.com
Chandra Brown	GDOT	4)631-1580	chbrown@dot.ga.gov
ROSHNI LAWRENCE	GDOT-PLANNING	404-631-1774	RoshniLawrence@dot.ga.gov
Angela Turner	GDOT Design Policy	404)631-1736	anturner@dot.ga.gov
Scott Gero	AECOM	404) 965-9726	scott.gero@aecom.com



**Project: SR 20****Meeting Date:**

March 10, 2017

**Facilitator: Nicole Law/Scott Gero**

**Place/Room:**

OGC 409

[illegible]

Co	Name	eMail	Phone
SEMC	Pete Hughes	Pete.hughes@samnee.com	678-455-1393
SEMC	TED BROWN	TED.BROWN@SAWNEE.com	678-455-1552
GDOT	Tina Apperson		
GDOT-Util.	Lynn Palmer	jlpalmer@dot.ga.gov	770-531-5752
GDOT-R/W	Kevin York	Keyork@dotagov.gov	770-531-5384
GDOT-DCE	NAROLD D. MULL	nmulle dot.ga.gov	770-531-5769
WINDSTREAM	MIKE SOUTHER	MIKE.SOUTHER@WINDSTREAM.COM	404-831-0415

**Concept Team Meeting- Cherokee Co 0003681**

### 3/10/2017 Sign In

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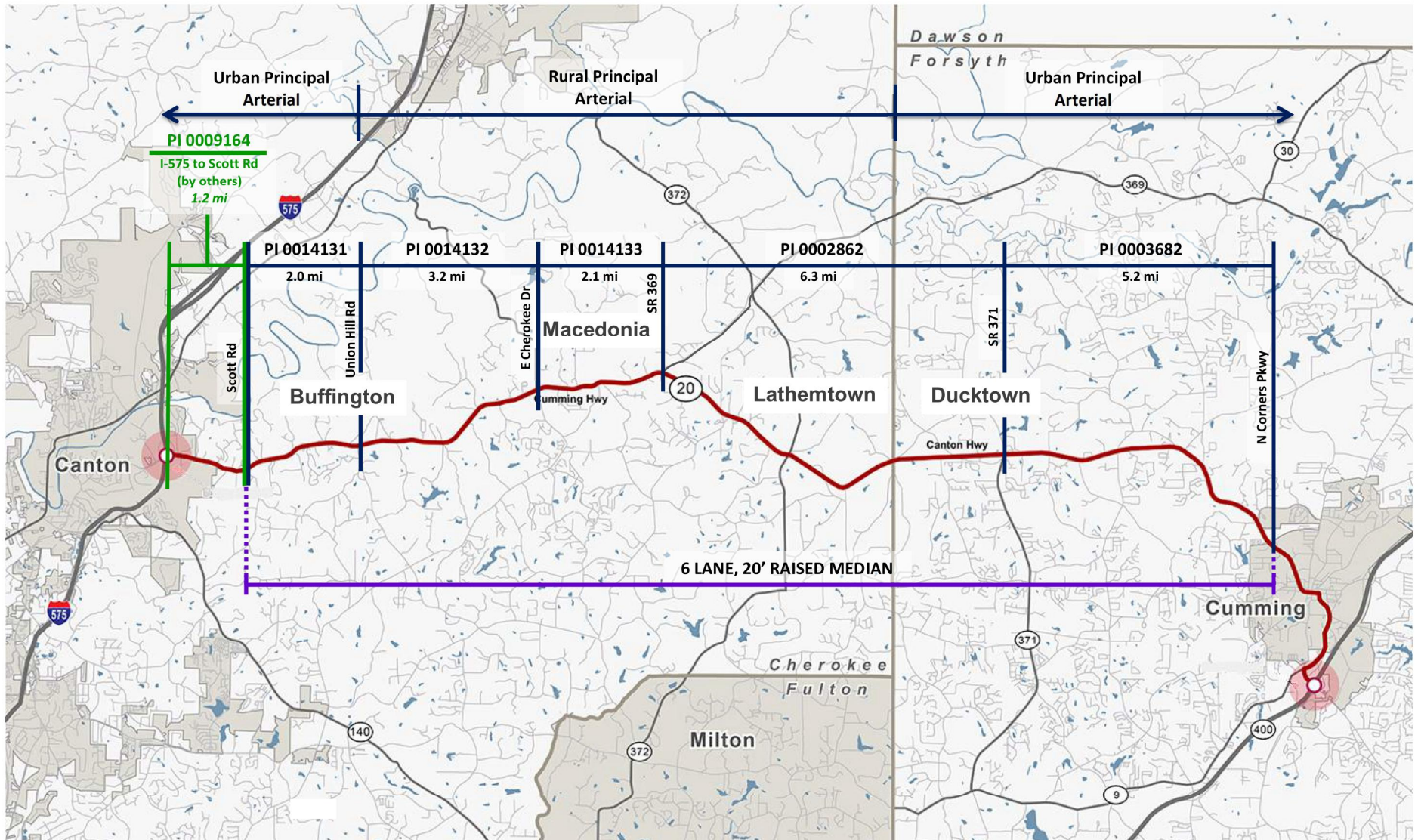
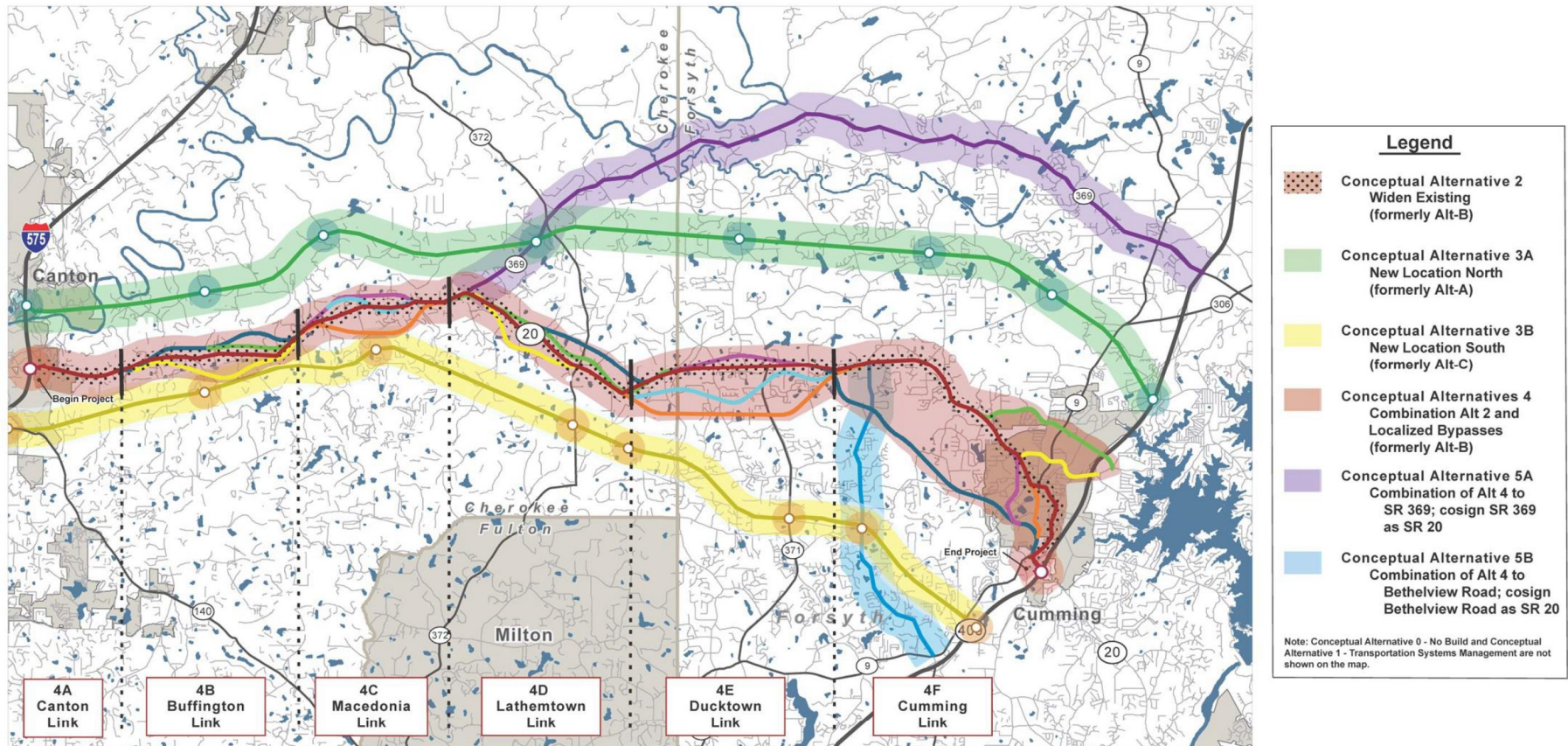




Figure 1.3 Conceptual Alternatives





Peak Hour Directional, Transitioning Areas, State Signalized Arterials, Class I (40 mph or higher posted speed limits), LOS D Criteria									
1460	- 4 Lanes Needed	Growth Rates:	I-575 to SR 369		SR 369 to SR 371		SR 371 to Cumming		
3200	- 6 Lanes Needed		3.4%	2014-2025 Growth Rate (I-575 to SR 369)	3.6%	2014-2025 Growth Rate (SR 369 to SR 371)	3.6%	2014-2025 Growth Rate (SR 371 to SR 400)	
4880	- 8 Lanes Needed		2.9%	2025-2045 Growth Rate (I-575 to SR 369)	3.0%	2025-2045 Growth Rate (SR 369 to SR 371)	3.2%	2025-2045 Growth Rate (SR 371 to SR 400)	



## Meeting Minutes

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**Subject:** Discussion with OPD on preliminary VE Study Recommendations

**Date:** March 3, 2017, 9:30 am

**Location:** GDOT 25<sup>th</sup> floor, OPC Conf Rm

**Invitees:** GDOT: Nicole Law (PM), Albert Shelby (State Program Delivery Administrator)  
Project Team: Scott Gero (PM), Lindsey Dunahoo (Lead Eng), Paola Rojas (Eng)

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**Review of the Draft VE Study Recommendations** - The VE Study was completed this week. Today's meeting is for the project team to go over the preliminary recommendations with the Office of Program Delivery to determine draft responses and direction forward on the various recommendations.

1.0 – Reduce from 6 to 4 lanes from Union Hill Rd to SR 371 (PI #'s 0014132, 0014133, 0002862) – No, we do not agree with implementing this recommendation. GDOT upper management has determined that the design will proceed with 6 lanes.

2.0 – Reduce lane widths from 12-feet-wide to 11-feet-wide – We agree that reducing the design to 11 foot lanes will reduce impacts to adjacent resources and will still provide an adequate facility for vehicular flow however, we only agree to a portion of this recommendation. See the next issue and response.

2.1 – Reduce inner 2 lane widths each direction from 12-feet-wide to 11-feet-wide (outside lane width each direction remains 12 ft). We agree to implement this recommendation. We feel that the outside lane should provide the full 12 ft lane width to accommodate tractor trailers on this truck route. The reduction in lane width of the two inner travel lanes will help reduce:

- The footprint and impacts to the adjacent parcels and resources
- The amount of runoff that needs to be treated and detained to meet MS4 and Drainage Design Policies.
- The distance pedestrians have to cross at intersections and therefore reducing the phases necessary for this movement.
- The cost through savings in materials needed for construction and maintenance of the roadway.

3.0 – Reduce median width from 20 ft to 16 ft – No, we do not agree with implementing this recommendation. The project proposes to provide a 6-lane section (3-lanes in each direction). GDOT Policy states that full median breaks are not allowed at side roads or access points unless there is a signal warranted and installed. Due to the 6-lane section, Restricted Crossing U-Turns (R-Cuts) will be installed to manage access and limit to one-way operation through the median. The design of the R-Cuts require that positive median separation (a raised median) be provided to manage traffic and discourage wrong way use of the opening. Although the VE Study team has developed a sketch of a way to provide a reduced section in the 16 ft median which consists of an 11 ft turn lane and back to back curb and gutter to provide a positive median separation, studies of other projects using similar raised median width reductions have found negative consequences with this reduced design width. Negative issues identified include:

- Reduced visibility of narrow raised median incurring impacts due to vehicles not observing and therefore not yielding to their intended prevention of crossing.
- Reduced width not an obviously large enough median width to deter those who recognize the obstruction but not finding it intimidating enough to prevent their crossing it anyways.

In addition, the project team prefers the full 20 ft median to provide enough green space to provide some landscaping to soften the affect of the ultimate facility of 6-lanes of traffic. There has been some public



objection to the 6 lanes vs 4 lanes section and the large expanse of pavement proposed. The 20 ft median will provide a larger green space in the middle to break up the expanse of asphalt and hopefully be more aesthetically pleasing and sensitive to the communities in which this project resides.

The project team therefore disagrees with the recommendation and prefers to continue with the 20 ft raised median to better manage traffic flow, provide a safer more visible channelizing barrier, and to provide a more aesthetically pleasing final product.

4.0 – Construct rural shoulder with 10-foot-wide overall shoulder with 4-foot-wide partial depth pavement. No, we do not agree with implementing this recommendation. The project resides within an MS4 region and therefore is subject to post construction stormwater management as well as the requirements of the Drainage Design Policy Manual with a post-developed flow increase. Post construction stormwater management requirements include stormwater runoff quality/reduction, stream channel protection, and overbank flood protection. In order to satisfy these requirements we intend to capture all of the runoff of the pavement through use of curb and gutter (an urban shoulder) into a separate closed drainage system which will pipe the roadway runoff to a permanent post construction stormwater detention basin. This permanent BMP will provide water treatment and detention before releasing downstream to a water of the US. Additionally the point outfalls will be limited therefore reducing the number of required BMPs. Utilizing a rural shoulder may allow sheet flow for treatment of water quality but this technique would not provide the necessary detention requirements to satisfy the post construction flow increases.

4.1 - Construct 12-foot-wide urban shoulder in lieu of 16-foot-wide urban shoulder. The project team agrees to partially apply this recommendation. In areas where a 16 ft shoulder can fit without significant impact to adjacent resources, we recommend keeping the 16 ft shoulder. This provides additional buffer between pedestrians on the sidewalk and the through traffic. This also provides more area for utility relocations to fit combined with other roadside elements. In areas where a reduction to a 12 ft shoulder width would avoid or minimize adverse impacts to adjacent resources, this reduced width shoulder would be employed.

7.0 – Eliminate ponds at five displacements The project team is evaluating the requirements of MS4 and the management of runoff to conform with the MS4 Permit as well as the drainage manual. The team is evaluating the design of BMP's to address both with every intent to minimize impacts and displacements. The project team feels this recommendation is shortsighted in that it only addresses consideration of MS4. The Drainage Design for Highways manual section 10.2.1.1 requires that the added runoff from a project that adds impervious surfaces does not adversely affect downstream for the 25 year storm. This additional requirement of the design team essentially encompasses or trumps the MS4 BMP infeasibility requirements. MS4 allows a method of evaluation and consideration whereby cost and/or impacts can render a need to meet MS4 requirements infeasible thereby eliminating this BMP. However, we are still obligated by the drainage manual to address the detention of additional runoff and therefore are still obligated to provide measures to satisfy this detention. For this project, the detention is being addressed with detention ponds and therefore they cannot be eliminated even to avoid a displacement although a avoiding displacements is the first choice in locating a pond.

10.0 – Perform detailed MS4 calculations to allow for elimination of ponds; acquire non-pond parcels first This project has an extremely accelerated schedule with R/W Authorization scheduled for FY 17 for this 18.8 mile long project. The magnitude of the effort required to perform detailed MS4 calculations to allow for elimination of ponds is not feasible to meet this accelerated schedule. The project team philosophy and approach to simplify and streamline the design process to establish conservative construction limits and subsequent Required R/W and Easements is as follows:

- Capture all runoff on SR 20 utilizing curb and gutter and a separate drainage system to pipe runoff from the roadway to detention ponds.
- Dry Detention Ponds are one of the possible MS4 BMP's for treating the water quality of the runoff as well as for detaining the water quantity of runoff. This dry pond BMP can treat 65% of



the TSS in the runoff. The MS4 permit requires that 80% of the TSS be removed from the runoff of newly added pavement. The weighted average of 65% of treatment by the pond of all the pavement runoff will for the majority of the drainage areas be equivalent to or exceed the 80% requirement of treatment of the additional pavement. The dry detention pond will in the majority of the drainage areas satisfy the water quality requirement of the MS4 permit and therefore eliminate lengthy and detailed analysis of multiple BMP alternatives or BMP trains. This in turn will save design time and get us to R/W Authorization sooner.

- The dry detention ponds will be initially sized to detain the volume of water from the 25 year storm event. When combined with the ability to achieve the TSS removal objective, these ponds will now satisfy both water quality and water quantity objectives as well as prevent downstream flooding to satisfy the downstream hydrologic assessment required by the drainage manual.
- This initial pond sizing will establish the conservative Required R/W and Easements to construct the pond including access roads for future maintenance. Now the team can prepare the Right of Way Plans to acquire the conservative project limits.
- Once the ponds have been sized, the drainage engineers will further detail evaluate whether the pond sizes can be reduced by:
  - Analyzing to see if allowing the new runoff to bypass the need for detention to pass the downstream hydrologic assessment without creating a downstream flooding situation. If so, the pond can then be secondarily analyzed and considered for whether or not it can be determined infeasible by means in accordance with the MS4 design process or if it can be replaced by other BMPs that would have smaller footprints and therefore require less R/W.
  - Analyzing the pre-development runoff flows and designing this flow rate into the detention pond outflow control structure along with the flow and outfall design necessary to successfully treat the removal of TSS (water quality) and then reducing this continuous flow across the design year storm duration from the overall pond size. This essentially optimizes the pond size down from its original conservative pond size developed in the earlier steps.

The design team agrees that the R/W acquisition team should begin acquiring parcels without the detention ponds or BMP's first allowing more time for the design team to conclude if detention ponds can be eliminated or reduced in size and then revise the Right of Way Plans with the savings prior to approaching the property owner for acquisition.

12.0 – Use consistent Right-of-Way width with permanent easements beyond. No, we do not agree with implementing this recommendation until further discussion and research can be completed. The project team plans to discuss with the District R/W Agents whether or not it makes sense to purchase all needed property as R/W or whether minimizing R/W and utilizing easements for construction of slopes would be appropriate for this corridor. Often times in rural areas, property owners have no desire to hang onto lands with an easement for a roadway slope when they can't find it to be useable for anything. In these cases they would prefer all needs to be required R/W and tie in slopes to be steeper. Typically in urban environments, placing the Required R/W at the should break minimizes impacts to adjacent parcels as these property owners would rather keep the slopes tying to existing as flatter slopes and useful as yards or other useful aspects of their property even though they may be permanent or even temporary easements. The project team will reach out to the District 1 and Distric 6 R/W agents for guidance and adjust accordingly.

17.0 – Use Design/Build as project delivery method to meet expedited schedule. No, we do not agree with implementing this recommendation. We have not seen conclusive evidence that the Design/Build project delivery method provides costs savings over traditional design bid build. We recognize that time savings could be realized through this method but not necessarily, cost savings.

**Schedule** – Albert said to add into our schedule time to present the design and R/W needs following the PFPR and before presenting to the public at the next round of PIOH's.

**PIOH Displays** – Albert agreed that proposed signals should be shown on PIOH displays even if they have not been approved as TE Studies through the District Traffic Engineers. All recognized that failing to show likely signals is confusing to the public and just creates further angst. Albert said to make sure to add a label such as "Pending approval of a TE Study" or similar.

**45 mph vs 55 mph:** Scott recommended that the project be designed to 45 mph to prevent the additional need for a paved 10 ft outside shoulder and 2' paved inside shoulder for speeds over 45 mph. This will save on footprint, an obvious concern by the public who is pushing back on the need for 6 lanes. It will also save on runoff for detention pond sizing and cleaning of pollutants. It will also save on overall construction and R/W costs by minimizing the footprint size. Albert said the best way to handle this would be to write an escalation memo for the Chief Engineer to request design to 55 mph with a variance from the need for the additional shoulder offset widths to the raised medians. Scott explained that currently all of SR 20 in Cherokee County and the first part into Forsyth County is currently posted as 45 mph. It then increases to 55 mph and drops back down to 50 mph just east of Sr 371/Post Rd to the end at N Corners Pkwy. Albert said to include a graphic depicting this in the escalation memo as it will help with the issue.

**Attachment 9**

**Meeting Minutes (Other)**



**SR 20 Improvements from Canton to Cumming**

Project No's: STP00-0002-00(862),  
STP00-0003-00(681), STP00-0003-00(682)  
PI No's: 0002862, 0003681, 0003682

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AECOM Proj.: 60267130

## Meeting Minutes

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**Subject:** Initial Concept Team Meetings for the SR 20 Corridor Improvements

**Date:** 2:00 pm March 5, 2013 at the GDOT District 1 Office (Gainesville)  
10:00 am March 6, 2013 at the GDOT District 6 Office (Cartersville)

**Attendees:** see attached sign-in sheets

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The meeting began with a round of introductions for everyone in attendance.

Karyn Matthews, GDOT PM, welcomed everyone and asked that they all participate and provide input as the project is presented throughout the meeting. She then introduced Scott Gero as the consultant Project Manager.

Scott Gero, Karyn Matthews, Claudia Bilotto (NEPA Lead), Don Gaines (traffic engineer), Leah Vaughan (Public Involvement lead), and Matt Scofield (Public Relations lead) went through a power point presentation that presented the project. See attached. The power point presentation touched on the following topics:

- Project location – SR 20 from I-575 to SR 400 in Cherokee and Forsyth counties
- History of projects – formerly 3 independent EA's => reissued as one EIS
- Schedule – 6 years to get to a ROD plus 2-3 more to get to letting.
- Project framework and approach
- Approach to public involvement
- Outreach to date –
  - Listening Tour – mtgs with city/county engineers and leaders, chambers of commerce, newspapers (Cherokee Tribune, Forsyth County News) – went over key takeaways from these meetings
  - Water Tank Rd Neighborhood Watch meeting – met with homeowners at their request to describe the project and process
- Metro Quest – the beta version of this new to GDOT software and website was presented for SR 20. It provides another tool for reaching out and gathering input. The screens include:
  - Welcome screen – general location and description of project and process
  - Priorities screen – allows user to prioritize their top 5 issues for the corridor
  - Show Us screen – interactive map that allows user to drop icons on map and enhance the icon's with information (ex. Work Icon – drop on location and enhance with travel mode and frequency)
  - Survey screen – further asks questions to understand the perspective of the user
  - Stay Involved screen – opportunity to provide additional demographic information including contact info, as well as provides links to project website, GDOT, & FHWA.
- Key messages for all when interacting with any agencies or public
- Project Justification Statement
- Draft Need and Purpose (final to be developed during the "Scoping" phase)
- Functional Classification – mix of Urban and Rural Principal Arterials
- Maps showing LOS 2010 and 2040



- **Action** – verify that the 2040 LOS no-build projections takes into account the passing lane project currently under construction in Cherokee County.
- High crash areas map – crash data collected from CARE for 2007-2009 (the most recent available data), considers 5 or more crashes per year to be a high crash location
- Planned and programmed projects on a map showing:
  - Programmed
  - Long Range
  - Aspirations
- Explanation of the “Scoping” process as required by an EIS
- Initial thoughts on design considerations
- Request of known maintenance issues – none provided
- Utilities – SUE will be used on the project. There was a call for any special utility issues.
  - GA Transmission (March 6 mtg) mentioned they have a proposed crossing. Locations were provided through Karyn Matthews by email on 2/7/13. There is an existing GTC line in Canton that is perpendicular to the corridor and there are some nearby facilities in Cumming though it is believed that they have been moved as a result of other projects. It should be fairly clean – they will double check.
  - AT&T mentioned that they have some facilities and would provide to our SUE (TBE Group). This includes 12 pair duct banks along SR 20, closer to Cumming, and includes locations under existing pavement.

Scott stressed that this project is currently seeking ways to “improve” SR 20. The scoping process will bare out whether or not the project becomes a widening project. At this point in the EIS and project development, the key message is that we are seeking ways to improve SR 20 so that we can safely and efficiently move people and goods through the corridor.

Notes from the District 1 meeting:

Teri Pope asked if the SR 20 project currently under construction were included on the project website. The team responded that all of the SR 20 projects have been consolidated onto one main page at [www.dot.ga.gov/sr20improvements](http://www.dot.ga.gov/sr20improvements). From that page, you can follow a link to specific project information.

The City of Cumming commented “The sooner the better”.

A representative mentioned that GTC was purchasing Right of Way now along the entire corridor for a new line between Canton and Cumming.

Neil Cantner asked if there are any specific areas where issues were worse than another. The team responded that each end of the corridor – the Canton and Cumming areas within the city limits and tie ins to GA 400 and I-575 – were anticipated to be the most complex.

Someone asked the duration of the project (8-10 years) and how many projects were included (three). Another attendee asked if staging would be discussed in this phase of the project. Scott responded that it would occur later as the alternatives are developed and most likely at the Concept Team Meeting.

Notes from the District 6 meeting:

Mike Haithcock (Dist 6 Asst Dist Engineer) commented that they have received some funding for some quick turnaround projects that were less than \$200K. The district has identified 7 or 8 projects to date that were located within right of way limits and did not involve utility relocations. Examples of these projects include right or left turn lanes or signals. He asked that as the project team evaluates the corridor, that if they see any potential small projects that would provide immediate benefit and fit the criteria, that they bring these to the attention of District 6. District 6 would then further evaluate to see if the projects fit into their funding and improvement plan. This should take place over the next 6 months.

Keith Posey (?) asked how the website will be publicized? The team responded that the GDOT project website address would be included on all project materials including flyers, webcards, press releases, and signage and would also be promoted through social media outlets including Facebook and Twitter. The MetroQuest website will be directly linked to the GDOT project website.

Mike Haithcock commented that distrust in Government is a general problem in the districts. He has found that making an effort to send GDOT staff to standing meetings in response to requests goes a long way. He offered the district's assistance in doing this throughout the course of the project.

Mike Haithcock commented that if there are solutions or projects that will potentially look at access control, the district could go in and buy access rights in advance.

Other notes:

Need to add proposed park at Water Tank Road to the Concept Layout.

Cynthia Burney provided information regarding Safety Projects along SR 20 and SR 140 – limits for the project are the western and eastern Cherokee County boundaries. These improvements include surface treatments, guardrail, and additional signage in some locations – all low cost improvements. The project is anticipated to let in December.



**Meeting Minutes**

AECOM Proj.: 60507210 (File 60267130)

**Subject:** Design issues**Date:** September 21, 2016, 10:00 am**Location:** GDOT OPC Conf Rm (25<sup>th</sup> floor)

<b>Attendees:</b>	Hiral Patel	GDOT Director of Engineering
	Brent Story	GDOT Design Policy & Support
	Dan Pass	GDOT Design Policy & Support
	Albert Shelby	GDOT Program Delivery Engineer
	Nicole Law	GDOT PM
	Scott Gero	AECOM PM
	Lindsey Dunnahoo	AECOM Engineer
	Jeff Wood	AECOM Traffic Engineer
	Laura Dawood	AECOM Environment Lead

- **Proposed Laneage & Cost –**

- Traffic data analyzed and projected out to Design Year 2045 to determine laneage needs (See attachment)
- AECOM recommended 6 through lanes from Scott Rd to SR 369 and then from SR 371 to N. Corners Pkwy (project end on the west side of Cumming). A 4-lane section is recommended in the middle from SR 369 to SR 371.
- The 6-lane (w 4-lane for PI 0002862) has a concept cost of \$315 MM. The 4-lane has a concept cost of \$270 MM. GDOT acknowledged that the relatively minor difference in cost was worth pursuing the 6-lane option since it meets the design year demand.
- The concept should include carrying 6-lanes the entire way so that it won't need to be revisited for future expansion later. This will be the preferred approach for now and what we should take to the public for comment. If there are concerns raised through the public involvement effort, then those areas would be reconsidered at that time.  
(Following the meeting it was determined that this approach will be presented to the Chief Engineer through an escalation memo to confirm.)

- **Access Control –**

- GDOT directed AECOM to design for Permitted Access and allow the District to determine which driveway access will be approved in the future. It would be too difficult for this corridor with the many existing driveways and access points to try to switch it to Partial Control of Access at this time. AECOM should try to combine driveways and pull back driveways from the functional area of intersections where feasible.
- The topography drops off to both the north and south sides in many locations which limits the adjacent network of roads. Many of the side roads, especially to the north of SR 20 tie directly to SR 20 and do not have a connecting parallel route. Therefore, many of the side roads need to have continued access to SR 20 as there are no other alternatives.

- **Other Design Issues:**

- The design speed should match the existing posted speeds. Alternative speed designs can be considered later in the design process if needed.
- Signals are determined based on warrant analysis
- AECOM proposed utilizing RCUT intersection control in lieu of additional signals to manage access. The context of the adjacent access will determine if the RCUT will be designed to accommodate passenger cars (can occur within the 6-lane and median footprint) or if it needs to accommodate tractor trailers (additional eyebrow paving needed for turning movement of large vehicles). RCUT median breaks to accommodate tractor trailers would be spaced at a logical distance and signed so that truck drivers would know not to try to U-turn in a passenger car only median break. GDOT agreed that this is a good approach to this corridor. It will improve safety and reduce friction points for the through movement providing better throughput and reduced travel times.
- The roundabouts would need to be peer reviewed. GDOT has considered 2 lane roundabouts, but 3 lanes is out of the norm to date. Consider 3 approach lanes tapering to 2 lane roundabout.
- Since this is state funded, consider assessing non-AASHTO standard situations and evaluating needs to improve sub-standard existing conditions on a case by case basis, and use data (e.g., crash) to support decision-making. For example, improving sags has not been a requirement even for FHWA projects.
- Other techniques for access control should be applied where feasible:
  - Consolidation of side roads and driveways
  - Elimination of dual driveways for parcels that can function with one, recommend design in this way and if there are concerns during RW acquisition, then design can be revisited on a case by case basis.
  - Acquisition of access rights from adjacent properties where feasible
- Median widths:
  - 20' raised (45 mph)
  - 24' raised (>45 mph) – provides a 2' buffer from the Type 7 curb of the raised median
  - 32' depressed (55 mph 4-lane) – decided not to use but rather to move forward with a 6-lane and 20' or 24' raised median
  - 44' depressed – if a 6-lane will not work in portions of PI 0002862 due to excessive impacts, then it may be best to provide a 4-lane with a 44' depressed median for portions that can accommodate this width and which can be expanded to a 6-lane in the future. The constrained areas would be a 4-lane with a reduced raised median and in the future if 6-lanes are needed, these areas of high impacts would have to be re-evaluated and addressed at that time. This scenario would only be considered if it is found that a 6-lane section would have unacceptable impacts if constructed now.
  - Median widths can be reduced in certain areas if we get pushback from the public.
- Shoulders:



- Brent instructed AECOM that it is acceptable to utilize a rural shoulder on one side of the road opposite of an urban shoulder if it fits into the context of the area. AECOM agreed and recommended a rural shoulder where possible to assist with water quality and MS4 design. It is more difficult to meet MS4 with a curb and gutter/closed drainage design than with rural shoulders. Brent Story agreed.
- Shoulder widths can be reduced in certain areas if we get pushback from the public.
- Public Involvement:
  - Based on the decisions today, AECOM will revise the layout and can then schedule meetings with local elected officials.
  - The project team should inform the District Engineers (Comer (Dist 6), Cook (Dist 1)) of any meetings and extend the offer for their attendance if available.
  - Elected state representatives can be informed through a letter and referral to displays on the website. This should be done in advance of the PIOH meeting dates.
  - Once the design is revised, a set of PIOHs (2 nights, 1 on east end and 1 on west end) can be scheduled and conducted. Anticipate not needing as much educational materials as at previous PIOHs. The displays should include:
    - Renderings/simulations (e.g., where the new road paints over the existing roadway and takes the viewer on a drive of the corridor)
    - Roll plots
    - Educational materials for RCUTs (Tyler Peak at D3 may have some good resources.)
  - The project team should anticipate that public input may affect the concept layout.
- Environment
  - Prior to going to PFPR, there needs to be a comfort level that resources have been identified and effects determinations are not going to change (e.g. from adverse to significantly adverse under GEPA).
  - Do not necessarily need an approved GEPA document

Displays/Handouts:

- Roll plots of 6-lane w signal and RCUT locations as well as edge of pavement for 4-lane scenario
- Handout: Corridor Map w/ PI Delineations & Laneage Requirements, Laneage Needs Spreadsheet, Typical Sections

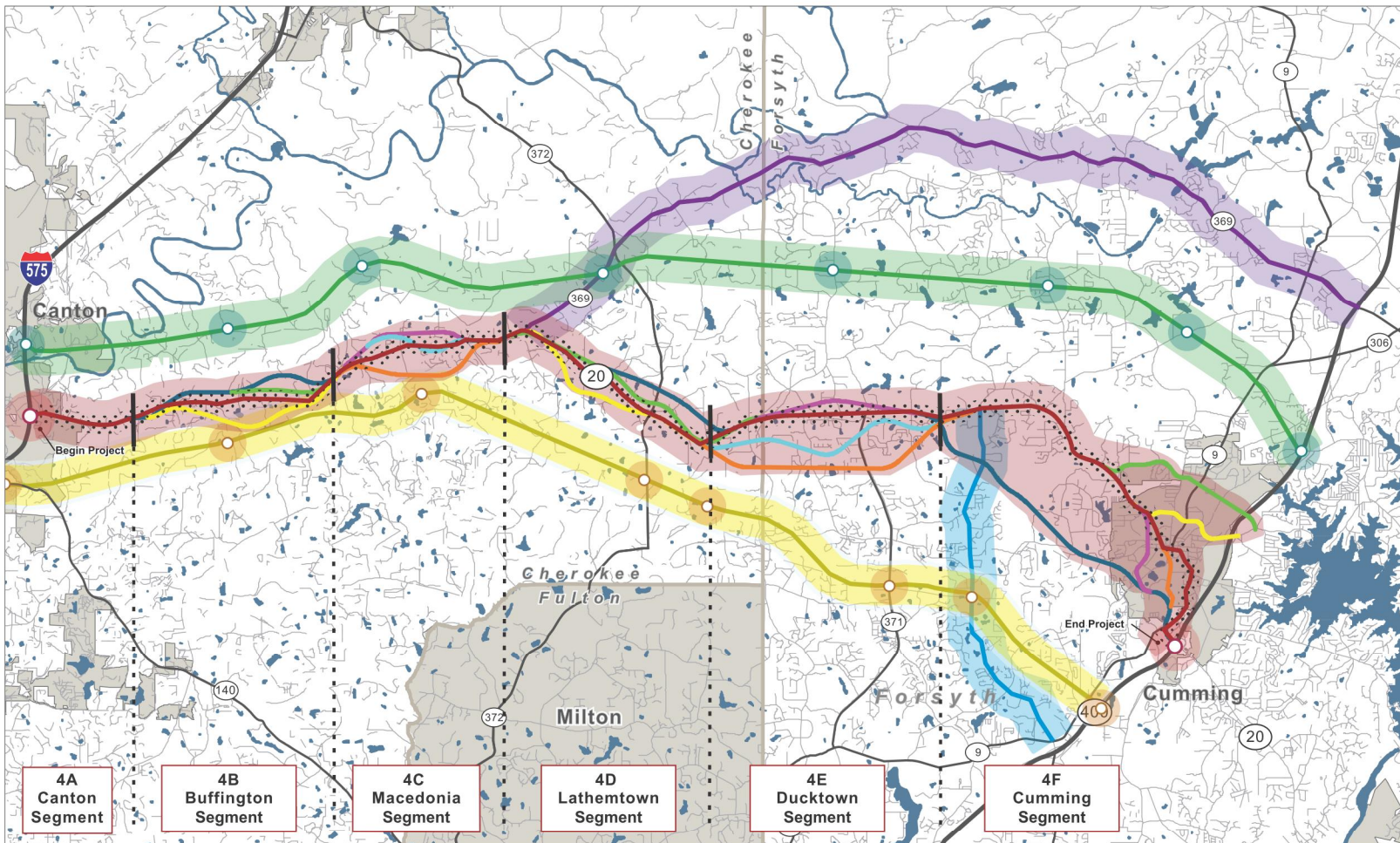
<i>Brief Project Description</i>		SR 20 Improvements from Scott Road to N. Corners Parkway (PIs 0014131, 0014132, 0014133, 0002862, 0003682)					
<i>Date of Open House</i>		12-6-2016		<i>End of Comment Period</i>		12-30-2016	
<i>Number in Attendance</i>		312					
<i>Officials in Attendance (list name and title)</i>		Scott Morgan, Representing City of Cumming Paul Oh, Representing Congressman Rob Woodall Media Present: Forsyth Herald, Forsyth County News					
<i>Comment Breakdown (for comments provided at the Open House) 6 total written comments received.</i>							
<i>For</i>	17	<i>Conditional</i>	10	<i>Uncommitted</i>	2 (plus 2 who didn't answer)	<i>Against</i>	2
<i>Major concerns:</i>		<p>In general, the project received support. The following were concerns voiced at the meeting:</p> <ul style="list-style-type: none"> <li>-Access and/or impacts to personal property and businesses,</li> <li>-Increased truck traffic,</li> <li>-Questions about benefits of restricted U-turn design,</li> <li>-Questions about why the project ends west of Cumming and does not continue to SR 400,</li> <li>-Design suggestions for specific locations,</li> <li>- Questions about what makes a property historic</li> </ul>					
<i>Prepared by (include firm's name if applicable):</i>		Leah Vaughan, Sycamore Consulting, Inc.					

<i>Brief Project Description</i>		SR 20 Improvements from Scott Road to N. Corners Parkway (PIs 0014131, 0014132, 0014133, 0002862, 0003682)					
<i>Date of Open House</i>		12-15-2016		<i>End of Comment Period</i>		12-30-2016	
513		312					
<i>Officials in Attendance (list name and title)</i>		State Representative Wes Cantrell Beatrice Torralba, Representing Senator David Perdue Geoff Morton, Cherokee County Media Present: Cherokee Tribune					
<i>Comment Breakdown (for comments provided at the Open House) 6 total written comments received.</i>							
<i>For</i>	8	<i>Conditional</i>	15	<i>Uncommitted</i>	3 (plus 1 who didn't answer)	<i>Against</i>	1
<i>Major concerns:</i>		<p>Anecdotally, the project received wide-spread support. The reporter from the Cherokee paper noted that he had never seen as many happy people at a PIOH. Of the written comments received, there were several major categories of concern:</p> <ul style="list-style-type: none"> <li>• Access and/or impacts to personal property and businesses, specifically related to the addition of a median/RCUTS;</li> <li>• Questions about benefits of the median and restricted U-turn design;</li> <li>• Concern about fair and speedy relocation/compensation for displacement properties and/or a desired for complete taking rather than having the road too close to homes;</li> <li>• Impact of increased noise and pollution;</li> <li>• Requests for traffic signals;</li> <li>• Tractor trailer traffic access to industrial park, other businesses where U-turns will be difficult.</li> <li>• Design suggestions for specific locations; and</li> <li>• Appreciation for the design and process.</li> </ul>					
<i>Prepared by (include firm's name if applicable):</i>		Leah Vaughan, Sycamore Consulting, Inc.					

## **Attachment 10**

### **Screen 2 Conceptual Alternatives**





### Legend

- Conceptual Alternative 2  
Widen Existing  
(formerly Alt-B)
- Conceptual Alternative 3A  
New Location North  
(formerly Alt-A)
- Conceptual Alternative 3B  
New Location South  
(formerly Alt-C)
- Conceptual Alternatives 4  
Combination Alt 2 and  
Localized Bypasses  
(formerly Alt-B)
- Conceptual Alternative 5A  
Combination of Alt 4 to  
SR 369; cosign SR 369  
as SR 20
- Conceptual Alternative 5B  
Combination of Alt 4 to  
Bethelview Road; cosign  
Bethelview Road as SR 20

Note: Conceptual Alternative 0 - No Build and Conceptual Alternative 1 - Transportation Systems Management are not shown on the map.



## 21.0 DISPLACEMENTS

Displacements presented in the table below distinguish between total displacements of a conceptual alternative and displacements per mile. Each table is formatted so that the Total column indicates both the total displacements and the rate of displacements per mile. It should be noted that displacements are not evenly distributed throughout the corridor. For example, in densely populated areas, clusters of displacements may occur. Therefore, the rate per mile does not differentiate between densely or sparsely populated areas. The number outside the parentheses represents the total displacements, while inside the rate of displacements. For example, Conceptual Alternative 3A shows 287 (12.7), so that this conceptual alternative has 287 total displacements at a rate of 12.7 displacements per mile. The figures below provide a summary of both combined displacements and rate of displacements per mile. The estimated number of displacements will serve as a proxy until a detailed assessment for each alternative is conducted in accordance with GDOT's Environmental Procedures Manual in the DEIS phase of project development.

In order to aggregate the number of potential displacements, aerial imagery was used to identify impacted structures for each alternative. The corridor was flown in 2012 to obtain geo-referenced, aerial imagery; however, several of the alternatives fall outside the extents of these aerials. Therefore, these aerials were supplemented with 2010 aerials that are publicly available from the United States Department of Agriculture and Google Maps aerials/street view (where available). Based on comparing active construction sites along the corridor, the 2012 aerial imagery and the current Google Maps aerial imagery were collected at similar times.

Cherokee and Forsyth counties provided their latest parcel maps within the study area. This data, along with the impacted structures and Google Maps aerials/street view, was used to identify displacements. Displacements are different than impacted structures because one building does not necessarily constitute one displacement. For example, if one parcel has a house with a separate garage, it would be counted as two structures but only one displacement. Similarly, a strip mall could have one building but hold multiple businesses and was therefore counted as multiple displacements.

Land use maps were provided by Cherokee and Forsyth counties and were used, along with aerials and Google Maps aerials/street view, to identify type of displacement. In the case of a discrepancy between sources, professional judgment was used to assign displacement type. The types of displacement identified are residential, commercial, industrial, and institutional.

Residential displacements include residences, such as houses and apartment complexes. Each house was considered one displacement. Displacements for apartment complexes were estimates based on building height. If a townhome building was impacted, only the townhomes the alternative touched were considered displacements; it was assumed that the building could be renovated to preserve the remaining townhomes.

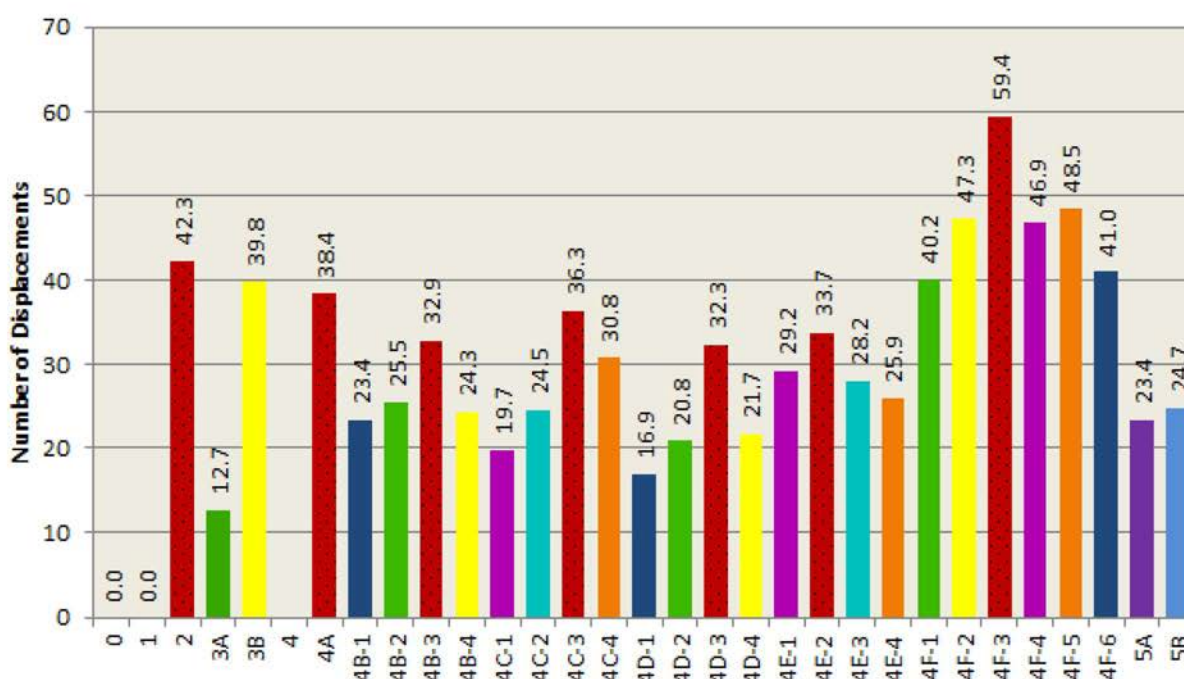
Commercial displacements include businesses and agricultural facilities, such as barns and chicken coops. The number of businesses in a building was estimated using Google Maps street view. Similar to townhome buildings, if a strip mall building was impacted, only the businesses the alternative impacted were considered displacements.

Industrial displacements include manufacturing facilities, poultry plants, and treatment plants.

Institutional displacements include public facilities such as schools, churches, government facilities, and utility sites. Common facilities in neighborhoods (i.e. tennis courts, pools, etc.) were also considered institutional displacements.

The following figure and table provides the dataset of potential displacements, which were calculated using aerial photography.

**Figure 21.1 Potential Quantitative and Qualitative Displacements per Mile - All Conceptual Alternatives**



Source: Cherokee and Forsyth counties parcel maps, aeriels, and Google Maps imagery

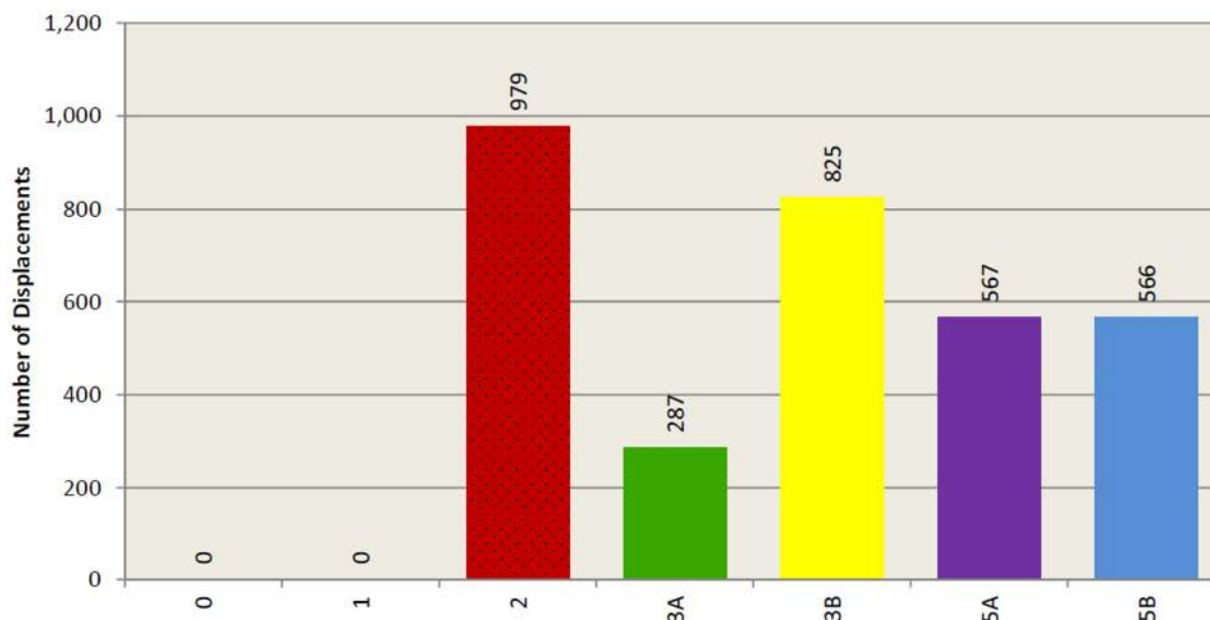
\*Note: Preliminary impacts for tables and figures are based on a high level of GIS analysis. As detailed analyses are conducted, and alternatives are refined, impacts to various resources may change.

\*\*Note: The lengths for Alternative 4 will be determined after various links are analyzed in subsequent analyses. The shortest distance for Alternative 4 would be 23.20 miles and the longest distance would be 25.43 miles.

\*\*\*Note: Displacements may occur in clusters within densely populated areas.



Figure 21.2 Potential Total Displacements - Corridor Alternatives

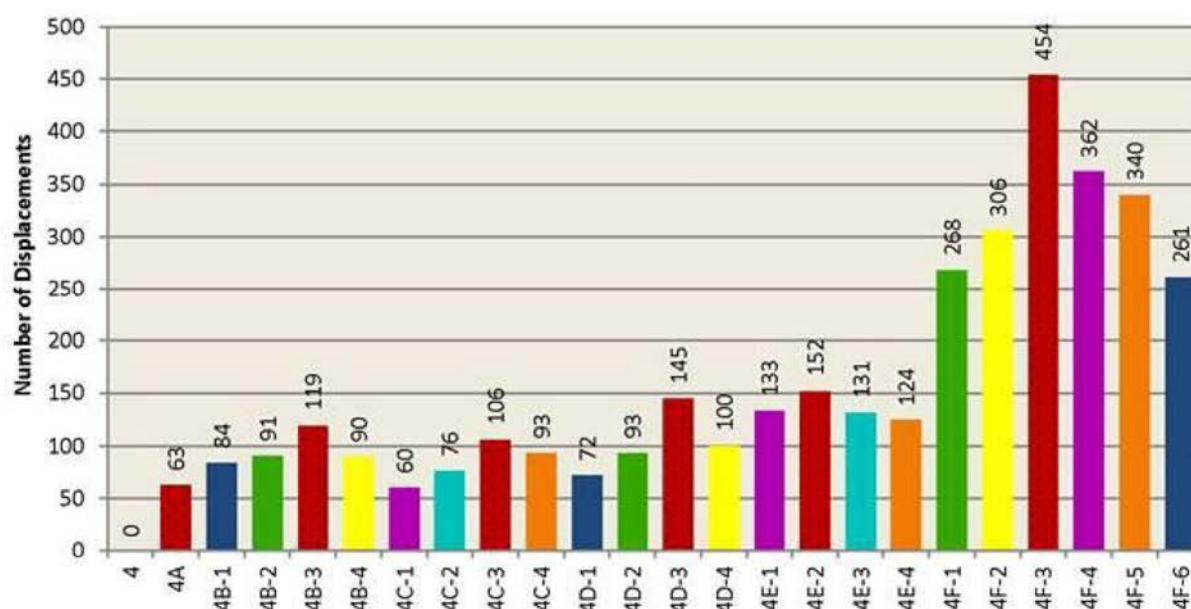


Source: Cherokee and Forsyth counties parcel maps, aerials, and Google Maps imagery

\*Note: Preliminary impacts for tables and figures are based on a high level of GIS analysis. As detailed analyses are conducted, and alternatives are refined, impacts to various resources may change.

\*\*Note: The lengths for Alternative 4 will be determined after various links are analyzed in subsequent analyses. The shortest distance for Alternative 4 would be 23.20 miles and the longest distance would be 25.43 miles.

Figure 21.3 Potential Total Quantitative and Qualitative Displacements - Conceptual Alternatives - Links



Source: Cherokee and Forsyth counties parcel maps, aeriels, and Google Maps imagery

\*Note: Preliminary impacts for tables and figures are based on a high level of GIS analysis. As detailed analyses are conducted, and alternatives are refined, impacts to various resources may change.

\*\*Note: The lengths for Alternative 4 will be determined after various links are analyzed in subsequent analyses. The shortest distance for Alternative 4 would be 23.20 miles and the longest distance would be 25.43 miles.

**Table 21.1 Potential Displacements**

Conceptual Alternative	Length of Corridor (miles)	Total Displacements (per mile)	Residential	Commercial	Industrial	Institutional	Qualitative
0	0	0 (0)	0	0	0	0	●
1	0	0 (0)	0	0	0	0	●
2	23.16	979 (42.3)	415	523	6	35	●
3A	22.61	287 (12.7)	251	32	0	4	●
3B	20.73	825 (39.8)	770	50	0	5	●
4A	1.64	63 (38.4)	32	31	0	0	●
4B-1	3.59	84 (23.4)	64	19	0	1	●
4B-2	3.57	91 (25.5)	71	19	0	1	●
4B-3	3.62	119 (32.9)	81	33	0	5	●
4B-4	3.70	90 (24.3)	78	9	0	3	●
4C-1	3.05	60 (19.7)	52	7	0	1	●
4C-2	3.1	76 (24.5)	65	10	0	1	●
4C-3	2.92	106 (36.3)	71	32	0	3	●
4C-4	3.03	93 (30.8)	85	6	0	2	●
4D-1	4.25	72 (16.9)	56	15	0	1	●
4D-2	4.47	93 (20.8)	69	22	1	1	●
4D-3	4.49	145 (32.3)	76	62	2	5	●
4D-4	4.61	100 (21.7)	63	36	0	1	●
4E-1	4.56	133 (29.2)	86	40	2	5	●
4E-2	4.51	152 (33.7)	82	61	3	6	●



**Table 21.1 Potential Displacements**

Conceptual Alternative	Length of Corridor (miles)	Total Displacements (per mile)	Residential	Commercial	Industrial	Institutional	Qualitative
4E-3	4.65	131 (28.2)	117	12	1	1	●
4E-4	4.78	124 (25.9)	114	8	1	1	●
4F-1	6.67	268 (40.2)	177	73	2	16	●
4F-2	6.47	306 (47.3)	137	143	2	24	○
4F-3	7.64	454 (59.4)	106	328	3	17	○
4F-4	7.27	341 (46.9)	115	204	7	15	○
4F-5	7.48	363 (48.5)	184	145	11	23	○
4F-6	6.36	261 (41.0)	190	49	15	7	●
5A	24.28	567 (23.4)	375	170	3	19	●
5B	16.65	566 (34.0)	327	216	5	18	●

Source: Cherokee and Forsyth counties parcel maps, aeriels, and Google Maps imagery

\*Note: Preliminary impacts for tables and figures are based on a high level of GIS analysis. As detailed analyses are conducted, and alternatives are refined, impacts to various resources may change.

\*\*Note: The lengths for Alternative 4 will be determined after various links are analyzed in subsequent analyses. The shortest distance for Alternative 4 would be 23.20 miles and the longest distance would be 25.43 miles.

Widening the existing SR 20 would result in 979 total displacements. About 53% of these displacements are commercial displacements, as there are many businesses along the existing road. This alternative has the largest number of displacements of any alternative and was rated as Needs Improvement. The Northern New Location alternative (3A) displacements are mostly residential (~87%). As this alternative has one of the lowest rates of displacements (12.7 displacements per mile), it receives a rating of Exceeds. The Southern New Location alternative (3B) has over 2.5 times the displacements of the Northern New Location. These displacements are still primarily residential (~93%). About a third of the residential displacements come from impacting an apartment complex next I-575. These could potentially be avoided by shifting the alignment to the north or south. This alternative rates as Meets due to its moderate rate of displacements.

The Canton link from I-575 to Buffington has an almost equal amount of residential and commercial displacements. The majority of commercial displacements are a result of the impact to the Canton Marketplace. This alternative rates as Meets due to its moderate rate of displacements.

4B-1 has the least amount of total displacements at 84, while 4B-3 has the most at 119. All alignments result in primarily residential displacements, ranging from 87% for 4B-4 to 68% for

4B-3. There are no industrial displacements for any of the alternatives and relatively few institutional displacements. 4B-1 rates as Exceeds, while the other alternatives rate as Meets.

4C-1 has the least amount of total displacements at 60, while 4C-3 has the most at 106. All of these alternatives result in primarily residential displacements. Compared to the other areas along the corridor, the Macedonia alternatives have a relatively low number of displacements. 4C-1 receives a rating of Exceeds, while 4C-2, 4C-3, and 4C-4 receive a rating of Meets.

4D-1 has the least amount of total displacements at 72, while 4D-3 nearly doubles that amount with the most total displacements at 143. The majority of displacements for 4D-1, 4D-2, and 4D-4 are residential, while 4D-3 is comprised of a more even spread between residential and non-residential displacements. All the Lathemtown alternatives receive a rating of Exceeds, except for 4D-3, which receives a rating of Meets.

4D-4 has the least amount of total displacements at 124, while 4D-3 has the most at 150. Compared to the other areas along the corridor, all the alternatives for Ducktown have a relatively high number of total displacements. 4D-3 and 4D-4 have a very high percentage of residential displacements, while the displacements for 4D-1 and 4D-2 are more evenly distributed. All of these alternatives receive a rating of Meets.

Although 4F-6 has the least amount of total displacements at 261, it also has the most residential displacements at 190. 4F-3 has the most total displacements at 451, but the least amount of residential displacements at 106. As expected, widening along the existing corridor (4F-3) has the most amount of commercial displacements by far. Compared to the other areas, the displacements resulting from these alternatives are more evenly distributed between residential and non-residential, as they are going through the more developed areas of the City of Cumming. 4F-1 and 4F-6 receive a rating of Meets; the rest of the conceptual alternatives in Cumming receive a rating of Needs Improvement.

5A would result in 567 total displacements, with about 66% of those being residential displacements. 5B assumes that SR 20 will be widened from I-575 to Bethelview, then diverted onto the existing Bethelview Rd (programmed to be constructed in 2014/2015); this alternative would result in 566 total displacements. Both of these partial rerouting alternatives receive a rating of Meets.



## 2.4 Costs/Other

### 2.4.1 Costs/Other Summary

Costs evaluated in Screen 2 are based on anticipated right of way (ROW), construction (CST), and operations and maintenance costs. ROW costs primarily reflect the amount of additional land (i.e. acres) required for acquisition including improvements, where price variability occurs by land use type (e.g. commercial, residential, agricultural, and industrial). Cost of construction was developed by estimating the main drivers of roadway construction and applying average percentage factors to develop costs for the secondary drivers. The two main drivers for construction costs are pavement (e.g. travel lanes and shoulders) and structures (e.g. bridges) and are estimated by using unit costs for the proposed areas. Secondary drivers for pavement consist of drainage, erosion control, signs, pavement markings, traffic control, and earthwork. Average percentage factors were developed by analyzing historic GDOT project costs and are indexed to the cost of the pavement. Structures do not have any secondary drivers for their construction costs. The factors impacting both ROW and CST cost estimates were calculated via desktop analyses. Costs at this phase are preliminary and are subject to change as detailed analyses are performed.

In order to illustrate the relationship of project costs with potential benefits a conceptual alternative can produce, a B/C ratio was calculated for each conceptual alternative (i.e. return on the dollar). The B/C ratio works to compare the user benefits of the conceptual alternative to the construction cost. The B/C ratio was developed based on correlating the benefits of the project with the project Need and Purpose, specifically, in the alternative's ability to address mobility and congestion relief needs.

One conclusion of the Costs/Other Evaluation was the need to develop a specific Marginal Utility Analysis. A marginal utility analysis could be used to quantify the how well an alternative performs for its cost. This analysis is provided in Appendix B.

Table 2.35 illustrates the Costs/Other criteria and the units of analysis that were used for each conceptual alternative. The ratings used for Costs/Other include 'Exceeds', 'Meets', and 'Needs Improvement'. Following this table is a discussion of each Costs/Other criteria, a brief discussion of what the criterion is, how it was assessed, and how the qualitative ratings were applied (to be completed upon agency coordination).

Section 3 provides a comprehensive summary of all performance results. Appendix A provides data for environmental and community impacts results from Screen 2 for each conceptual alternative. Appendix B provides further details of the approach, assumptions, and context for evaluation as well as providing results for each criterion.

**Table 2.34 Costs/Other Criteria**

Performance Criteria*	Units
Total Costs  (including Right of Way, Construction, Operations and Maintenance)	\$ (Million)
Benefit/Cost Ratio	B/C
Constructability	Qualitative

\*Analysis of these criteria is provided in Sections 2.4.2 – 2.4.7. A summary of results is found in Appendix A, Screen 2 Comprehensive Matrix. Detailed analysis of these criteria is found in Appendix B.

## 2.4.2 Cost Summary

Project costs were based on the right of way (ROW) costs, construction (CST) costs, and operations and maintenance costs, but the alternatives' costs were grouped into one lump sum category to help illustrate the comprehensive amount of capital investment necessary to construct and maintain each alternative. Among the alternatives, there was an extensive amount of variability in ROW and CST costs to the extent that comparing them could prove challenging. Therefore, to account for this variability, alternatives' total project costs were reported as one lump sum of ROW and CST costs. The costs for operations and maintenance were considered negligible since this component accounted for such a low percentage of the total project costs. Costs at this phase are preliminary and are subject to change as detailed analyses are performed.

The costs for conceptual alternatives 3A and 3B were exorbitantly higher than the other corridor widening alternatives, so they were rated as 'Needs Improvement' due to these being over double the average costs of the other widening alternatives (e.g. \$615.6 million and \$630.2 million, respectively). The average costs for conceptual alternatives 2, 5A, and 5B was \$252 million; each of these conceptual alternatives had costs that fell within the range of the average, therefore received a 'Meets' rating. The average combined ROW and CST costs were \$68.1M per link for conceptual alternatives 4A, 4B-(1, 2, 3, 4), 4C-(1, 2, 3, 4), 4E-(1, 2, 3, 4), and 4F-(1, 2, 3, 4, 5, 6), therefore all conceptual alternatives with costs falling within the range of greater than \$40 million but less than \$80 million were considered within the average and received a 'Meets' rating. The conceptual alternatives that were \$40 million or less received an 'Exceeds' rating. The TSM conceptual alternative 1 had an estimated cost slightly over \$2 million due to the type of improvements being minor in nature, especially since it may not require or only require a minimum amount of ROW. Conceptual alternatives 4A and 4C-3 had project costs under or equal to \$40 million. The threshold applied for the 'Needs Improvement' rating was project costs exceeding \$80 million, which applied to conceptual alternative 4E-4 and conceptual alternatives 4F-1 thru 6.

Due to the project costs having natural breaks in terms of the cost differential among the alternatives, it assisted with the establishment of the thresholds for which the qualitative ratings were based. A conceptual alternative's cost was not the key determinant factor for evaluating its overall rating; however project costs did have an impact due to it helping



illustrate the degree of monetary investment necessary for implementing a specific alternative.

**Ratings Justification:** The qualitative ratings used to assess the impact of a conceptual alternative's costs were Exceeds, Meets, and Needs Improvement based on natural breaks. If an alternative's project costs were considerably lower than other alternatives' costs, then it received an 'Exceeds' rating. Alternatives with project costs that fell more in line with the average project costs received a 'Meets' rating. For the cases where an alternative's project costs were considerably higher than the average project costs or were so high that it was challenging to draw practical comparisons, those alternatives received a 'Needs Improvement'.

**Table 2.35 Total Costs Qualitative Ratings**

Rating	Legend	Alternative(s)
<b>Exceeds</b>	●	<div>0 - No Build</div> <div>1 - Transportation Systems Management</div> <div>4A-1 - Canton Red (Existing)</div> <div>4C-3 - Macedonia Red (Existing)</div> <div>4C-4 - Macedonia Orange (South)</div>
<b>Meets</b>	◐	<div>2 - Widen Existing</div> <div>4B-1 - Buffington Blue (North)</div> <div>4B-2 - Buffington Green (North)</div> <div>4B-3 - Buffington Red (Existing)</div> <div>4B-4 - Buffington Yellow (South)</div> <div>4C-1 - Macedonia Pink (North)</div> <div>4C-2 - Macedonia Teal (North)</div> <div>4D-1 - Lathemtown Blue (North)</div> <div>4D-2 - Lathemtown Green (North)</div> <div>4D-3 - Lathemtown Red (Existing)</div> <div>4D-4 - Lathemtown Yellow (South)</div> <div>4E-1 - Ducktown Pink (North)</div> <div>4E-2 - Ducktown Red (Existing)</div> <div>4E-3 - Ducktown Teal (South)</div> <div>5A - Alt 4 and SR 369</div> <div>5B - Alt 4 and Bethelview</div>
<b>Needs Improvement</b>	○	<div>3A - North</div> <div>3B - South</div> <div>4E-4 - Ducktown Orange (South)</div> <div>4F-1 - Cumming Green (Sawnee Dr.)</div> <div>4F-2 - Cumming Yellow (Elm St.)</div> <div>4F-3 - Cumming Red (Existing)</div> <div>4F-4 - Cumming Pink (Tolbert St.)</div> <div>4F-5 - Cumming Orange (Veterans Memorial Blvd.)</div> <div>4F-6 - Cumming Blue (Chamblee Gap Rd.)</div>

Note: Costs at this phase are preliminary and are subject to change as detailed analyses are performed.



### 2.4.3 Right of Way

The tools used to determine the number and type of land use impacts were GIS and county land use and zoning maps for Forsyth and Cherokee Counties. The primary tool used for calculating ROW costs based on the pre-determined ROW impacts was GDOT's Office of Planning RUCEST (Right of Way and Utility Relocation Cost Estimate Tool). This tool is used to develop right of way planning level cost estimates for a diverse set of project types, ranging from auxiliary lanes, bridges, frontage roads, multi-use trails, turn lanes, sidewalks, roundabouts, and traditional widening projects. The pricing variables used within RUCEST are derived from actual historical data from previously let projects in coordination with GDOT's ROW Office and its Utility Office. Assumptions concerning ROW primarily involved the determination of ROW width (assumed to be 250 feet for conceptual alternatives 2, 4A, 4B[1, 2, 3, 4], 4C[1, 2, 3, 4], 4D[1, 2, 3, 4], 4E[1, 2, 3, 4], 4F[1, 2, 3, 4, 5, 6], 5A and 5B; and assumed to be 300 feet for conceptual alternatives 3A and 3B), inventorying land use types (i.e., commercial, residential, industrial, agricultural), and counting the number of improvements and displacements by land use type. Additionally, the particular county an alternative was located is a significant variable to capture. Appendix B provides additional details on the evaluation of this criterion. Costs at this phase are preliminary and are subject to change as detailed analyses are performed.

### 2.4.4 Construction

Construction costs estimates for this analysis also include bridges and interchanges. The assumptions for pavement widths are 65 feet for four lane facilities; 89 feet for six lane facilities, and 92 feet for conceptual alternatives 3A and 3B. The primary tool utilized for calculating construction costs is GDOT's CES (Cost Estimating System).

There was variability in costs for roadway segments on existing alignment compared to segments on new alignment; the same applies to the contingency percentage as well which is covered in a later section of this report. The differential between new alignment and existing alignment is attributed to the amount of earthwork necessary, whereas less earthwork is required for widening on existing alignment compared to a substantial amount more required for new alignments segments. Appendix B provides additional details on the evaluation of this criterion. Costs at this phase are preliminary and are subject to change as detailed analyses are performed.

### 2.4.5 Operations and Maintenance

Calculating the anticipated costs of maintaining a new or improved roadway facility for SR 20 is captured in operations and maintenance. These costs are typically based on maintaining quality pavement, bridges, and signage along the corridor; however, most of these cost items are difficult to project due to them being based on the severity of need as well as being tied to scheduled inspections. Therefore, operations and maintenance costs were based on resurfacing, since resurfacing needs are easily foreseeable and anticipated. It was assumed that a roadway facility will be resurfaced at least twice within its 20 year design life. The key driver in resurfacing costs is the amount of pavement needed (i.e. square yard and tonnage).

Costs are expressed in terms of annual projections by dividing the total construction costs by 20 to represent the design life of twenty years. The total construction costs are based on the total number of miles to repave/resurface. The constant variable used for each conceptual alternative was \$54 per ton for asphalt. Appendix B provides additional

details on the evaluation of this criterion. Costs at this phase are preliminary and are subject to change as detailed analyses are performed.

#### 2.4.6 Benefit/Cost Ratio

The Benefits-to-Cost (B/C) Ratio developed for this project measures the benefits, as related to the Need and Purpose objectives, and compares them to the total project costs. The Need and Purpose objectives for the SR 20 Corridor Improvements project are: improve Mobility, reduce Congestion, and improve Safety along the corridor. Mobility can be measured using monetized travel time savings and is the basis of the B/C ratio. Congestion reduction is discussed further in Appendix B, and safety could not be included at this time due to the complexity of the analysis being inconsistent with the level of design at this Screen 2 Alternatives Analysis phase.

The benefit calculated for the B/C ratio represents, in dollars, the time saved for a single user on a single trip if a conceptual alternative were constructed. The cost calculated for the B/C ratio represents the total project cost (right-of-way acquisition and construction) required for that user to make the same trip. Appendix B provides additional details on the evaluation of this criterion.




Since this metric does not calculate monetized benefits associated with V/C ratio and safety improvements, its results should not be used as a primary criterion for decision-making. The results of this analysis provide a level of sensitivity to other, stronger criteria and should be used to fine-tune rankings of conceptual alternatives. If this metric is combined with the results of the marginal utility analysis, it can provide better clarity on how a particular conceptual alternative performs associated with the Need and Purpose objectives for this project. Costs at this phase are preliminary and are subject to change as detailed analyses are performed.

**Ratings Justification:** The natural breaks in the quantitative data fall into the following ranges and were assigned the corresponding qualitative ratings:

- B/C ratio > 3.8 - Exceeds
- $2.0 > \text{B/C ratio} < 3.8$  - Meets
- B/C ratio < 2.0 - Needs Improvement



Table 2.36 Benefit/Cost Qualitative Ratings

Rating	Legend	Alternative(s)
<b>Exceeds</b>		<div>1 - Transportation Systems Management</div> <div>4B-1 - Buffington Blue (North)</div> <div>4B-2 - Buffington Green (North)</div> <div>4B-3 - Buffington Red (Existing)</div> <div>4B-4 - Buffington Yellow (South)</div> <div>4C-4 - Macedonia Orange (South)</div> <div>4D-4 - Lathetown Yellow (South)</div> <div>4F-1 - Cumming Green (Sawnee Dr.)</div> <div>4F-6 - Cumming Blue (Chamblee Gap Rd.)</div>
<b>Meets</b>		<div>2 - Widen Existing</div> <div>3A - North</div> <div>3B - South</div> <div>4C-1 - Macedonia Pink (North)</div> <div>4C-2 - Macedonia Teal (North)</div> <div>4D-1 - Lathetown Blue (North)</div> <div>4D-2 - Lathetown Green (North)</div> <div>4E-1 - Ducktown Pink (North)</div> <div>4E-3 - Ducktown Teal (South)</div> <div>4E-4 - Ducktown Orange (South)</div> <div>4F-2 - Cumming Yellow (Elm St.)</div> <div>4F-3 - Cumming Red (Existing)</div> <div>4F-4 - Cumming Pink (Tolbert St.)</div> <div>4F-5 - Cumming Orange (Veterans Memorial Blvd.)</div> <div>5A - Alt 4 and SR 369</div>
<b>Needs Improvement</b>		<div>0 - No Build</div> <div>4A-1 - Canton Red (Existing)</div> <div>4C-3 - Macedonia Red (Existing)</div> <div>4D-3 - Lathetown Red (Existing)</div> <div>4E-2 - Ducktown Red (Existing)</div> <div>5B - Alt 4 and Bethelview</div>

Note: Costs at this phase are preliminary and are subject to change as detailed analyses are performed.

## 2.4.7 Constructability

The constructability measure for the SR 20 Alternatives Analysis provides a qualitative measure for the risks associated with the construction cost or overall project schedule. Risk identifies areas of uncertainty in the project's construction cost or overall project schedule that are reasonably foreseeable at the early stage in project development. The method for determining constructability for the SR 20 Corridor Improvement Project's alternatives consists of three categories: structural, roadway, and community impacts to schedule risks. Costs at this phase are preliminary and are subject to change as detailed analyses are performed.




- 1) Structural risks identify risks associated with the construction of major structures (bridges or tunnels), construction of roadway on embankment, and with right-of-way acquisition. Structural risks for cost and project schedule are mostly dependent on the number of structures constructed and the complexity of the construction. For example, standard GDOT bridges do not require complex construction techniques or staging practices to construct, whereas long-span bridges require complex staging and maintenance of traffic practices to properly construct. Additionally, a vast number of bridges on an alternative may increase its risk for cost (availability of materials) and/or schedule (takes longer to construct numerous bridges).
- 2) Roadway risks for cost and project schedule are mostly dependent on the complexity of construction staging or building the alternative under traffic. For example, a new location facility does not require much construction staging while vehicles are present, as the construction occurs in areas where no vehicles travel. Alternatively, a standard roadway widening provides a moderate level of risk to schedule as the construction of new roadway components must be constructed piecemeal as opposed to all at once. Lastly, very complex roadway staging typically requires extensive temporary pavement and several detours to construct under traffic.
- 3) Community Impacts to Schedule risks for cost and project schedule are mostly dependent on the number of properties required to acquire prior to the construction of the project. For example, in urban areas where there are numerous acquisitions, the project schedule can be highly uncertain as numerous negotiations with property owners must occur. However, in rural and largely undeveloped areas, right-of-way acquisition occurs at a fast pace as there are fewer property owners. Risks associated with construction cost typically are associated with improvements that are negotiated into the project. An example is for the Georgia DOT to construct a retaining wall on a property to minimize the total amount of property acquired.

Appendix B provides additional details on the evaluation of this criterion.

**Ratings Justification:** All three risk categories are aggregated together (for comparison purposes) to form an overall constructability rating. This constructability rating represents the total uncertainty to the construction cost and project schedule for an alternative. These evaluations are based solely on professional judgment by a licensed engineer.

- Exceeds – Low risk alternative
- Meets – Medium risk alternative
- Needs Improvement – High risk alternative

Table 2.37 Constructability Qualitative Ratings

Rating	Legend	Alternative(s)
<b>Exceeds</b>		<div>0 - No Build</div> <div>1 - Transportation Systems Management</div> <div>4A-1 - Canton Red (Existing)</div> <div>4B-1 - Buffington Blue (North)</div> <div>4B-2 - Buffington Green (North)</div> <div>4B-4 - Buffington Yellow (South)</div> <div>4C-1 - Macedonia Pink (North)</div> <div>4C-4 - Macedonia Orange (South)</div> <div>4D-1 - Lathemtown Blue (North)</div> <div>4D-2 - Lathemtown Green (North)</div> <div>4E-1 - Ducktown Pink (North)</div> <div>4E-3 - Ducktown Teal (South)</div> <div>4E-4 - Ducktown Orange (South)</div> <div>4F-1 - Cumming Green (Sawnee Dr.)</div> <div>4F-6 - Cumming Blue (Chamblee Gap Rd.)</div>
<b>Meets</b>		<div>2 - Widen Existing</div> <div>3A - North</div> <div>4B-3 - Buffington Red (Existing)</div> <div>4C-2 - Macedonia Teal (North)</div> <div>4C-3 - Macedonia Red (Existing)</div> <div>4D-3 - Lathemtown Red (Existing)</div> <div>4D-4 - Lathemtown Yellow (South)</div> <div>4E-2 - Ducktown Red (Existing)</div> <div>4F-2 - Cumming Yellow (Elm St.)</div> <div>4F-5 - Cumming Orange (Veterans Memorial Blvd.)</div> <div>5A - Alt 4 and SR 369</div> <div>5B - Alt 4 and Bethelview</div>
<b>Needs Improvement</b>		<div>3B - South</div> <div>4F-3 - Cumming Red (Existing)</div> <div>4F-4 - Cumming Pink (Tolbert St.)</div>

\* Note: The lengths for Alternative 4 will be determined after various links are analyzed in subsequent analyses. The shortest distance for Alternative 4 would be 23.20 miles and the longest distance would be 25.43 miles.



SR 20 Improvements (Canton to Cumming)			Alternatives								
PF's: 0003681, 0002862, 0003682			0. No Build	1. Transportation Systems Mgmt (Localized Improvements)	1. Qualitative	2. Urban Existing	2. Qualitative	3A New Location (North)	3A. Qualitative	3B. New Location (South)	3B. Qualitative
Screen 2 Performance Criteria	Units										
Performance	Travel Time Savings (2040)	Minutes (Total)	total congested trip time 197 minutes	qualitative	F	reduced by 72 minutes	E	reduced by 67 minutes	E	reduced by 77 minutes	E
	User Benefits	Hours of Delay (Total)	11,200 cumulative hours of delay	qualitative	F	reduced by 2,700	M	reduced by 6,000	E	reduced by 7,200	E
		Fuel Saved (per capita)	Cumulative consumption 510 gallons	qualitative	F	88.2	E	94.5	E	101.6	E
	Level of Service (2040)	Volume / Capacity Ratio (V/C)	1.03	qualitative	F	0.91	F	0.95	F	0.89	F
	Travel Time Index (2040)	Free Flow/ Congested Travel Time	2.28	qualitative	F	1.85	M	1.94	M	1.80	M
	Access to Employment Centers (2040)	# of Origin / Destination (O/D) Trips in Canton/Cumming Only	320,400 total trips	qualitative	F	318,200	M	318,300	F	317,500	F
	Access management	Qualitative	F	F	F	M	M	M	M	M	
	Safety	Qualitative	F	F	F	M	M	M	M	M	
Overall Performance		Qualitative	F	F	F	F	F	F	F		
Potential Environmental and Community Impacts	Streams	Linear Feet (Linear Feet/mile)	0	0	E	11985.3 (491.8)	M	35794.9 (1583.1)	Ni	39834 (1921.6)	Ni
	Wetlands	Acres (Acres/mile)	0	0	E	2.1 (0.1)	M	4.9 (0.2)	M	19.3 (0.9)	Ni
	Lakes & Ponds	Acres (Acres/mile)	0	0	E	1.4 (0.1)	M	2.2 (0.1)	M	6.9 (0.3)	Ni
	Floodplains	Acres (Acres/mile)	0	0	E	26.8 (1.2)	M	128.7 (5.7)	Ni	203.4 (9.8)	Ni
	Conservation Areas/Parks/Section 4(f)	Acres (Acres/mile)	0	0	E	25.8 (1.1)	Ni	12.3 (0.5)	Ni	0 (0)	E
	Land and Water Conservation/Section 6(f)	Acres (Acres/mile)	0	0	E	4.3 (0.2)	Ni	0 (0)	E	0 (0)	E
	Protected Species Areas	Linear feet of streams with darter habitat (Linear feet of streams/mile)				2396.3 (102.2)	E	35496 (1,583.2)	Ni	22840 (1,101.8)	Ni
	Protected Species	#	0	0	E	6	Ni	6	Ni	6	Ni
	Noise Receptors	# (#/mile)	0 (0)	0 (0)	E	879 (42.3)	M	287 (12.7)	E	825 (39.8)	M
	Environmental Justice Population (Low-Income)	% low-income block groups of total block groups intersected by alternative	0	0	E	43.9%	M	60.0%	Ni	31.3%	M
	Environmental Justice Population (Minority)	% minority block groups of total blockgroups intersected by alternative	0	0	E	34.8%	M	33.3%	M	37.5%	M
	Farmland	Acres (Acres/mile)	0 (0)	0 (0)	E	338.3 (8.4)	E	364.5 (17.1)	M	492.4 (23.8)	Ni
	Number of Displacements	# of Structures (#/mile)	0 (0)	0 (0)	E	319 (12.3)	M	287 (12.7)	E	825 (39.8)	M
	Residential	# of Structures	0	0		415		251		770	
	Commercial	# of Structures	0	0		523		32		50	
	Industrial	# of Structures	0	0		6		0		0	
	Institutional	# of Structures	0	0		38		4		5	
	Potential Historic Properties/Section 4(f)	# of properties with structures over 45 years of age (acres)/#(mile)	0 (0) / (0)	0 (0) / (0)	E	496 (214.22) / (17.5)	Ni	64 (392.75) / (2.83)	M	84 (357.27) / (4.05)	M
	Potential Archaeological Sites/Section 4(f)	# of pre-recorded archaeological sites	0	0	E	4	M	2	Ni	5	M
	Cemeteries	#	0	0	E	2	Ni	0	E	2	Ni
	Native American Interests	#	0	0	E	6	E	2	Ni	3	M
	Air Quality	Qualitative	N/A		M		M		M		M
	Indirect and Cumulative Effects	Qualitative	M		M		M		M		M
	Construction Impacts	Qualitative	E		E		Ni		E		M
	Mitigation / Avoidance Potential (\$Million)*	Qualitative	E (0)		E (0)		Ni (2.8)		M (8.3)		M (9.1)
Overall Impacts		Qualitative	E		E		M		M		M
Costs/ Other	Total Costs	\$ (Million)	0 (E)	2.82	E	200.22	M	616.42	Ni	630.86	Ni
	Right of Way (250')	\$ (Million)	0	N/A		137.1		94		88.9	
	Construction	\$ (Million)	0	2.3		142.8		521.7		541.3	
	Operations & Maintenance	\$ (Million) /year	0.52	0.52		0.52		0.72		0.66	
	Benefit/Cost Ratio	B/C	Ni	qualitative	E	1.0	M	2.3	M	2.5	M
	Constructability	Qualitative	E		E		M		M		Ni
	Marginal Utility	Qualitative	Ni		Ni		M		Ni		Ni
	Overall Costs	Qualitative	Ni		Ni		M		Ni		Ni
Overall		Qualitative	F		F		M		F		F

Legend:  
E- Exceeds; M- Meets; Ni- Needs Improvement  
\* Environmental Mitigation (Wetlands/Streams only)/ Individual Permit Anticipated on 2, 3A/B, 4A-F, 5A/B  
Distances of Alternatives:  
0 = 0 miles; 1 = 300 ft from intersections; 2 = 23.1 miles;  
3A = 22.6 miles; 3B = 20.7 miles;  
4A = 1.6 miles; 4B-1 = 3.59 miles; 4B-2 = 3.57 miles; 4B-3 = 3.62 miles; 4B-4 = 3.70 miles;  
4C-1 = 3.05 miles; 4C-2 = 3.10 miles; 4C-3 = 2.91 miles; 4C-4 = 3.03 miles;  
4D-1 = 4.25 miles; 4D-2 = 4.47 miles; 4D-3 = 4.49 miles; 4D-4 = 4.61 miles;  
4E-1 = 4.56 miles; 4E-2 = 4.51 miles; 4E-3 = 4.65 miles; 4E-4 = 4.78 miles;  
4F-1 = 6.67 miles; 4F-2 = 6.47 miles; 4F-3 = 7.64 miles; 4F-4 = 7.27 miles; 4F-5 = 7.48 miles;  
4F-6 = 6.36 miles;  
5A = 24.3 miles; 5B = 22.9 miles (environmental analysis length of 16.65 miles)  
\*\*Note- The lengths for Alternative 4 will be determined after various links are analyzed in subsequent analyses. The shortest distance for Alternative 4 would be 23.20 miles and the longest distance would be 25.43 miles.

Legend:

E- Exceeds; M- Meets; Ni- Needs Improvement

\* Environmental Mitigation (Wetlands/Streams only)/ Individual Permit Anticipated on 2, 3A/B, 4A-F, 5A/B

Distances of Alternatives:

0 = 0 miles; 1 = 300 ft from intersections; 2 = 23.1 miles;

3A = 22.6 miles; 3B = 20.7 miles;

4A = 1.6 miles; 4B-1 = 3.59 miles; 4B-2 = 3.57 miles; 4B-3 = 3.62 miles; 4B-4 = 3.70 miles;

4C-1 = 3.05 miles; 4C-2 = 3.10 miles; 4C-3 = 2.91 miles; 4C-4 = 3.03 miles;

4D-1 = 4.25 miles; 4D-2 = 4.47 miles; 4D-3 = 4.49 miles; 4D-4 = 4.61 miles;

4E-1 = 4.56 miles; 4E-2 = 4.51 miles; 4E-3 = 4.65 miles; 4E-4 = 4.78 miles;

4F-1 = 6.67 miles; 4F-2 = 6.47 miles; 4F-3 = 7.64 miles; 4F-4 = 7.27 miles; 4F-5 = 7.48 miles;

4F-6 = 6.36 miles;

5A = 24.3 miles; 5B = 22.9 miles (environmental analysis length of 16.65 miles)

\*\*Note: The lengths for Alternative 4 will be determined after various links are analyzed in subsequent analyses. The shortest distance for Alternative 4 would be 23.20 miles and the longest distance would be 25.43 miles.



SR 20 Improvements (Canton to Cumming)			Alternatives									
PI's: 0003681, 0002862, 0003682			4A. Canton		4B. Buffington							
Screen 2 Performance Criteria		Units	4A. 1475 to South Road	4A. Qualitative	4B-1. Blue (North)	4B-1. Qualitative	4B-2. Green (North)	4B-2. Qualitative	4B-3. Red (Fading)	4B-3. Qualitative	4B-4. Yellow (South)	4B-4. Qualitative
Performance	Travel Time Savings (2040)	Minutes (Total)	reduced by 0 minutes	F	reduced by 27 minutes	E	reduced by 27 minutes	E	reduced by 22 minutes	E	reduced by 27 minutes	E
	User Benefits	Hours of Delay (Total)	increased by 100	F	reduced by 2,300	E	reduced by 2,300	E	reduced by 1,800	E	reduced by 2,300	E
		Fuel Saved (per capita)	0.3	M	13	M	13.3	M	14.2	M	13.3	M
	Level of Service (2040)	Volume / Capacity Ratio (V/C)	0.63	M	0.46	M	0.46	M	0.8	M	0.46	M
	Travel Time Index (2040)	Free Flow/ Congested Travel Time	1.48	M	1.11	E	1.11	E	1.5	M	1.11	E
	Access to Employment Centers (2040)	# of Origin / Destination (O/D) Trips in Canton/Cumming Only	335,000	M	335,800	E	335,800	E	335,000	E	335,800	E
	Access management	Qualitative	M	M	M	M	M	M	M	M	M	M
	Safety	Qualitative	M	M	M	M	M	M	M	M	M	M
Overall Performance		Qualitative		F		M		M		M		M
Potential Environmental and Community Impacts	Streams	Linear Feet (Linear Feet/mile)	0 (0)	E	3328.4 (927.1)	M	2378.2 (666.3)	M	0 (0)	E	1696.5 (458.5)	M
	Wetlands	Acres (Acres/mile)	0 (0)	E	0 (0)	E	0 (0)	E	0 (0)	E	0 (0)	E
	Lakes & Ponds	Acres (Acres/mile)	0 (0)	E	0 (0)	E	0 (0)	E	0 (0)	E	0 (0)	E
	Floodplains	Acres (Acres/mile)	0 (0)	E	0 (0)	E	0 (0)	E	0 (0)	E	0 (0)	E
	Conservation Areas/Parks/Section 4(f)	Acres (Acres/mile)	0 (0)	E	0 (0)	E	0 (0)	E	4,311.75	Ni	0.9 (0.3)	Ni
	Land and Water Conservation/Section 6(f)	Acres (Acres/mile)	0 (0)	E	0 (0)	E	0 (0)	E	4,311.75	Ni	0.9 (0.3)	Ni
	Protected Species Areas	Linear feet of streams with darter habitat (Linear feet of streams/mile)	0 (0)	E	3328 (927)	Ni	2379 (666.3)	M	0 (0)	E	1697 (458.6)	M
	Protected Species	#	0	E	6	Ni	6	Ni	0	Ni	6	Ni
	Noise Receptors	# (#/mile)	63 (38.4)	M	64 (23.4)	M	91 (25.5)	M	119 (32.5)	M	90 (24.3)	M
	Environmental Justice Population (Low-Income)	% low-income block groups of total block groups intersected by alternative	50.0%	M	50.0%	M	50.0%	M	50.0%	M	50.0%	M
	Environmental Justice Population (Minority)	% minority block groups of total blockgroups intersected by alternative	100.0%	Ni	50.0%	M	50.0%	M	50.0%	M	50.0%	M
	Farmland	Acres (Acres/mile)	27.66 (17.0)	Ni	35.5 (9.9)	E	36.8 (10.3)	E	36.3 (10.2)	E	35.3 (9.5)	E
	Number of Displacements	# of Structures (#/mile)	63 (38.4)	M	84 (23.4)	M	91 (25.5)	M	119 (32.5)	M	90 (24.3)	M
	Residential	# of Structures	52		64		71		81		78	
	Commercial	# of Structures	11		19		19		13		9	
	Industrial	# of Structures	0		0		0		0		0	
	Institutional	# of Structures	0		1		1		0		3	
	Potential Historic Properties/Section 4(f)	# of properties with structures over 45 years of age (acres)/(#/mile)	18 (39.23) / (8.15)	Ni	52 (104.3) / (14.48)	M	62 (127.5) / (17.37)	M	74 (177.3) / (29.44)	M	39 (82.4) / (10.54)	M
	Potential Archaeological Sites/Section 4(f)	# of pre-recorded archaeological sites	1	M	0	E	0	E	0	E	0	E
	Cemeteries	#	0	E	0	E	0	E	0	E	0	E
	Native American Interests	#	0	E	0	E	0	E	0	E	0	E
	Air Quality	Qualitative		M		M		M		M		M
	Indirect and Cumulative Effects	Qualitative		M		M		M		M		M
	Construction Impacts	Qualitative		Ni		E		E		M		E
	Mitigation / Avoidance Potential (\$Million)*	Qualitative		Ni		M		E		M		M
Overall Impacts		Qualitative		M		Ni		Ni		M		M
Costs/ Other	Total Costs	\$ (Million)	15.25	E	50.69	M	55.99	M	44.33	M	54.79	M
	Right of Way (250')	\$ (Million)	8.4		15.8		21.8		23.1		18.9	
	Construction	\$ (Million)	8.8		34.8		34.1		21.2		35.8	
	Operations & Maintenance	\$ (Million) /year	0.96		0.09		0.09		0.08		0.09	
	Benefit/Cost Ratio	B/C	0.1	Ni	5.1	E	4.6	E	4.3	E	4.9	E
	Constructability	Qualitative		E		E		E		M		E
	Marginal Utility	Qualitative		M		M		M		M		M
	Overall Costs	Qualitative		M		M		M		M		M
Overall		Qualitative		M		F		F		M		M
Legend:												
E- Exceeds; M- Meets; Ni- Needs Improvement												
* Environmental Mitigation (Wetlands/Streams only)/ Individual Permit Anticipated on 2, 3A/8, 4A-F, 5A/8												
Distances of Alternatives:												
0 = 0 miles; 1 = 300 ft from intersections; 2 = 23.1 miles;												
3A = 22.6 miles; 3B = 20.7 miles;												
4A = 1.6 miles; 4B-1 = 3.59 miles; 4B-2 = 3.57 miles; 4B-3 = 3.62 miles; 4B-4 = 3.70 miles;												
4C-1 = 3.05 miles; 4C-2 = 3.10 miles; 4C-3 = 2.91 miles; 4C-4 = 3.03 miles;												
4D-1 = 4.25 miles; 4D-2 = 4.47 miles; 4D-3 = 4.49 miles; 4D-4 = 4.61 miles;												
4E-1 = 4.36 miles; 4E-2 = 4.51 miles; 4E-3 = 4.65 miles; 4E-4 = 4.78 miles;												
4F-1 = 6.67 miles; 4F-2 = 6.47 miles; 4F-3 = 7.64 miles; 4F-4 = 7.27 miles; 4F-5 = 7.48 miles;												
4F-6 = 6.36 miles;												
5A = 24.3 miles; 5B = 22.9 miles (environmental analysis length of 16.65 miles)												
**Note- The lengths for Alternative 4 will be determined after various links are analyzed in subsequent analyses. The shortest distance for Alternative 4 would be 23.20 miles and the longest distance would be 25.43 miles.												

Legend:

E- Exceeds; M- Meets; Ni- Needs Improvement

\* Environmental Mitigation (Wetlands/Streams only)/ Individual Permit Anticipated on 2, 3A/B, 4A-F, 5A/B

Distances of Alternatives:

0 = 0 miles; 1 = 300 ft from intersections; 2 = 23.1 miles;

3A = 22.6 miles; 3B = 20.7 miles;

4A = 1.6 miles; 4B-1 = 3.59 miles; 4B-2 = 3.57 miles; 4B-3 = 3.62 miles; 4B-4 = 3.70 miles;

4C-1 = 3.05 miles; 4C-2 = 3.10 miles; 4C-3 = 2.91 miles; 4C-4 = 3.03 miles;

4D-1 = 4.25 miles; 4D-2 = 4.47 miles; 4D-3 = 4.49 miles; 4D-4 = 4.61 miles;

4E-1 = 4.56 miles; 4E-2 = 4.51 miles; 4E-3 = 4.65 miles; 4E-4 = 4.78 miles;

4F-1 = 6.67 miles; 4F-2 = 6.47 miles; 4F-3 = 7.64 miles; 4F-4 = 7.27 miles; 4F-5 = 7.48 miles;

4F-6 = 6.36 miles;

5A = 24.3 miles; 5B = 22.9 miles (environmental analysis length of 16.65 miles)

\*\*Note: The lengths for Alternative 4 will be determined after various links are analyzed in subsequent analyses. The shortest distance for Alternative 4 would be 23.20 miles and the longest distance would be 25.43 miles.





SR 20 Improvements (Canton to Cumming)			Alternatives							
PFs: 0003681, 0002862, 0003682			4C: Macedonia							
Screen 2 Performance Criteria	Units		4C-1 Pink (North)	4C-1 Qualitative	4C-2 Blue (North)	4C-2 Qualitative	4C-3 Red (East)	4C-3 Qualitative	4C-4 Orange (South)	4C-4 Qualitative
Performance	Travel Time Savings (2040)	Minutes (Total)	reduced by 11 minutes	M	reduced by 11 minutes	M	reduced by 8 minutes	F	reduced by 11 minutes	M
	User Benefits	Hours of Delay (Total)	reduced by 800	E	reduced by 800	E	reduced by 50	F	reduced by 800	E
		Fuel Saved (per capita)	16.3	M	15.8	M	19.3	M	16.5	M
	Level of Service (2040)	Volume / Capacity Ratio (V/C)	0.86	F	0.86	F	1.03	F	0.86	F
	Travel Time Index (2040)	Free Flow/ Congested Travel Time	1.71	M	1.71	M	1.85	M	1.71	M
	Access to Employment Centers (2040)	# of Origin / Destination (O/D) Trips in Canton/Cumming Only	335,100	M	335,100	M	335,270	M	335,100	M
	Access management	Qualitative	M		M		M		M	
	Safety	Qualitative	M		M		M		M	
	Overall Performance	Qualitative	F		F		F		F	
Potential Environmental and Community Impacts	Streams	Linear Feet (Linear Feet/mile)	3670.0 (1203.3)	NI	1027.7 (331.5)	M	132.8 (43.2)	E	1350.8 (445.6)	M
	Wetlands	Acres (Acres/mile)	0 (0)	E	0 (0.0)	E	0 (0.0)	E	0 (0.0)	E
	Lakes & Ponds	Acres (Acres/mile)	0.4 (0.13)	M	0.1 (0.05)	M	0.2 (0.1)	M	0 (0)	E
	Floodplains	Acres (Acres/mile)	1.8 (0.6)	M	1.8 (0.6)	M	1.8 (0.6)	M	1.8 (0.6)	M
	Conservation Areas/Parks/Section 4(f)	Acres (Acres/mile)	36.6 (11.8)	NI	35.7 (11.5)	NI	35.3 (11.5)	NI	0 (0)	E
	Land and Water Conservation/Section 6(f)	Acres (Acres/mile)	0 (0)	E	0 (0)	E	0 (0)	E	0 (0)	E
	Protected Species Areas	Linear feet of streams with darter habitat (Linear feet of streams/mile)	3670 (1,203.3)	NI	1028 (331.5)	M	103 (35.2)	E	1351 (445.6)	M
	Protected Species	#	6	NI	6	NI	8	NI	6	NI
	Noise Receptors	# (#/mile)	60 (19.7)	M	76 (24.5)	M	106 (35.7)	M	93 (30.8)	M
	Environmental Justice Population (Low-Income)	% low-income block groups of total block groups intersected by alternative	33.3%	M	33.3%	M	33.3%	M	33.3%	M
	Environmental Justice Population (Minority)	% minority block groups of total blockgroups intersected by alternative	0.0%	E	0.0%	E	0.0%	E	0.0%	E
	Farmland	Acres (Acres/mile)	55.5 (19.2)	M	45.6 (14.7)	M	11.7 (3.7)	E	58.6 (19.3)	M
	Number of Displacements	# of Structures (#/mile)	60 (19.7)	M	76 (24.5)	M	106 (35.7)	M	93 (30.8)	M
	Residential	# of Structures	52		65		71		85	
	Commercial	# of Structures	7		10		13		6	
	Industrial	# of Structures	0		0		0		0	
	Institutional	# of Structures	1		1		3		2	
	Potential Historic Properties/Section 4(f)	# of properties with structures over 45 years of age (acres)/(#/mile)	25 (85.8) / (8.28)	M	48 (15.48)	M	55 (18.84)	M	18 (58.8) / (5.94)	M
	Potential Archaeological Sites/Section 4(f)	# of pre-recorded archaeological sites	1	M	1	M	1	M	0	E
	Cemeteries	#	0	E	0	E	1	NI	0	E
	Native American Interests	#	0	E	0	E	0	F	0	E
	Air Quality	Qualitative		M		M		M		M
	Indirect and Cumulative Effects	Qualitative		M		M		M		M
	Construction Impacts	Qualitative		E		E		NI		E
	Mitigation / Avoidance Potential (\$Million)*	Qualitative		M		M		NI		M
	Overall Impacts	Qualitative		NI		NI		M		M
Costs/ Other	Total Costs	\$ (Million)	47.37	M	49.97	M	39.07	E	39.67	E
	Right of Way (250')	\$ (Million)	18.7		23.9		23.9		12.5	
	Construction	\$ (Million)	28.6		26		15.1		27.1	
	Operations & Maintenance	\$ (Million) /year	0.07		0.07		0.07		0.07	
	Benefit/Cost Ratio	B/C	3.8	M	3.2	M	3.8	NI	3.9	E
	Constructability	Qualitative		E		M		M		E
	Marginal Utility	Qualitative		M		M		NI		M
	Overall Costs	Qualitative		M		M		M		M
	Overall	Qualitative		F		F		M		M

Legend:

E- Exceeds; M- Meets; NI- Needs Improvement

\* Environmental Mitigation (Wetlands/Streams only)/ Individual Permit Anticipated on 2, 3A/B, 4A-F, 5A/B

Distances of Alternatives:

0 = 0 miles; 1 = 300 ft from intersections; 2 = 23.1 miles;

3A = 22.6 miles; 3B = 20.7 miles;

4A = 1.6 miles; 4B-1 = 3.59 miles; 4B-2 = 3.57 miles; 4B-3 = 3.62 miles; 4B-4 = 3.70 miles;

4C-1 = 3.05 miles; 4C-2 = 3.10 miles; 4C-3 = 2.91 miles; 4C-4 = 3.03 miles;

4D-1 = 4.25 miles; 4D-2 = 4.47 miles; 4D-3 = 4.49 miles; 4D-4 = 4.61 miles;

4E-1 = 4.56 miles; 4E-2 = 4.51 miles; 4E-3 = 4.65 miles; 4E-4 = 4.78 miles;

4F-1 = 6.67 miles; 4F-2 = 6.47 miles; 4F-3 = 7.64 miles; 4F-4 = 7.27 miles; 4F-5 = 7.48 miles;

4F-6 = 6.36 miles;

5A = 24.3 miles; 5B = 22.9 miles (environmental analysis length of 16.65 miles)

\*\*Note: The lengths for Alternative 4 will be determined after various links are analyzed in subsequent analyses. The shortest distance for Alternative 4 would be 23.20 miles and the longest distance would be 25.43 miles.



SR 20 Improvements (Canton to Cumming)			Alternatives							
PFs: 0003681, 0002862, 0003682			4D. Lathetown							
Screen 2 Performance Criteria	Units		4D-1 Blue (North)	4D-1 Qualitative	4D-2 Green (North)	4D-2 Qualitative	4D-3 Red (East)	4D-3 Qualitative	4D-4 Yellow (South)	4D-4 Qualitative
Performance	Travel Time Savings (2040)	Minutes (Total)	reduced by 20 minutes	M	reduced by 20 minutes	M	reduced by 14 minutes	M	reduced by 20 minutes	M
	User Benefits	Hours of Delay (Total)	reduced by 1,600	E	reduced by 1,600	E	reduced by 500	F	reduced by 1,600	E
		Fuel Saved (per capita)	16.8	M	14.6	M	20.8	M	13.2	M
	Level of Service (2040)	Volume / Capacity Ratio (V/C)	0.79	M	0.79	M	0.88	F	0.79	M
	Travel Time Index (2040)	Free Flow/ Congested Travel Time	1.48	M	1.48	M	1.71	M	1.48	M
	Access to Employment Centers (2040)	# of Origin / Destination (O/D) Trips in Canton/Cumming Only	334,800	M	334,800	M	335,200	M	334,800	M
	Access management	Qualitative		M		M		M		M
	Safety	Qualitative		M		M		M		M
	Overall Performance	Qualitative		M		M		F		M
Potential Environmental and Community Impacts	Streams	Linear Feet (Linear Feet/mile)	2676.6 (676.8)	M	2228.0 (498.4)	M	2194.5 (488.0)	M	2412.0 (523.2)	M
	Wetlands	Acres (Acres/mile)	0 (0.0)	E	0 (0.0)	E	0 (0.0)	E	0 (0.0)	E
	Lakes & Ponds	Acres (Acres/mile)	0.8 (0.2)	M	1.3 (0.3)	NI	0.4 (0.1)	M	1.9 (0.4)	NI
	Floodplains	Acres (Acres/mile)	13.4 (3.1)	NI	12.0 (2.7)	NI	10.7 (2.4)	NI	10.7 (2.3)	NI
	Conservation Areas/Parks/Section 4(f)	Acres (Acres/mile)	0 (0)	E	0 (0)	E	0 (0)	E	0 (0)	E
	Land and Water Conservation/Section 6(f)	Acres (Acres/mile)	0 (0)	E	0 (0)	E	0 (0)	E	0 (0)	E
	Protected Species Areas	Linear feet of streams with darter habitat (Linear feet of streams/mile)	2877 (676.8)	M	2228 (498.4)	M	2190 (488.0)	M	2412 (523.2)	M
	Protected Species	#	6	NI	6	NI	8	NI	6	NI
	Noise Receptors	# (#/mile)	72 (16.9)	E	93 (20.8)	M	143 (32.7)	M	100 (21.7)	M
	Environmental Justice Population (Low-Income)	% low-income block groups of total block groups intersected by alternative	25.0%	E	25.0%	E	20.0%	E	20.0%	E
	Environmental Justice Population (Minority)	% minority block groups of total blockgroups intersected by alternative	0.0%	E	0.0%	E	0.0%	E	0.0%	E
	Farmland	Acres (Acres/mile)	74.3 (17.5)	M	68.1 (15.2)	M	43.2 (9.6)	E	49.9 (10.8)	E
	Number of Displacements	# of Structures (#/mile)	72 (16.9)	E	93 (20.8)	M	143 (32.7)	M	100 (21.7)	M
	Residential	# of Structures	56		69		36		63	
	Commercial	# of Structures	15		22		63		36	
	Industrial	# of Structures	0		1		2		0	
	Institutional	# of Structures	1		1		5		1	
	Potential Historic Properties/Section 4(f)	# of properties with structures over 45 years of age (acres)/(#/mile)	41 (136.4) / (9.65)	M	53 (132.6) / (11.86)	M	80 (156.0) / (35.94)	M	63 (147.1) / (13.67)	M
	Potential Archaeological Sites/Section 4(f)	# of pre-recorded archaeological sites	3	M	1	M	1	M	2	M
	Cemeteries	#	0	E	0	E	1	NI	0	E
	Native American Interests	#	3	M	1	M	4	M	1	M
	Air Quality	Qualitative		M		M		M		M
	Indirect and Cumulative Effects	Qualitative		M		M		M		M
	Construction Impacts	Qualitative		E		M		M		M
	Mitigation / Avoidance Potential (\$Million)*	Qualitative		M		M		M		M
	Overall Impacts	Qualitative		M		NI		M		NI
Costs/ Other	Total Costs	\$ (Million)	65.4	M	75.8	M	65.98	M	60	M
	Right of Way (250')	\$ (Million)	26.7		36		43.7		22.5	
	Construction	\$ (Million)	38.6		39.7		25.7		37.4	
	Operations & Maintenance	\$ (Million) /year	0.1		0.1		0.05		0.1	
	Benefit/Cost Ratio	B/C	3.3	M	3	M	1.8	NI	3.9	E
	Constructability	Qualitative		E		E		M		M
	Marginal Utility	Qualitative		M		M		NI		M
	Overall Costs	Qualitative		M		M		M		M
Overall		Qualitative		M		F		M		F

Legend:

E- Exceeds; M- Meets; NI- Needs Improvement

\* Environmental Mitigation (Wetlands/Streams only)/ Individual Permit Anticipated on 2, 3A/B, 4A-F, 5A/B

Distances of Alternatives:

0 = 0 miles; 1 = 300 ft from intersections; 2 = 23.1 miles;

3A = 22.6 miles; 3B = 20.7 miles;

4A = 1.6 miles; 4B-1 = 3.59 miles; 4B-2 = 3.57 miles; 4B-3 = 3.62 miles; 4B-4 = 3.70 miles;

4C-1 = 3.05 miles; 4C-2 = 3.10 miles; 4C-3 = 2.91 miles; 4C-4 = 3.03 miles;

4D-1 = 4.25 miles; 4D-2 = 4.47 miles; 4D-3 = 4.49 miles; 4D-4 = 4.61 miles;

4E-1 = 4.56 miles; 4E-2 = 4.51 miles; 4E-3 = 4.65 miles; 4E-4 = 4.78 miles;

4F-1 = 6.67 miles; 4F-2 = 6.47 miles; 4F-3 = 7.64 miles; 4F-4 = 7.27 miles; 4F-5 = 7.48 miles;

4F-6 = 6.36 miles;

5A = 24.3 miles; 5B = 22.9 miles (environmental analysis length of 16.65 miles)

\*\*Note: The lengths for Alternative 4 will be determined after various links are analyzed in subsequent analyses. The shortest distance for Alternative 4 would be 23.20 miles and the longest distance would be 25.43 miles.





SR 20 Improvements (Canton to Cumming)			Alternatives							
PFs: 0003681, 0002862, 0003682			4E, Ducktown							
Screen 2 Performance Criteria	Units		4E-1 Pink (North)	4E-1 Qualitative	4E-2 Red (East)	4E-2 Qualitative	4E-3 Blue (South)	4E-3 Qualitative	4E-4 Orange (South)	4E-4 Qualitative
Performance	Travel Time Savings (2040)	Minutes (Total)	reduced by 8 minutes	F	reduced by 5 minutes	F	reduced by 8 minutes	F	reduced by 8 minutes	F
	User Benefits	Hours of Delay (Total)	reduced by 500	F	reduced by 100	F	reduced by 500	F	reduced by 500	F
		Fuel Saved (per capita)	15.2	M	18.2	M	14.1	M	12.4	M
	Level of Service (2040)	Volume / Capacity Ratio (V/C)	0.73	M	0.83	M	0.73	M	0.73	M
	Travel Time Index (2040)	Free Flow/ Congested Travel Time	1.40	M	1.40	M	1.40	M	1.40	M
	Access to Employment Centers (2040)	# of Origin / Destination (O/D) Trips in Canton/Cumming Only	335,000	M	335,000	M	335,000	M	335,000	M
	Access management	Qualitative	M		M		M		M	
	Safety	Qualitative	M		M		M		M	
	Overall Performance	Qualitative	F		F		F		F	
	Streams	Linear Feet (Linear Feet/mile)	5762.9 (1263.8)	NI	5744.3 (1263.8)	M	5503.1 (1193.5)	NI	4650.2 (972.84)	M
Potential Environmental and Community Impacts	Wetlands	Acres (Acres/mile)	0 (0.0)	E	0 (0.0)	E	0 (0.0)	E	0 (0.0)	E
	Lakes & Ponds	Acres (Acres/mile)	0 (0)	E	0.0 (0.2)	M	3.4 (0.7)	NI	3.3 (0.7)	NI
	Floodplains	Acres (Acres/mile)	8.8 (1.5)	M	2.0 (0.4)	M	8.3 (1.8)	M	12.6 (2.6)	NI
	Conservation Areas/Parks/Section 4(f)	Acres (Acres/mile)	0 (0)	E	0 (0)	E	0 (0)	E	0 (0)	E
	Land and Water Conservation/Section 6(f)	Acres (Acres/mile)	0 (0)	E	0 (0)	E	0 (0)	E	0 (0)	E
	Protected Species Areas	Linear feet of streams with darter habitat (Linear feet of streams/mile)	4729 (1,037.1)	NI	314 (60.7)	E	144 (31)	E	144 (30.1)	E
	Protected Species	#	6	NI	6	NI	6	NI	6	NI
	Noise Receptors	# (#/mile)	133 (29.2)	M	152 (33.7)	M	131 (28.2)	M	124 (25.9)	M
	Environmental Justice Population (Low-Income)	% low-income block groups of total block groups intersected by alternative	50.0%	M	42.9%	M	50.0%	M	50.0%	M
	Environmental Justice Population (Minority)	% minority block groups of total blockgroups intersected by alternative	0.0%	E	0.0%	E	0.0%	E	0.0%	E
	Farmland	Acres (Acres/mile)	65.5 (14.4)	M	48.4 (10.3)	E	102.1 (22.0)	NI	68.9 (14.4)	M
	Number of Displacements	# of Structures (#/mile)	133 (29.2)	M	152 (33.7)	M	131 (28.2)	M	124 (25.9)	M
	Residential	# of Structures	86		92		117		114	
	Commercial	# of Structures	40		61		12		8	
	Industrial	# of Structures	2		1		1		1	
	Institutional	# of Structures	5		5		1		1	
	Potential Historic Properties/Section 4(f)	# of properties with structures over 45 years of age (acres)/(#/mile)	77 (171.17) (18.89)	M	95 (190.49) (21.06)	M	31 (6.87) (6.87)	M	31 (110.17) (6.49)	M
	Potential Archaeological Sites/Section 4(f)	# of pre-recorded archaeological sites	1	M	1	M	0	E	1	NI
	Cemeteries	#	1	NI	1	NI	0	E	0	E
	Native American Interests	#	0	E	0	E	0	E	0	E
	Air Quality	Qualitative		M		M		M		M
	Indirect and Cumulative Effects	Qualitative		M		M		M		M
	Construction Impacts	Qualitative		E		E		M		M
	Mitigation / Avoidance Potential (\$Million)*	Qualitative		E (1.1)		E (1.1)		E (1.7)		E (1.1)
	Overall Impacts	Qualitative		M		M		NI		NI
Costs/ Other	Total Costs	\$ (Million)	75.4	M	54.96	M	73.81	M	85.81	NI
	Right of Way (250')	\$ (Million)	39.1		34.5		24.5		32.6	
	Construction	\$ (Million)	36.2		20.1		49.2		53.1	
	Operations & Maintenance	\$ (Million) /year	0.1		0.08		0.11		0.11	
	Benefit/Cost Ratio	B/C	2.8	M	1.8	M	2.9	M	2.6	M
	Constructability	Qualitative		E		M		E		E
	Marginal Utility	Qualitative		M		M		M		M
	Overall Costs	Qualitative		M		M		M		M
	Overall	Qualitative		M		M		F		F

Legend:

E- Exceeds; M- Meets; NI- Needs Improvement

\* Environmental Mitigation (Wetlands/Streams only)/ Individual Permit Anticipated on 2, 3A/B, 4A-F, 5A/B

Distances of Alternatives:

0 = 0 miles; 1 = 300 ft from intersections; 2 = 23.1 miles;

3A = 22.6 miles; 3B = 20.7 miles;

4A = 1.6 miles; 4B-1 = 3.59 miles; 4B-2 = 3.57 miles; 4B-3 = 3.62 miles; 4B-4 = 3.70 miles;

4C-1 = 3.05 miles; 4C-2 = 3.10 miles; 4C-3 = 2.91 miles; 4C-4 = 3.03 miles;

4D-1 = 4.25 miles; 4D-2 = 4.47 miles; 4D-3 = 4.49 miles; 4D-4 = 4.61 miles;

4E-1 = 4.56 miles; 4E-2 = 4.51 miles; 4E-3 = 4.65 miles; 4E-4 = 4.78 miles;

4F-1 = 6.67 miles; 4F-2 = 6.47 miles; 4F-3 = 7.64 miles; 4F-4 = 7.27 miles; 4F-5 = 7.48 miles;

4F-6 = 6.36 miles;

5A = 24.3 miles; 5B = 22.9 miles (environmental analysis length of 16.65 miles)

\*\*Note: The lengths for Alternative 4 will be determined after various links are analyzed in subsequent analyses. The shortest distance for Alternative 4 would be 23.20 miles and the longest distance would be 25.43 miles.





SR 20 Improvements (Canton to Cumming)			Alternatives											
PFs: 0003681, 0002862, 0003682			4F: Cumming											
Screen 2 Performance Criteria	Units		4F-1: Green (North)	4F-1: Qualitative	4F-2: Yellow (North)	4F-2: Qualitative	4F-3: Red (East)	4F-3: Qualitative	4F-4: Pink (South)	4F-4: Qualitative	4F-5: Orange (Veterans Memorial)	4F-5: Qualitative	4F-6: Blue (South) - Chamblee Gap	4F-6: Qualitative
Performance	Travel Time Savings (2040)	Minutes (Total)	reduced by 20 minutes	M	reduced by 25 minutes	E	reduced by 25 minutes	E	reduced by 25 minutes	E	reduced by 25 minutes	E	reduced by 19 minutes	M
	User Benefits	Hours of Delay (Total)	reduced by 1,500	M	reduced by 600	F	reduced by 600	F	reduced by 600	F	reduced by 600	F	reduced by 1,700	M
		Fuel Saved (per capita)	36.1	M	31.1	M	31.1	M	31.1	M	31.1	M	41.6	M
	Level of Service (2040)	Volume / Capacity Ratio (V/C)	0.94	F	0.95	F	0.95	F	0.95	F	0.95	F	0.97	F
	Travel Time Index (2040)	Free Flow/ Congested Travel Time	1.96	F	2.01	F	2.01	F	2.01	F	2.01	F	2.09	F
	Access to Employment Centers (2040)	# of Origin / Destination (O/D) Trips in Canton/Cumming Only	335,300	M	335,200	M	335,300	M	335,200	M	335,200	M	337,900	M
	Access management	Qualitative	M	M	M	M	M	M	M	M	M	M	M	M
	Safety	Qualitative	M	M	M	M	M	M	M	M	M	M	M	M
	Overall Performance	Qualitative	F	F	F	F	F	F	F	F	F	F	F	F
Potential Environmental and Community Impacts	Streams	Linear Feet (Linear Feet/mile)	8555.7 (982.56)	M	6185.7 (956.06)	M	9181.1 (943.71)	M	11582.7 (1549.82)	Ni	8708.39 (1197.85)	Ni	9015.7 (1417.57)	Ni
	Wetlands	Acres (Acres/mile)	0 (0.0)	E	2.1 (0.32)	M	2.1 (0.27)	M	2.1 (0.28)	M	0.8 (0.11)	M	5.1 (0.8)	Ni
	Lakes & Ponds	Acres (Acres/mile)	0 (0)	E	0 (0)	E	0 (0)	E	1.2 (0.2)	M	0.5 (0.07)	M	1.0 (0.2)	M
	Floodplains	Acres (Acres/mile)	9.7 (1.5)	M	9.9 (1.5)	M	14.6 (1.8)	M	15 (2)	M	15.1 (2.1)	M	29.4 (3.2)	Ni
	Conservation Areas/Parks/Section 4(f)	Acres (Acres/mile)	11.2 (1.7)	Ni	15.7 (2.4)	Ni	11.2 (1.8)	Ni	11.2 (1.8)	Ni	11.2 (1.8)	Ni	0 (0)	E
	Land and Water Conservation/Section 6(f)	Acres (Acres/mile)	0 (0)	E	4.4 (0.7)	Ni	0 (0)	E	0 (0)	E	0 (0)	E	0 (0)	E
	Protected Species Areas	Linear feet of streams with darter habitat (Linear feet of streams/mile)	0 (0)	E	0 (0)	E	0 (0)	E	0 (0)	E	0 (0)	E	0 (0)	E
	Protected Species	#	3	Ni	3	Ni	3	Ni	3	Ni	3	Ni	3	Ni
	Noise Receptors	# (#/mile)	268 (40.2)	M	306 (47.3)	Ni	494 (59.4)	Ni	341 (46.9)	Ni	363 (48.5)	Ni	261 (41.0)	M
	Environmental Justice Population (Low-Income)	% low-income block groups of total block groups intersected by alternative	66.7%	Ni	63.6%	Ni	58.3%	Ni	66.7%	Ni	66.7%	Ni	63.6%	Ni
	Environmental Justice Population (Minority)	% minority block groups of total block groups intersected by alternative	33.3%	M	54.5%	Ni	50.5%	Ni	50.0%	M	50.0%	M	45.50%	M
	Farmland	Acres (Acres/mile)	29.7 (4.5)	E	37.5 (5.6)	E	38.2 (5.8)	E	52.4 (7.0)	M	61.3 (8.4)	E	100 (15.7)	M
	Number of Displacements	# of Structures (#/mile)	265 (40.2)	M	306 (47.3)	Ni	454 (55.4)	Ni	341 (46.9)	Ni	363 (48.5)	Ni	261 (41.0)	M
	Residential	# of Structures	177		137		106		115		194		190	
	Commercial	# of Structures	73		143		128		204		145		49	
	Industrial	# of Structures	2		2		2		7		11		15	
	Institutional	# of Structures	16		24		17		15		23		7	
	Potential Historic Properties/Section 4(f)	# of properties with structures over 45 years of age (acres)/#(mile)	72 (109.8)/ (10.79)	M	101 (156.1)	M	130 (123.21) (14.45)	Ni	83 (141.4)/ (11.42)	Ni	94 (118.3)/ (12.57)	M	37 (54.0)/ (5.82)	M
	Potential Archaeological Sites/Section 4(f)	# of pre-recorded archaeological sites	0	E	0	E	0	E	0	E	0	E	1	M
	Cemeteries	#	0	E	0	E	0	Ni	1	Ni	0	E	0	E
	Native American Interests	#	0	E	0	E	0	E	0	E	0	E	0	E
	Air Quality	Qualitative		M		M		M		M		M		M
	Indirect and Cumulative Effects	Qualitative		M		M		M		M		M		M
	Construction Impacts	Qualitative		M		Ni		Ni		Ni		Ni		M
	Mitigation / Avoidance Potential (\$Million)*	Qualitative		E		E		Ni		M		E		E
	Overall Impacts	Qualitative		M		M		M		M		M		F
Costs/ Other	Total Costs	\$ (Million)	101.59	Ni	94.68	Ni	121.11	Ni	91.97	Ni	117.52	Ni	86.88	Ni
	Right of Way (250')	\$ (Million)	45.9		47.1		79.8		49		54.8		32.1	
	Construction	\$ (Million)	55.5		47.4		59.1		42.8		62.5		54.6	
	Operations & Maintenance	\$ (Million) /year	0.19		0.18		0.21		0.17		0.22		0.18	
	Benefit/Cost Ratio	B/C	4.0	E	2.8	M	2.6	M	2.5	M	3.3	M	4.6	E
	Constructability	Qualitative		E		M		Ni		Ni		M		E
	Marginal Utility	Qualitative		M		M		M		M		M		M
	Overall Costs	Qualitative		M		M		M		M		M		M
	Overall	Qualitative		M		M		M		M		M		F

Legend:

E- Exceeds; M- Meets; Ni- Needs Improvement

\* Environmental Mitigation (Wetlands/Streams only)/ Individual Permit Anticipated on 2, 3A/B, 4A-F, 5A/B

Distances of Alternatives:

0 = 0 miles; 1 = 300 ft from intersections; 2 = 23.1 miles;

3A = 22.6 miles; 3B = 20.7 miles;

4A = 1.6 miles; 4B-1 = 3.59 miles; 4B-2 = 3.57 miles; 4B-3 = 3.62 miles; 4B-4 = 3.70 miles;

4C-1 = 3.05 miles; 4C-2 = 3.10 miles; 4C-3 = 2.91 miles; 4C-4 = 3.03 miles;

4D-1 = 4.25 miles; 4D-2 = 4.47 miles; 4D-3 = 4.49 miles; 4D-4 = 4.61 miles;

4E-1 = 4.56 miles; 4E-2 = 4.51 miles; 4E-3 = 4.65 miles; 4E-4 = 4.78 miles;

4F-1 = 6.67 miles; 4F-2 = 6.47 miles; 4F-3 = 7.64 miles; 4F-4 = 7.27 miles; 4F-5 = 7.48 miles;

4F-6 = 6.36 miles;

5A = 24.3 miles; 5B = 22.9 miles (environmental analysis length of 16.65 miles)

\*\*Note: The lengths for Alternative 4 will be determined after various links are analyzed in subsequent analyses. The shortest distance for Alternative 4 would be 23.20 miles and the longest distance would be 25.43 miles.

SR 20 Improvements (Canton to Cumming)			Alternatives			
PI's: 0003681, 0002862, 0003682			5. Alt 4 Plus Reroutings			
Screen 2 Performance Criteria	Units		SA: Widen SR 20 and Reroute onto Bethelview Rd Widened SR 369	SA: Qualitative	SB: Widen SR 20 and Reroute onto Bethelview Rd	SB: Qualitative
Performance	Travel Time Savings (2040)	Minutes (Total)	reduced by 40 minutes (majority of travel time savings come from widening of Buffington and Macedonia)	M	qualitative	M
	User Benefits	Hours of Delay (Total)	reduced by 2,700	M	qualitative	M
		Fuel Saved (per capita)	67.5	M	qualitative	M
	Level of Service (2040)	Volume / Capacity Ratio (V/C)	0.98	F	qualitative	F
	Travel Time Index (2040)	Free Flow/ Congested Travel Time	2.07	F	qualitative	F
	Access to Employment Centers (2040)	# of Origin / Destination (O/D) Trips in Canton/Cumming Only	324,600	F	qualitative	M
	Access management	Qualitative		M		F
	Safety	Qualitative		M		M
	Overall Performance	Qualitative		F		F
Potential Environmental and Community Impacts	Streams	Linear Feet (Linear Feet/mile)	14,760.3 (607.92)	M	4,197.0 (252.07)	M
	Wetlands	Acres (Acres/mile)	1.0 (0.04)	M	0.0	E
	Lakes & Ponds	Acres (Acres/mile)	2.9 (0.1)	M	1.4 (0.1)	M
	Floodplains	Acres (Acres/mile)	43.5 (1.8)	M	11.8 (0.7)	M
	Conservation Areas/Parks/Section 4(f)	Acres (Acres/mile)	15.9 (0.7)	NI	14.57 (0.88)	NI
	Land and Water Conservation/Section 6(f)	Acres (Acres/mile)	4.3 (0.2)	NI	4.28 (0.3)	NI
	Protected Species Areas	Linear feet of streams with darter habitat (Linear feet of streams/mile)	14036 (578.1)	M	2,366 (142.1)	M
	Protected Species	#	6	NI	6	NI
	Noise Receptors	# (#/mile)	567 (23.4)	M	566 (34)	M
	Environmental Justice Population (Low-Income)	% low-income block groups of total block groups intersected by alternative	23.5%	E	23.5%	E
	Environmental Justice Population (Minority)	% minority block groups of total blockgroups intersected by alternative	23.5%	E	20.0%	E
	Farmland	Acres (Acres/mile)	360.9 (16.1)	M	168.4 (10.1)	E
	Number of Displacements	# of Structures (#/mile)	567 (23.4)	M	566 (34)	M
	Residential	# of Structures	375		327	
	Commercial	# of Structures	170		216	
	Industrial	# of Structures	3		5	
	Institutional	# of Structures	19		18	
	Potential Historic Properties/Section 4(f)	# of properties with structures over 45 years of age (acres)/(#/mile)	240 (449.4) / (9.88)	M	314 (531.5) / (18.86)	M
	Potential Archaeological Sites/Section 4(f)	# of pre-recorded archaeological sites	8	NI	4	M
	Cemeteries	#	9	NI	6	NI
	Native American Interests	#	5	NI	1	NI
	Air Quality	Qualitative		M		M
	Indirect and Cumulative Effects	Qualitative		M		M
	Construction Impacts	Qualitative		M		M
	Mitigation / Avoidance Potential (\$Million)*	Qualitative		NI (2.5)		M (0.6)
	Overall Impacts	Qualitative		M		M
Costs/ Other	Total Costs	\$ (Million)	248.55	M	229.08	M
	Right of Way (250')	\$ (Million)	102		133.6	
	Construction	\$ (Million)	146		95.1	
	Operations & Maintenance	\$ (Million) /year	0.55		0.38	
	Benefit/Cost Ratio	B/C	2.3	M	qualitative	NI
	Constructability	Qualitative		M		M
	Marginal Utility	Qualitative		NI		NI
	Overall Costs	Qualitative		NI		NI
	Overall	Qualitative		F		F

Legend:

E- Exceeds; M- Meets; NI- Needs Improvement

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5A = 24.3 miles; 5B = 22.9 miles (environmental analysis length of 16.65 miles)

\*\*Note: The lengths for Alternative 4 will be determined after various links are analyzed in subsequent analyses. The shortest distance for Alternative 4 would be 23.20 miles and the longest distance would be 25.43 miles.

\*NOTE: SR 369 programmed for widening in Regional Transportation Plan and Bethelview Road to be widened under separate project

**Attachment 11**

**VE Implementation Letter**




**DEPARTMENT OF TRANSPORTATION  
STATE OF GEORGIA**

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**INTERDEPARTMENT CORRESPONDENCE**

**FILE:** Cherokee & Forsyth Co. **OFFICE:** Engineering Services  
P.I. No.: 0014131, 0014132, 0014133, 0002862, 0003682  
SR 20 from CR281/Scott Road to SR 400

**DATE:** August 2, 2017

**FROM:** Lisa L. Myers, State Project Review Engineer 

**TO:** Albert Shelby, Director of Program Delivery  
Attn.: Cleopatra James

**SUBJECT: IMPLEMENTATION OF VALUE ENGINEERING STUDY ALTERNATIVES**

The VE Study for the above projects was held February 27 thru March 2, 2017. Revised responses were received on August 1, 2017. Recommendations for implementation of Value Engineering Study Alternatives are indicated in the table below. The Project Manager shall incorporate the VE alternatives recommended for implementation to the extent reasonable in the design of the project. Please note, if the implementation of any VE recommendation requires a Design Exception and/or Design Variance, those must be requested separately.

ALT #	Description	Potential Savings/ LCC	Implement	Comments
1.0	Reduce widening from 6 to 4 lanes at Union Hill Road to SR 371.	\$23,515,000	No	The growth trends show that soon after the design year, volumes will be great enough to require 6-lanes. GDOT prefers to provide 6-lanes for consistency as well as to address the likely need so the design team will proceed with the original design.
2.0	Reduce Lane widths from 12' to 11' wide for all lanes.	\$9,484,000	No	The design team has agreed to 2.1 instead.
2.1	Reduce inner lane widths in each direction from 12' to 11' wide (outside lanes remain 12' wide).	\$6,335,000	Yes	This will be implemented.
3.0	Reduce median width from 20' to 16' wide.	\$2,730,000	No	Please review the design team's entire explanation for rejecting this idea. The narrower median suggestion would make it more difficult for large vehicles to use the Restricted Crossing U-Turns (R-Cuts). The proposed 20 foot wide median allows for landscaping in a larger green space for the current context sensitive design.

4.0	Construct rural shoulder with 10' wide overall shoulder with 4' wide partial depth pavement.	\$7,872,000	No	This corridor resides in a MS4 region and runs along a topographical ridge line. See the designer's response for more details, but a rural shoulder would not provide any containment or retention to help satisfy water quality goals of MS4.
4.1	Construct 12' wide urban shoulder in lieu of the 16' wide shoulder.	Proposed = \$5,430,000 Actual = \$1,097,730	Yes, with modifications	The designers will use this narrow shoulder option in areas to help minimize adverse impacts to adjacent resources.
7.0	Eliminate ponds at five property displacements for (PI# 0002862 & 0003682)	Proposed = \$4,150,000 Actual = \$1,245,000	Yes, with modifications	Designers will partially implement this suggestion and reduce the required ROW where feasible for the modified savings amount.
10.0	Perform detailed MS4 calculations to allow for elimination of ponds; acquire non-pond parcels first.	Proposed = \$21,755,000 Actual = \$14,503,300	Yes, with modifications	Please see the designers attached full responses for 4.0, 7.0 and 10.0 but after further analysis it is assumed that the ponds can be reduced in size which will reduce the required ROW for the modified savings amount.
12.0	Use a consistent required Right of Way width; and use permanent easement beyond.	Proposed = \$16,950,000 Actual = \$8,430,000	Yes, with modifications	This will be partially implemented for the modified savings amount.
17.0	Use Design/Build Delivery method to meet expedited schedule.	\$8,831,000	No	Time savings could be realized through this delivery method, but with the current accelerated schedule set by the GDOT Commissioner the time has already been condensed.

The Office of Engineering Services concurs with the Project Manager's responses.

Approved: Margaret B. Pirkle Date: 8.16.17  
Margaret Pirkle, PE, Chief Engineer

LLM/EAR/MJS

Attachments

Cc: Hiral Patel  
Albert Shelby/Kimberly Nesbitt/Cleopatra James  
John Hancock  
Aaron Burgess  
Lisa Wesley  
Andrew Pearson  
Chuck Hasty/Matt Sanders